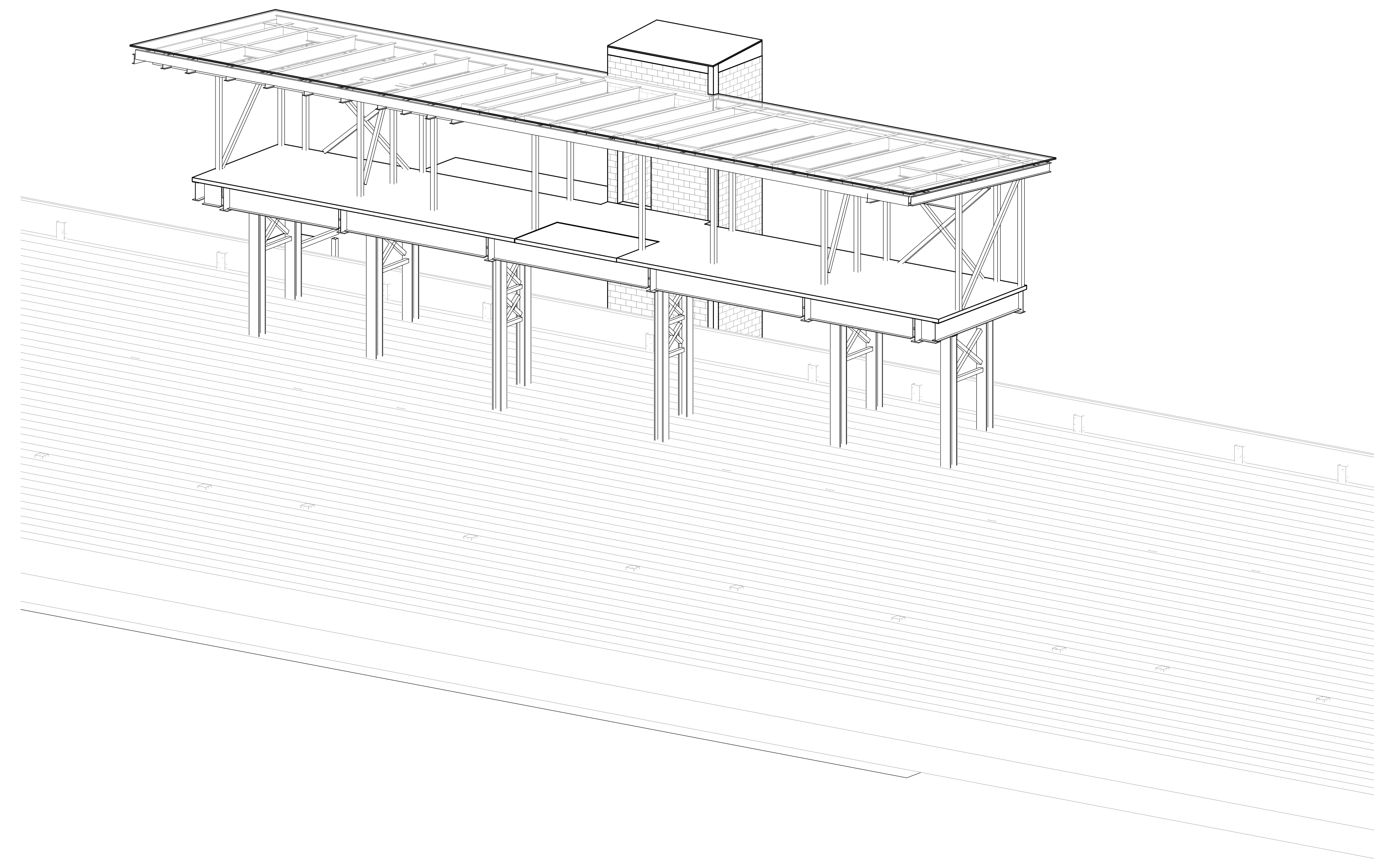
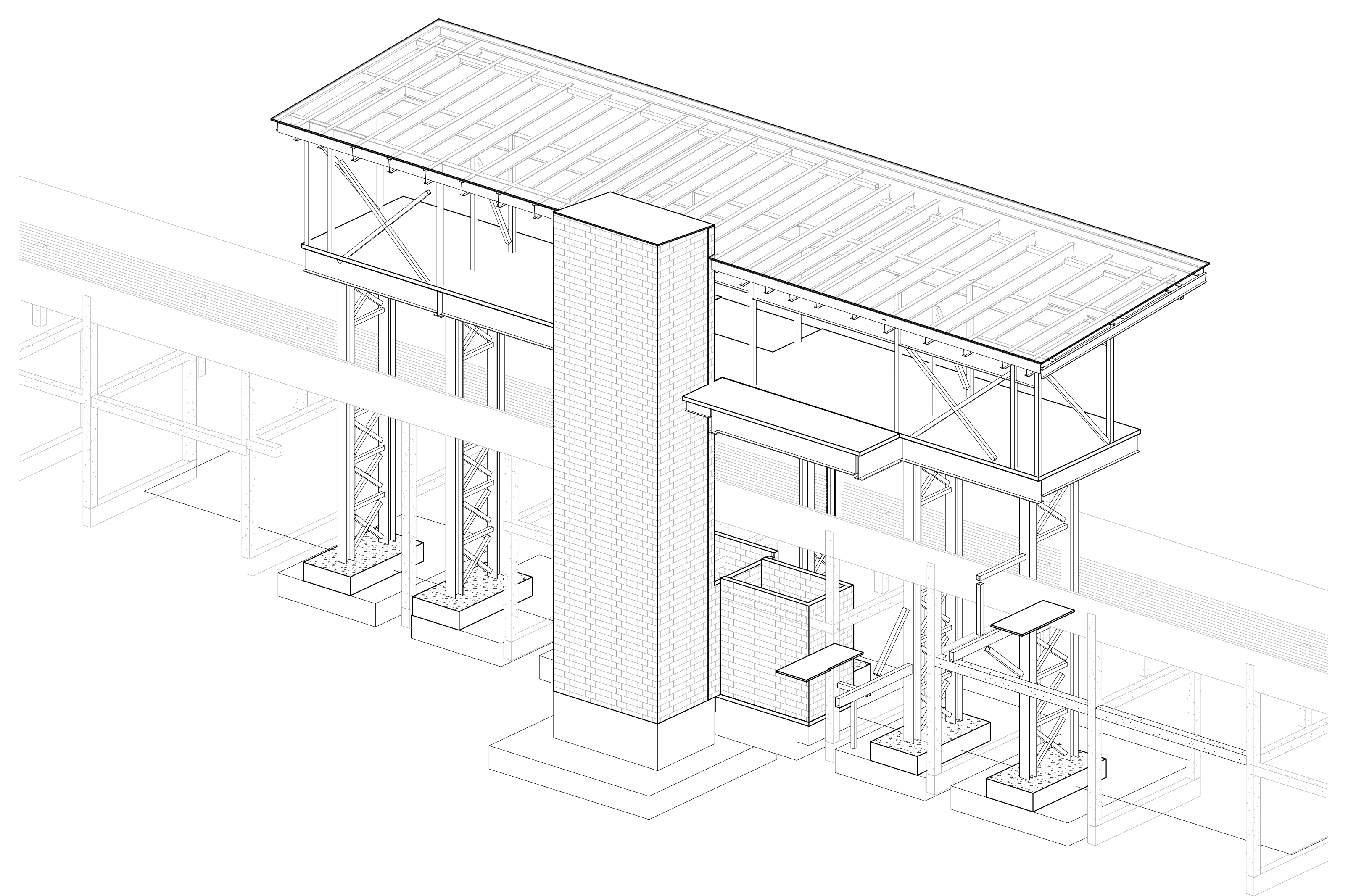


STRUCTURAL SHEET LIST

SHEET NUMBER	SHEET NAME
S-000	3D VIEW & SHEET LIST
S-010	GENERAL NOTES
S-011	GENERAL NOTES
S-012	GENERAL NOTES
S-013	GENERAL SUBGRADE NOTES AND TYP DETAILS
S-101	PRESS BOX FOUNDATION PLAN
S-102	PRESS BOX LEVEL 1 FRAMING PLAN
S-103	PRESS BOX ROOF FRAMING PLAN
S-104	STAIR FRAMING PLANS
S-300	GENERAL CONCRETE AND STL REINF NOTES AND TYP DETAILS
S-301	GENERAL SLAB-ON-GRADE NOTES AND TYP DETAILS
S-302	GENERAL GRADE BEAM NOTES AND TYP DETAILS
S-303SF	GENERAL FOUNDATION NOTES AND TYP DETAILS
S-310	FOUNDATION DETAILS
S-311	FOUNDATION DETAILS
S-313	FOUNDATION DETAILS (CMU)
S-315	ELEVATOR PIT DETAILS
S-400	GENERAL CMU NOTES AND TYP DETAILS
S-401	GENERAL CMU NOTES AND TYP DETAILS
S-500	GENERAL STEEL NOTES AND TYP DETAILS
S-501	GENERAL STEEL NOTES AND TYP DETAILS
S-503	GENERAL STEEL CONNECTION NOTES AND TYP DETAILS
S-504	GENERAL COMPOSITE STEEL NOTES AND TYP DETAILS
S-505	GENERAL COMPOSITE STEEL NOTES AND TYP DETAILS
S-510	TYP FLOOR FRAMING DETAILS
S-525	PRESS BOX BUILDING SECTIONS
S-528	PRESS BOX BUILDING SECTIONS
S-600	TYPICAL WIND BRACING ELEVATIONS
S-601	TYPICAL WIND BRACING ELEVATIONS
S-610	TYPICAL WIND BRACING DETAILS



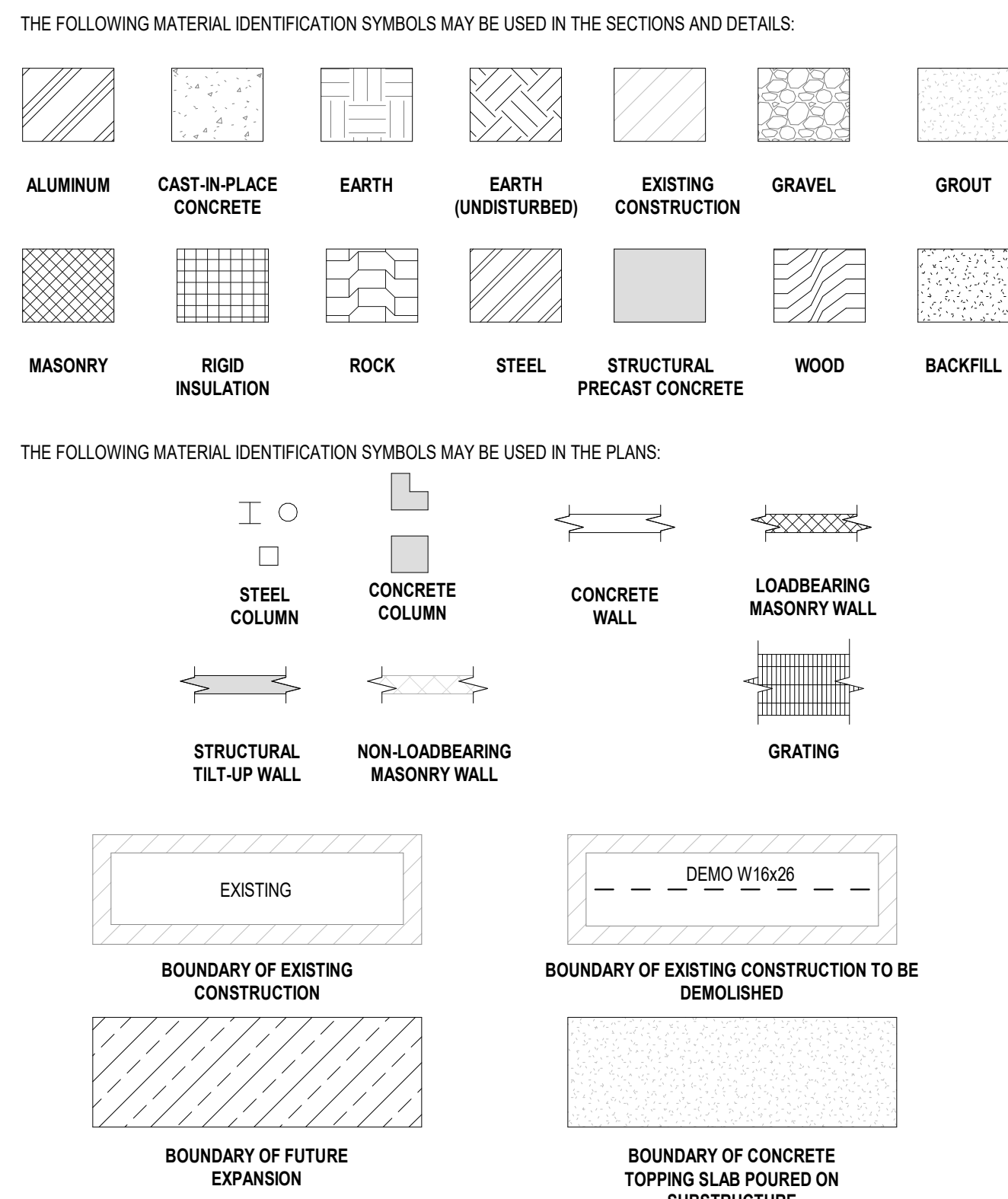
1 3D VIEW 1



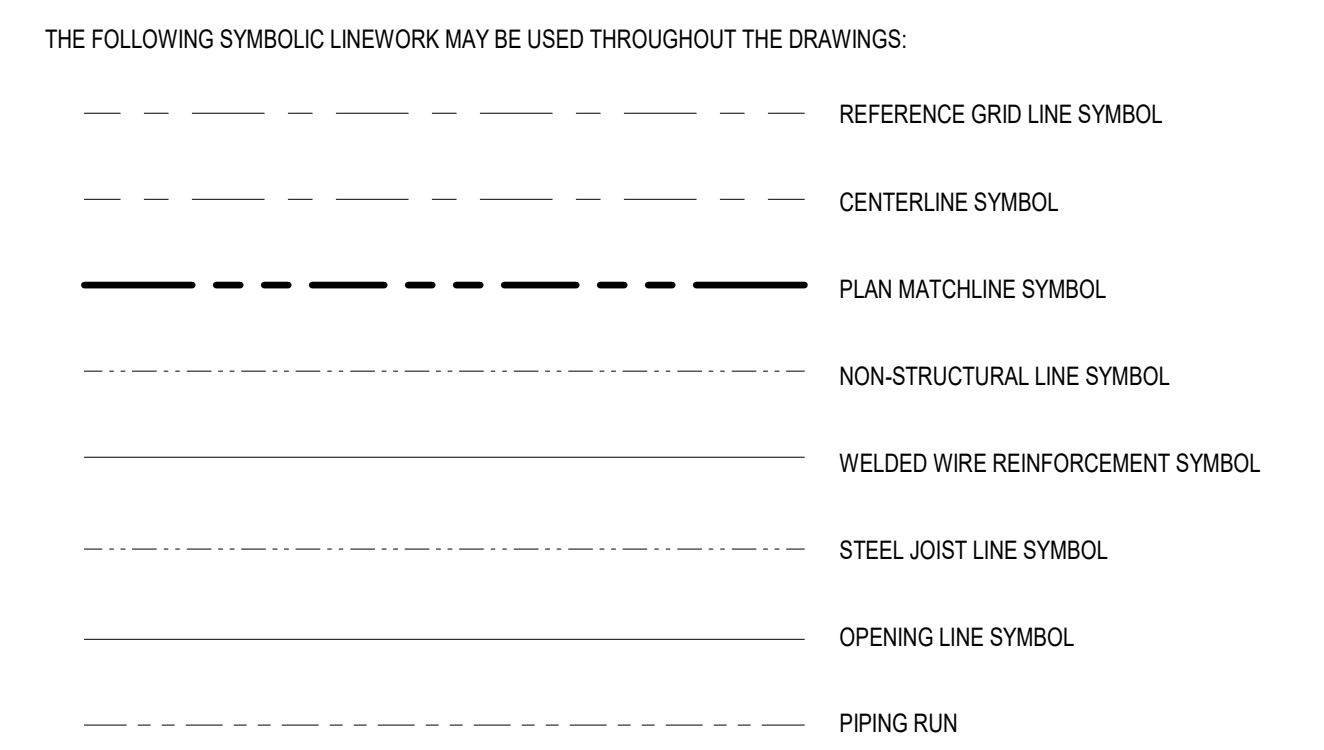
2 3D VIEW 2

NOTE:
 3D IMAGES PRESENTED FOR INFORMATIONAL PURPOSES ONLY AND DO NOT SUPERCEDE INFORMATION SHOWN IN 2D PLANS AND DETAILS. PERSPECTIVE VIEWS ARE PROVIDED FOR A GENERAL UNDERSTANDING OF THE OVERALL STRUCTURAL FRAME. NOT ALL STRUCTURAL ELEMENTS ARE NECESSARILY SHOWN. THESE VIEWS SHOULD NOT BE USED FOR BIDDING, DETAILING OR ERECTION.

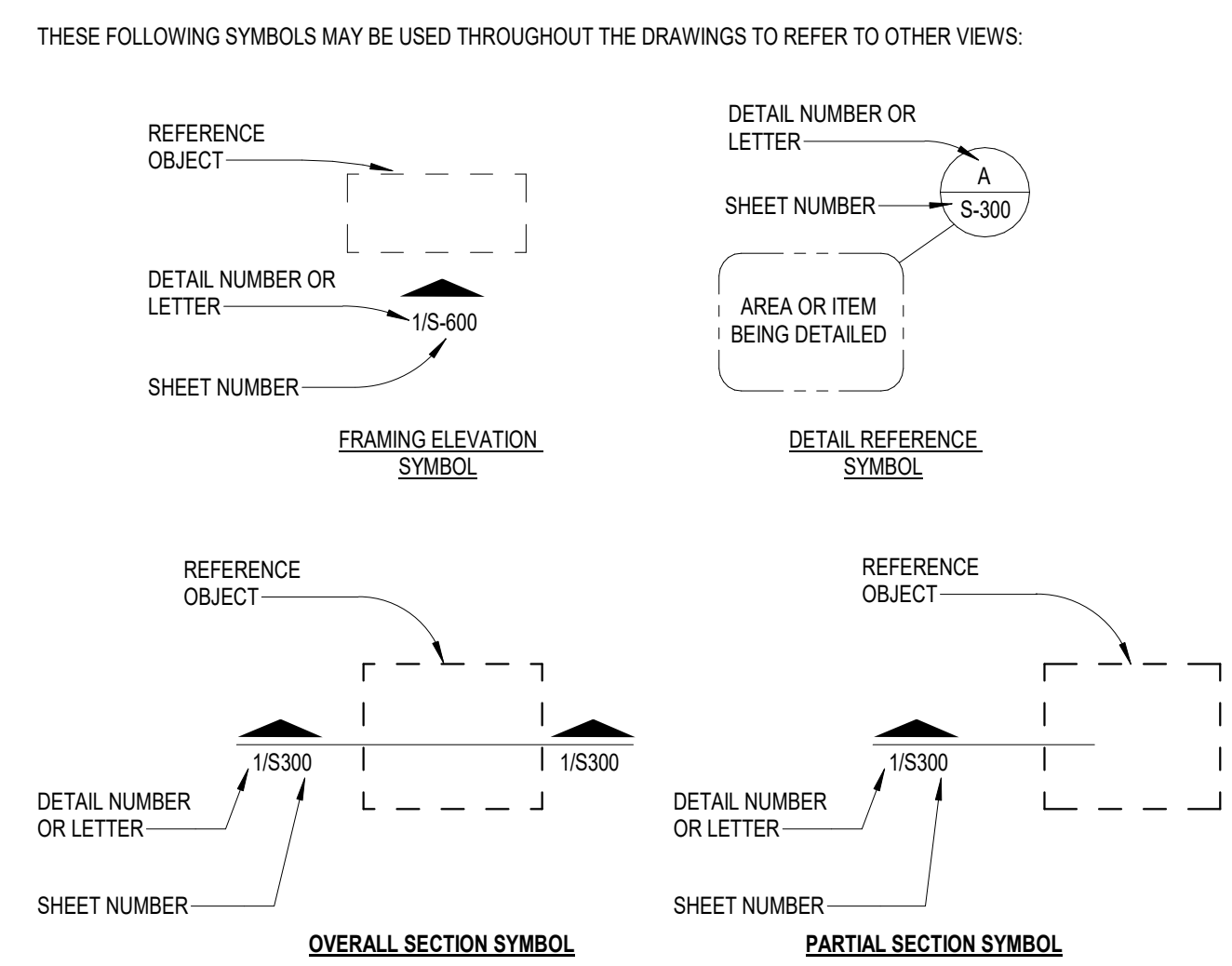
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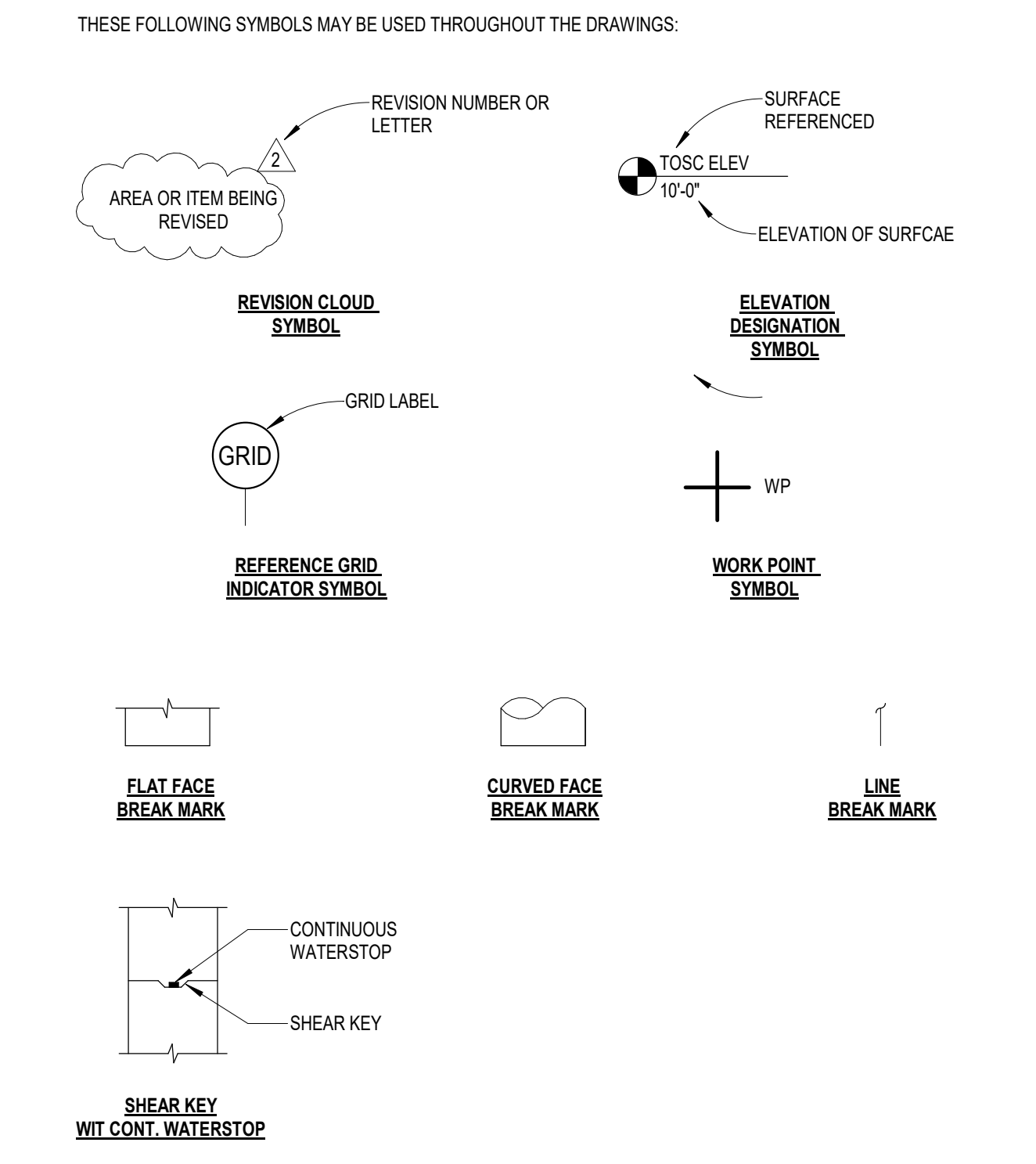
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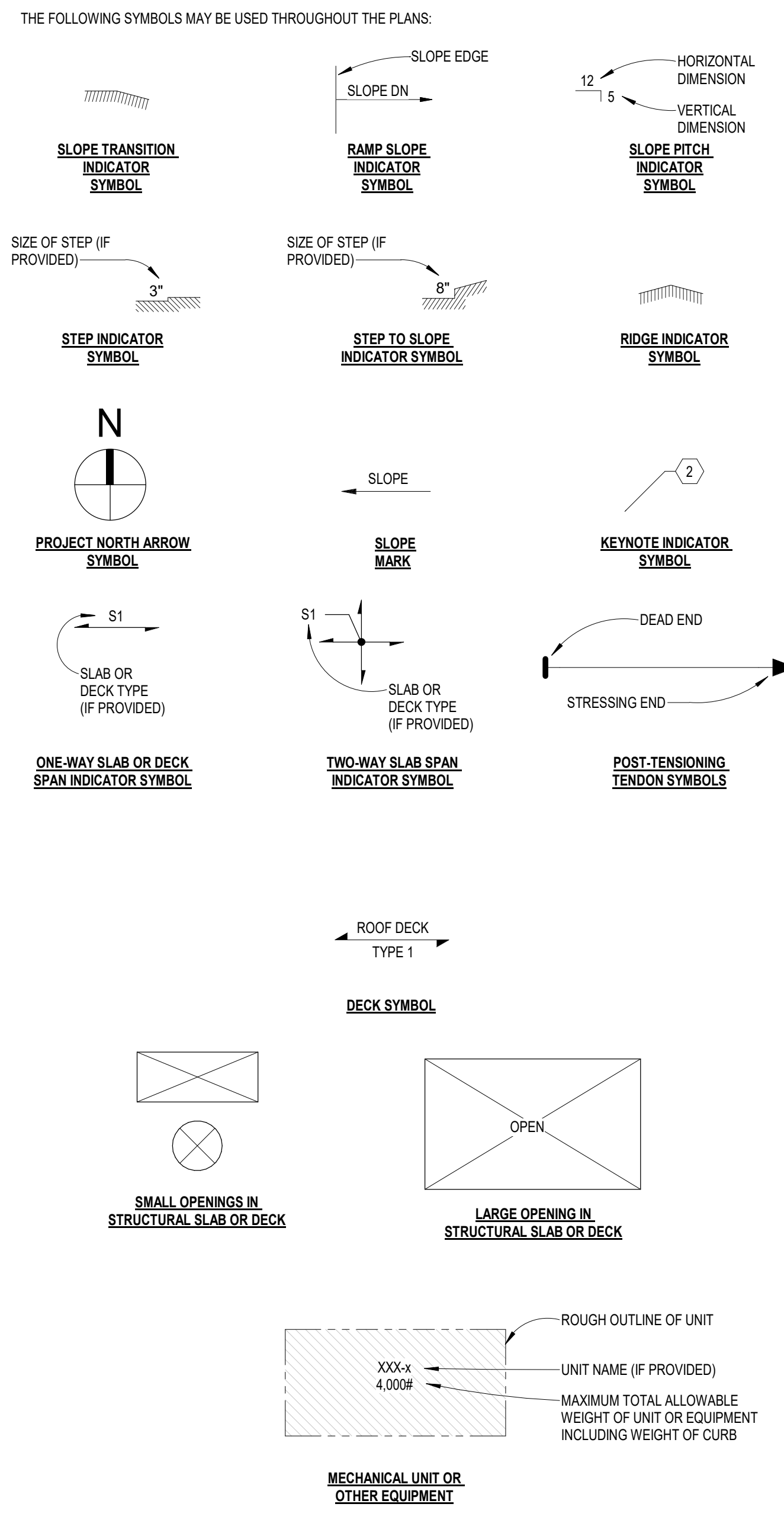
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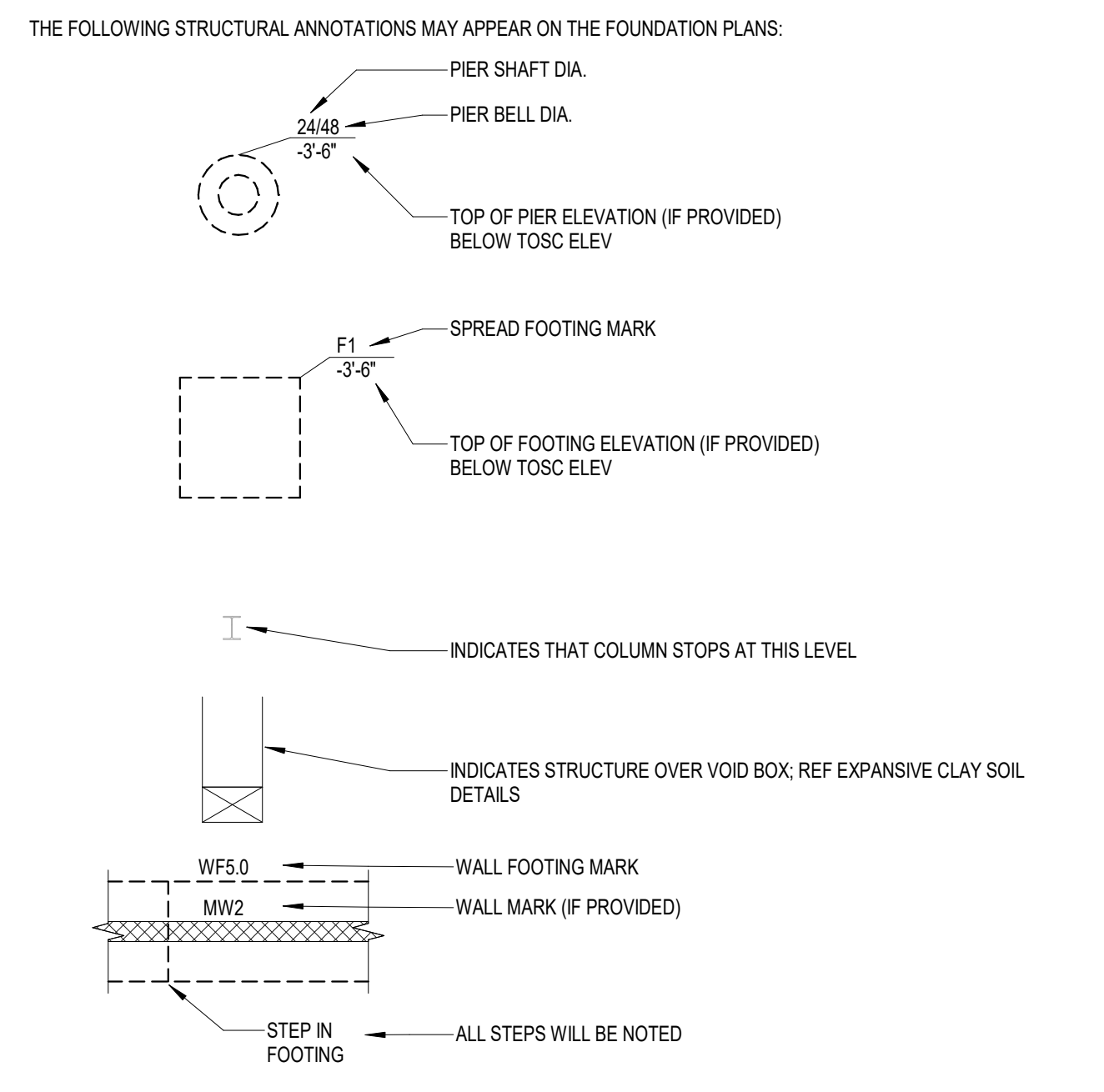
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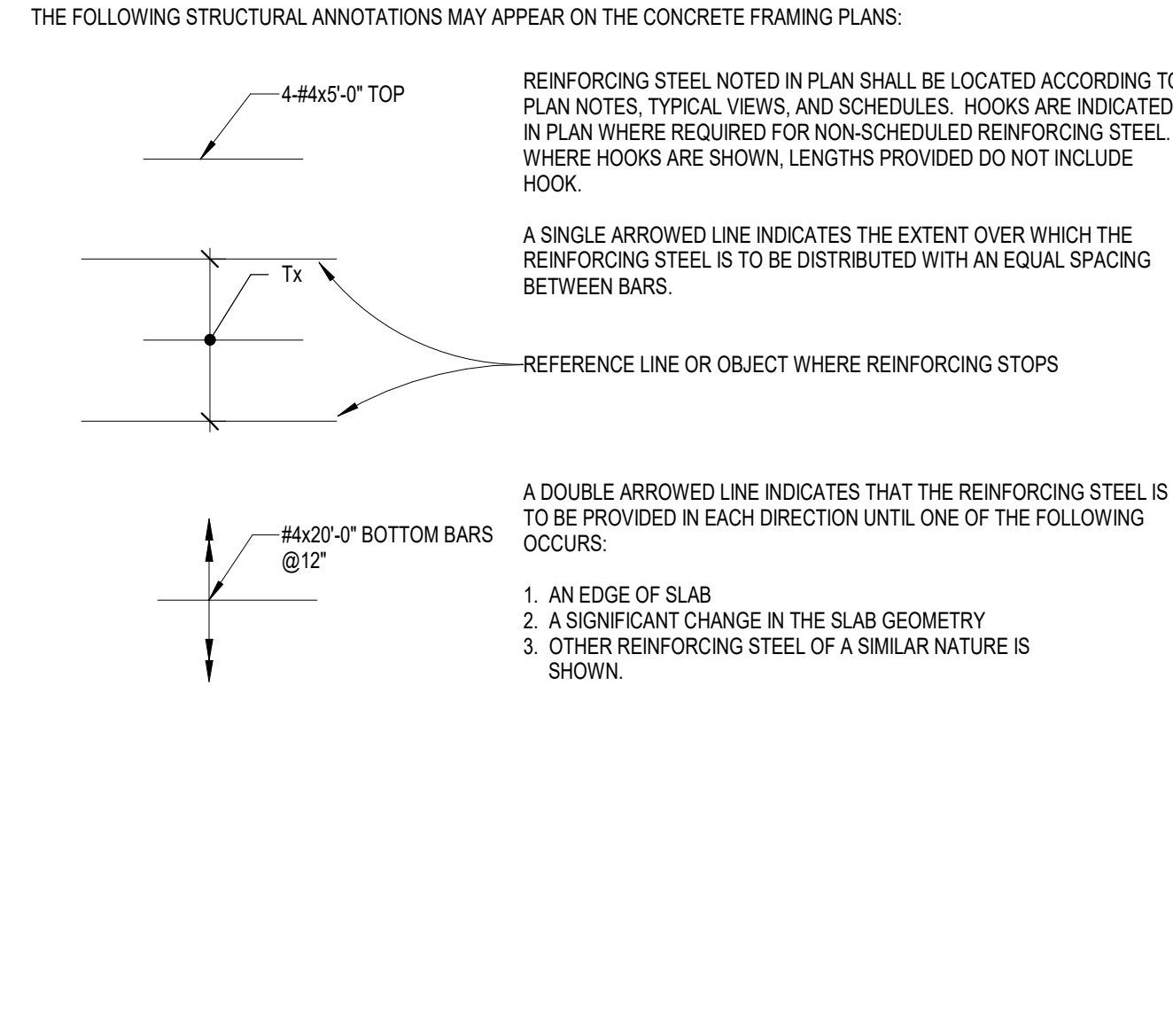
PLAN SYMBOLS



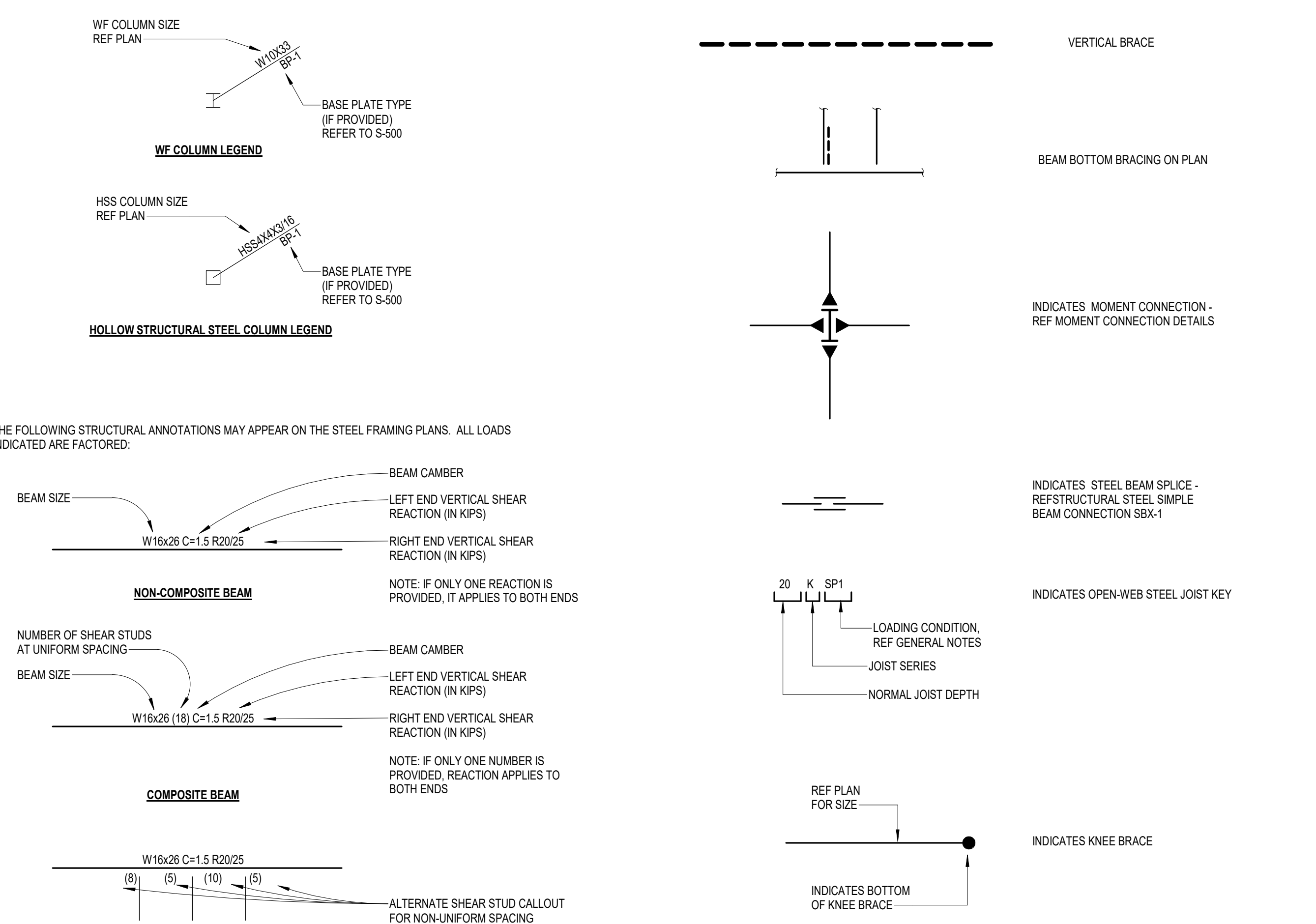
FOUNDATION ANNOTATION STYLES



CONCRETE ANNOTATION STYLES



STEEL ANNOTATION STYLES



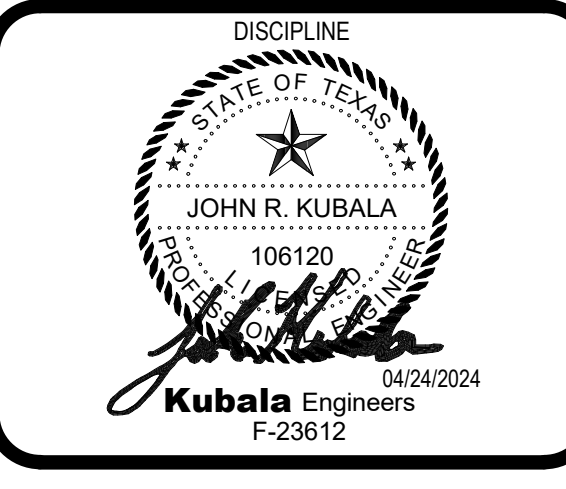
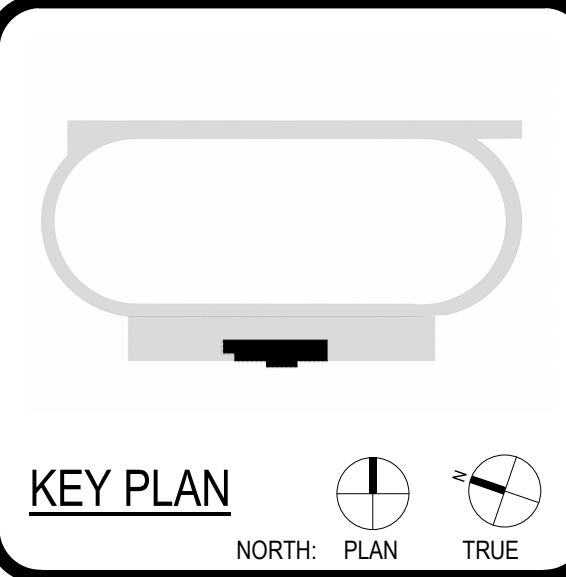
TYPICAL STRUCTURAL ABBREVIATIONS

A	ANCHOR BOLT	D	DOWEL BAR ANCHOR	H	HEIGHT	Q	ON CENTER	I	TEMP	TEMPERATURE
AB	ABOVE FINISH FLOOR	DBL	DOUBLE	HORIZ	HORIZONTAL	OD	OUTSIDE DIAMETER	T	TENSION	TERRAZO
ACI	AMERICAN CONCRETE INSTITUTE	DET	DETAIL	HSA	HEADED STUD ANCHOR	OF	OUTSIDE FACE	THK	THICK	TOP AND BOTTOM
ADDL	ADDITIONAL	DIA	DIAMETER	DIAG	DIAGONAL	OH	OPPOSITE HAND	T&B	TOP OF BEAM	TOP OF FOOTING
ADJ	ADJACENT	DIM(S)	DIMENSION(S)	IBC	INTERNATIONAL BUILDING CODE	OPNG(S)	OPENING(S)	TOB	TOP OF BEAM	TOP OF STRUCTURAL CONCRETE
AFF	ABOVE FINISHED FLOOR	DL	DEAD LOADS	INFO	INFORMATION	E	POWDER ACTUATED FASTENER	TOF	TOP OF FOOTING	TOP OF STEEL
AFS	ARCH FINISH SURFACE	DM	DOUBLE TEE	INT	INTERIOR	PAF	PRECAST AND PRESTRESSED CONCRETE INSTITUTE	TOSC	TOP OF WALL	TYPICAL
AGGR	AGGREGATE	DS	DOWNSPOUT	INTERM	INTERMEDIATE	PCI	POST-TENSIONING INSTITUTE	TOW	TYPICAL	
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	DWG(S)	DRAWING(S)	ISO	ISOLATION	PE	PROFESSIONAL ENGINEER	UNO	UNLESS NOTED OTHERWISE	
AISI	AMERICAN IRON AND STEEL INSTITUTE	DWL(S)	DOWEL(S)	IT	INVERTED TEE	PEMB	PRE-ENGINEERED METAL BUILDING			
ALT	ALTERNATE			JT	JOINT	PENTR	PENETRATION			
ARCH	ARCHITECTURAL	E	EACH	JBE	JOIST BEARING ELEVATION	PL	PLATE			
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	EA	EACH	JST	JOIST	PLAM	PLASTIC LAMINATE			
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	EAO	EDGE ANGLE OFFSET	JT	JOIST	PLF	POUNDS PER LINEAR FOOT			
AWS	AMERICAN WELDING SOCIETY	EDC	ELECTRICAL DISTRIBUTION CENTER	K	1000 POUNDS	PP	PARTIAL PENETRATION			
		EF	EACH FACE	KSF	KIPS PER SQUARE FOOT	PREFAB	PREFABRICATED			
		EIFS	EXTERIOR INSULATION AND FINISH SYSTEM	KSI	KIPS PER SQUARE INCH	PRELIM	PRELIMINARY			
BF	BACK FACE	EJ	EXPANSION JOINT	L	ANGLE OR LENGTH	PSF	POUNDS PER SQUARE FOOT	W	WATERSTOP	WATERPROOFING
BFB	BOTTOM FLANGE BRACE	EL	ELEVATION	Lb	POUNDS	PT	POST-TENSIONING	WB	WIND BRACE	WIND LOAD
BFf	BELOW FINISH FLOOR	ELEC	ELECTRICAL	Ld	TENSION DEVELOPMENT LENGTH AS SHOWN IN REINFORCING LAP SCHEDULE	Q	QUANTITY	WL	WIND LOAD	
BL	BLOCK LINTEL	ENGR	ENGINEER	LL	LIVE LOAD			W	WITH	WITHOUT
BLE	BRACKET LEDGE ELEVATION	ENR	ENGINEER	LLH	LONG LEG HORIZONTAL			W/O	WITHOUT	WORKING POINT
BLK	BLOCK	EQ	EQUAL	LLV	LONG LEG VERTICAL			WP	WORKING POINT	WOOD
BLKG	BLOCKING	EQUIV	EQUIVALENT	LOC	LOCATION			WD	WOOD	
BM	BEAM	EW	EACH WAY	LONG	LONGITUDINAL					
BO	BOTTOM OF	EXIST	EXISTING	LSH	LONG SIDE HORIZONTAL					
BOA	BACK OF ANGLE	EXP	EXPANSION	LSV	LONG SIDE VERTICAL					
BOS	BOTTOM OF STEEL	EXT	EXTERIOR	LWC	LIGHT WEIGHT CONCRETE					
BOT	BOTTOM	EX-STR	EXTRA STRONG							
BP	BASE PLATE	E	EACH							
BRG	BEARING	FABR	FABRICATOR							
BRKT	BRACKET	FD	FLOOR DRAIN							
BLE	BRICK LEDGE ELEVATION	FDN	FOUNDATION							
B/TWN	BETWEEN	FF	FINISHED FLOOR							
		FL	FLOOR							
C	CHANNEL	FLG	FLANGE							
COMP	COMPRESSION	FM	FACTORY MUTUAL							
CANT	CANTILEVER	FRP	FIBER REINFORCED PLASTIC							
CGS	CENTROID OF TENDONS	FS	FAR SIDE							
CIP	CAST IN PLACE	FTG	FOOTING							
CJ	CONTROL JOINT	FP	FIREPROOFING							
CL	CENTER LINE	FV	FIELD VERIFY							
CJP	COMPLETE JOINT PENETRATION									
CLR	CLEAR	G	GAUGE							
CMU	CONCRETE MASONRY UNIT	GA	GAUGE							
COL	COLLUMN	GB	GRADE BEAM							
COMP	COMPRESSION	GALV	GALVANIZED							
CONN	CONNECTION	GC	GENERAL CONTRACTOR							
CONC	CONCRETE	GR	GRADE							
CONST	CONSTRUCTION	GYP BD	GYPSUM BOARD							
CONT	CONTINUOUS									
CONTR	CONTRACTOR									
COR	CORNER									
CRSI	CONCRETE REINFORCING STEEL INSTITUTE									



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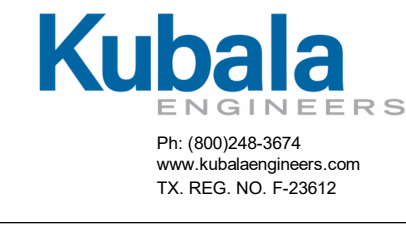


CLIENT: GALVESTON ISD

No.	Description	Date
1	AS#1	04/24/2024

ISSUE FOR PROPOSAL

GENERAL NOTES



GENERAL:

CODES, DRAWINGS AND SPECIFICATIONS:

1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE (IBC 2021). ALL CODES AND SPECIFICATIONS LISTED ABOVE SHALL INCLUDE ALL AMENDMENTS AND ADDENDA IN FORCE AT THE DATE OF THE CONTRACT DOCUMENTS.

TYPICAL DETAILS:

1. TYPICAL DETAILS SHOWN ON THE DRAWINGS SHALL APPLY TO ALL SIMILAR LIKE CONDITIONS OCCURRING ON THE PROJECT WHETHER OR NOT THEY ARE KEYS IN AT EACH PARTICULAR LOCATION.

MISCELLANEOUS:

- WHERE CONFLICTS EXIST BETWEEN THE VARIOUS PUBLICATIONS AS SPECIFIED HEREIN, THE STRICTER REQUIREMENTS SHALL GOVERN UNLESS NOTED OTHERWISE WHERE CONFLICTS EXIST BETWEEN THE VARIOUS PARTS OF THE STRUCTURAL CONTRACT DOCUMENTS (STRUCTURAL DOCUMENTS, SPECIFICATIONS) AS SPECIFIED HEREIN, THE STRICTER REQUIREMENTS SHALL GOVERN.
- IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND TO SUBMIT SUCH DOCUMENTS TO ALL SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD.
- THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE.
- SLEEVES AND BLOCKOUTS REQUIRED FOR PASSAGE OF DUCTWORK, PIPING, DRAINS, CONDUIT, ETC., AND ANCHORS REQUIRED FOR ANCHORING EQUIPMENT AND PIPING ARE NOT GENERALLY INDICATED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL DETERMINE SUCH REQUIREMENTS FROM OTHER SERIES DRAWINGS, SUBCONTRACTORS, AND SUPPLIERS AND SHALL COORDINATE THE LOCATIONS AND DETAILS FOR THESE ITEMS PRIOR TO FABRICATION OR CONSTRUCTION OF THE STRUCTURE. ANY CONFLICTS BETWEEN THESE ITEMS AND THE BUILDING STRUCTURE SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT FOR RESOLUTION.
- VERIFY, OR ESTABLISH, LOCATIONS AND DIMENSIONS OF ALL FRAMED OPENINGS RELATED TO EQUIPMENT OR DUCTWORK, INCLUDING INSULATION, IF ANY, WHERE SUBSTANTIAL RELOCATION OR RECONFIGURATION IS REQUIRED. SUBMIT A DRAWING TO THE ARCHITECT FOR REVIEW.
- MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL WHICH ARE NOT AS SPECIFIED IN THE DOCUMENTS SHALL BE ACCOMPANIED BY A CURRENT TEST REPORT (BY ICC EVALUATION SERVICE, INC.) OR ICBO REPORT BY INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS), MATERIALS OR PRODUCTS THAT DO NOT HAVE AN ICBO OR ICBO REPORT INDICATING THE SUBSTITUTED MATERIAL OR PRODUCT TO BE EQUAL TO THAT SPECIFIED, WILL NOT BE CONSIDERED.
- IF CERTAIN FEATURES ARE NOT FULLY SHOWN OR SPECIFIED ON THE DRAWINGS OR IN THE SPECIFICATIONS, THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS SHOWN OR SPECIFIED IN SIMILAR CONDITIONS.
- THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
- ALL HEAVILY LOADED VEHICLES, CONCRETE TRUCKS AND CRANES SHALL NOT BE DRIVEN ACROSS GRADE BEAMS OR BUILDING SLABS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGES TO THE SLAB INDUCED FROM THIS TYPE OF EQUIPMENT.
- ERECTION OF STRUCTURAL STEEL MAY NOT BEGIN UNTIL CONCRETE FOUNDATION HAS CURED FOR A MINIMUM OF THREE DAYS. STRUCTURAL STEEL OR OTHER HEAVY LOADS SHALL NOT BE STOCKPILED ON ANY SLAB UNTIL IT HAS CURED FOR A MINIMUM OF SEVEN DAYS.
- NOTE THAT THE GROUND FLOOR SLAB IS A GROUND SUPPORTED SLAB AT GRADE AS PER THE DESIGN RECOMMENDED IN THE SOIL REPORT. IT IS NOT A STRUCTURAL SLAB AND AS SUCH IT IS NOT DESIGNED FOR ANY EXTERNAL UPWARD OR DOWNWARD LOADS. IT IS INTENDED TO BE ENTIRELY SUPPORTED BY THE PREPARED GROUND UNDER THE SLAB. THE CONTRACTOR SHOULD NOTE THAT THE PERFORMANCE OF THE SLAB AS DESIGNED AND INTENDED BY THE SOIL ENGINEER IS HIGHLY DEPENDENT ON HOW WELL THE CONTRACTOR FOLLOWS THE SITE PREPARATION INSTRUCTION IN THE SOIL REPORT.
- ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. THE ABILITY OF THE STRUCTURAL FRAME TO RESIST THE REQUIRED CODE FORCES DERIVES FROM THE COMPLETE INSTALLATION OF THE LATERAL FORCE RESISTING SYSTEMS AND DIAPHRAGMS DESCRIBED BELOW. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND ALL DESIGNATED CONCRETE ELEMENTS (IF ANY) HAVE REACHED A MINIMUM OF 75% OF THEIR DESIGN STRENGTH.
- THE STRUCTURE HAS BEEN DESIGNED FOR THE LOADS IDENTIFIED WITHIN THESE STRUCTURAL DRAWINGS THAT ARE ANTICIPATED TO BE APPLIED TO THE FINAL STRUCTURE ONCE COMPLETED AND OCCUPIED. THE CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING THE ADEQUACY OF THE STRUCTURE TO SUPPORT ANY APPLIED CONSTRUCTION LOADS, INCLUDING THOSE DUE TO CONSTRUCTION VEHICLES OR EQUIPMENT, MATERIAL HANDLING OR STORAGE, SHORING OR RESHORING, OR ANY OTHER CONSTRUCTION ACTIVITY. THE CONTRACTOR SHALL SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED VERIFYING THE ADEQUACY OF THE STRUCTURE FOR ANY PROPOSED CONSTRUCTION LOADS THAT ARE IN EXCESS OF THE STATED DESIGN LOADS. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE TO DESIGN OR CHECK THE STRUCTURE FOR LOADS APPLIED TO THE STRUCTURE FOR ANY CONSTRUCTION ACTIVITY.

WIND-BORNE DEBRIS REGION:

1. THE PROPOSED BUILDING IS LOCATED WITHIN ONE OF THE TEXAS DEPARTMENT OF INSURANCE DESIGNATED PER 2018 IBC / IRC CATASTROPHE ZONES. THE OWNER IS HEREBY INFORMED THAT THEY WILL NEED TO HIRE AN INDEPENDENT THIRD-PARTY SPECIAL INSPECTION COMPANY TO PROVIDE WINDSTORM CERTIFICATION (WP-1 FORM, WP-2 FORM, AND WP-8 CERTIFICATE) FOR THE BUILDING / PROJECT. THE INDEPENDENT WINDSTORM INSPECTION COMPANY MUST HAVE A LICENSED ENGINEER CERTIFIED BY AND APPROVED BY THE STATE OF TEXAS TO PARTICIPATE IN THE TEXAS WINDSTORM INSURANCE ASSOCIATION'S WINDSTORM PROGRAM. THE APPLICATION FOR WINDSTORM BUILDING INSPECTION, FORM WP-1, MUST BE SUBMITTED TO THE TEXAS DEPARTMENT OF INSURANCE PRIOR TO THE START OF CONSTRUCTION. PLEASE NOTE: THE WP-1 FORM, THE WINDSTORM CERTIFICATION AND INSPECTION WORK, AND THE COMPLETION OF THE WP-2 FORM HAVE NOT BEEN INCLUDED IN THE STRUCTURAL ENGINEER'S (EOR'S) SCOPE OF WORK FOR THIS PROJECT. AS NOTED ABOVE, THIS SERVICE SHALL BE PROVIDED BY AN ENGINEER APPOINTED BY THE COMMISSIONER OF INSURANCE TO INSPECT AND CERTIFY COMPLIANCE OF THE STRUCTURES AS ELIGIBLE FOR WINDSTORM HAIL AND ANY OTHER AVAILABLE COVERAGE THROUGHOUT THE TEXAS WINDSTORM INSURANCE ASSOCIATION. ARRANGEMENTS FOR WINDSTORM INSPECTIONS SHALL BE COORDINATED BETWEEN THE OWNER, THE GENERAL CONTRACTOR, AND THE WINDSTORM INSPECTOR PRIOR TO THE START OF CONSTRUCTION. ALL PERTINENT SUBMITTALS RELATED TO THE EXTERIOR BUILDING ENVELOPE SHALL NOT BE CONSIDERED APPROVED UNTIL THE WINDSTORM CERTIFICATION COMPANY HAS REVIEW AND APPROVED THE SUBMITTAL. THE GC SHALL COORDINATE WITH THE WINDSTORM CERTIFICATION COMPANY TO DETERMINE ALL SUBMITTALS REQUIRED FOR REVIEW AND APPROVAL.

DESIGN CRITERIA:

DEAD LOADS:

1. DEAD LOADS INCLUDE THE WEIGHT OF THE STRUCTURAL COMPONENTS AND ALLOWANCES FOR PERMANENT PARTITIONS, PERMANENT FIXTURES, FINISHES, ROOFING, MECHANICAL, ELECTRICAL, PLUMBING AND FIRE PROTECTION MATERIALS SHOWN OR SPECIFIED.

2. LOADINGS FOR MECHANICAL ROOMS ARE BASED ON THE WEIGHTS OF ASSUMED EQUIPMENT AS INDICATED ON THE MECHANICAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS, WHERE INDICATED). ANY CHANGES IN TYPE, SIZE, LOCATION OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE ARCHITECT FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT.

DESIGN DEAD LOADS IS AS FOLLOWS:

FLOOR SELF WEIGHT + 10 PSF COLLATERAL
SELF WEIGHT + 25 PSF COLLATERAL

LIVE LOADS:

1. DESIGN LIVE LOADING IS AS FOLLOWS:

ROOF 20 PSF, 300# CONCENTRATED LOAD (REDUCIBLE)
TYPICAL FLOORS 100 PSF, 1000# CONCENTRATED LOAD (REDUCIBLE)
ALL SLABS-ON-GRADE 100 PSF
MECHANICAL/ELECTRICAL ROOM (MIN.) 100 PSF (UNREDUCIBLE)
ELEVATOR MACHINE ROOM 150 PSF (UNREDUCIBLE)
HANDRAILS AND GUARDRAILS 50 PSF OR 200# APPLIED AT TOP RAIL IN ANY DIRECTION
STAIRWAYS AND EXITS 100 PSF + 300# CONCENTRATED LOAD AT TREAD MIDSPAN

SNOW LOADS:

1. DESIGN SNOW LOADING IS AS FOLLOWS:

GROUND SNOW LOAD, P_s 0 PSF
SNOW LOAD IMPORTANCE FACTOR I 1
SNOW RISK CATEGORY III

WIND LOADS:

1. WIND PRESSURES ARE BASED ON THE PROVISIONS OF THE AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-16, CITY OF GALVESTON AMENDMENTS, THE TEXAS DEPARTMENT OF INSURANCE WINDSTORM ZONE, AND THE FOLLOWING CRITERIA:

A. WIND DESIGN DATA (USED FOR PRESSURE DETERMINATION FOR THE DESIGN OF BUILDING)

- BASIC WIND SPEED, V 160 MPH (3-SECOND GUST) (ULTIMATE)
- WIND OCCUPANCY CATEGORY III
- WIND IMPORTANCE FACTOR, I 1.0
- WIND EXPOSURE CATEGORY C
- INTERNAL PRESSURE COEFFICIENT, G_{pi} +0.18
- WIDTH OF END ZONE, Z_e 8 FT

B. WIND-BORNE DEBRIS DATA (USED TO DETERMINE IF BUILDING IS LOCATED WITHIN A WIND-BORNE DEBRIS REGION ONLY. DO NOT USE FOR ANY PRESSURE DETERMINATIONS!)**

- WIND-BORNE DEBRIS BASIC WIND SPEED TRIGGER, V 140 MPH (3-SECOND GUST) (ULTIMATE)
- BASIC WIND SPEED, V 150 MPH
- WIND OCCUPANCY CATEGORY II
- WIND-BORNE DEBRIS PROTECTION REQUIRED? YES

*** - THE INFORMATION SHOWN IN WIND LOADS SECTION 1.B IS ONLY PROVIDED TO INDICATE IF THE BUILDING IS WITHIN A WIND-BORNE DEBRIS AREA. THIS SECTION DOES NOT APPLY TO ANY COMPONENTS THAT ARE LISTED AS DELEGATED DESIGN. ALL DELEGATED DESIGN COMPONENTS SHALL BE DESIGN FOR THE WIND DESIGN DATA AND DESIGN WIND PRESSURES PROVIDED IN SECTION 1.A AND 1.C.

C. DESIGN WIND PRESSURES

1. MAIN WIND-FORCE RESISTING SYSTEM (MWFRS)

I. WALLS (WW+LW)

0'-3/8' 65 PSF
3'-0/8' 70 PSF

2. COMPONENTS AND CLADDING

I. WALLS (AREA = 75 SF)

INTERIOR ZONE 65 PSF
END ZONE 75 PSF

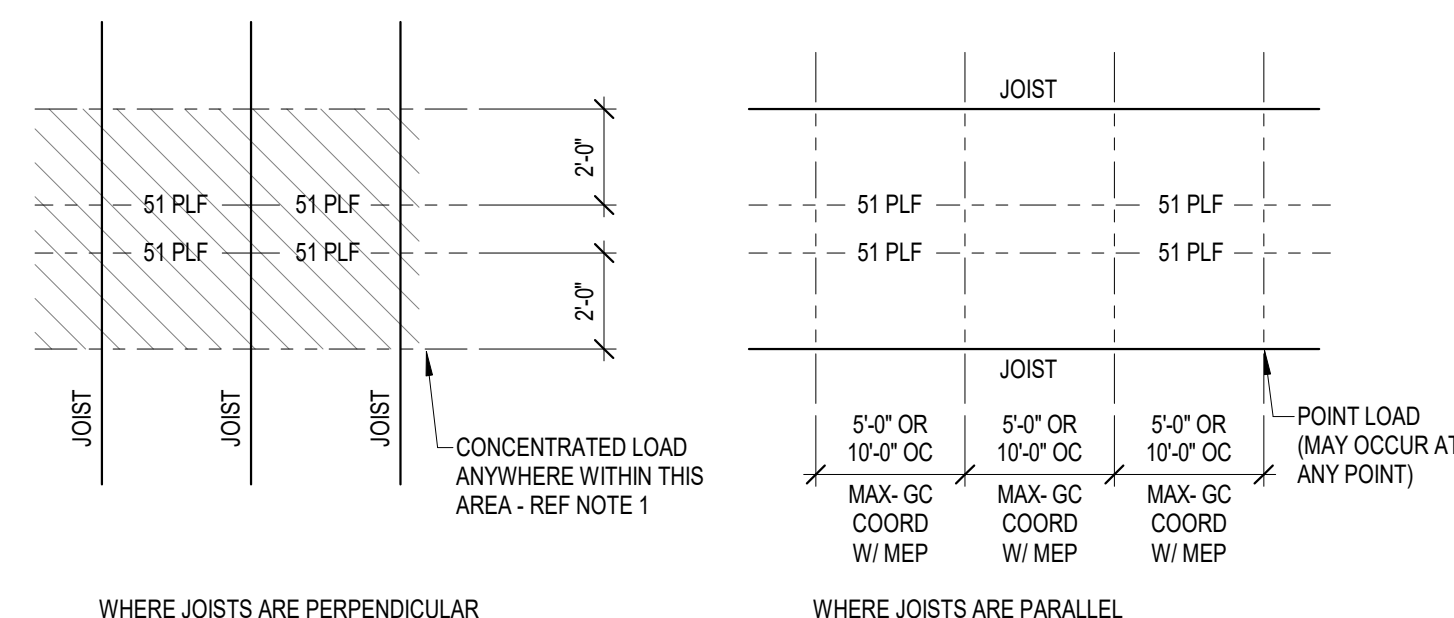
III. ROOF UPLIFT (GROSS) - ROOF ATTACHMENTS (AREA = 10 SF)

INTERIOR ZONE 1' 80 PSF
INTERIOR ZONE 80 PSF
END ZONE 110 PSF
CORNER ZONE 172 PSF
OVERHANG INTERIOR ZONE 110 PSF
OVERHANG END ZONE 110 PSF
OVERHANG CORNER ZONE 172 PSF

MISCELLANEOUS LOADS:

1. ALL JOISTS ALONG PIPE RUN SHALL BE DESIGNED FOR SUSPENDED PIPE WEIGHT. IT'S THE GENERAL CONTRACTOR'S RESPONSIBILITY TO COORDINATE HOW OFTEN PIPES WILL BE HUNG (5'-0" OC OR 10'-0" OC) SO THAT PROPER LOADING MAY BE ACCOUNTED FOR. IT IS RECOMMENDED TO HANG FROM EACH JOIST (GC MUST COORDINATE WITH MEP CONTRACTOR TO ENSURE SAME HANGING PATTERN IS FOLLOWED).

ADDITIONAL JOIST DEAD-LOAD FOR MECHANICAL PIPE LOADS WITH WATER		
PIPE SIZE (DIAMETER)	PERPENDICULAR CONDITION	PARALLEL CONDITION
3"	110 LBS AT 10'-0"	11 PLF
4"	170 LBS AT 10'-0"	17 PLF
5"	240 LBS AT 10'-0"	24 PLF
6"	320 LBS AT 10'-0"	32 PLF
8"	510 LBS AT 10'-0"	51 PLF
10"	750 LBS AT 10'-0"	75 PLF



WHERE JOISTS ARE PERPENDICULAR

WHERE JOISTS ARE PARALLEL

PIPE RUN JOIST LOADING

NOTE: WHERE PIPES RUN PERPENDICULAR TO JOISTS, JOISTS THAT ARE SUPPORTING PIPES SHALL BE DESIGNED FOR ADDITIONAL CONCENTRATED DEAD LOAD AT ANY POINT ALONG JOIST SPAN WITHING THE HATCHED AREA.

2. ARCHITECTURAL BARRIER ACT STANDARD:

ITEM	REQUIRED CAPACITY
GRAB BAR	250 POUNDS ANY DIRECTION
TUB OR SHOWER SEAT	250 POUNDS ANY DIRECTION
FASTENERS & MOUNTING DEVICES	250 POUNDS ANY DIRECTION

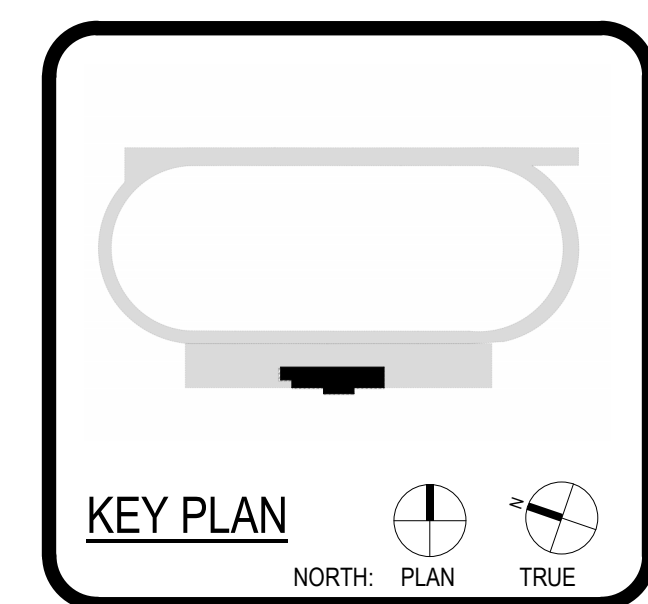
BUILDING MOVEMENT AND DEFLECTION:

A. ROOF MEMBERS	DEFLECTION LIMIT
1. LIVE LOAD MAXIMUM ALLOWABLE DEFLECTION	SPAN/360
2. TOTAL LOAD MAXIMUM ALLOWABLE DEFLECTION	SPAN/240
B. FLOOR MEMBERS	
1. LIVE LOAD MAXIMUM ALLOWABLE DEFLECTION	SPAN/360
2. TOTAL LOAD MAXIMUM ALLOWABLE DEFLECTION	SPAN/240
C. MEMBERS SUPPORTING MASONRY	
1. LIVE LOAD MAXIMUM ALLOWABLE DEFLECTION	SPAN/600 OR 3/8"
D. BUILDING FRAME	
1. MAXIMUM ALLOWABLE STORY DRIFT	HEIGHT/500 (TOTAL BUILDING HEIGHT)



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TX Firm BR 1698
TX Firm BR 1698
1113.000.0013
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LEAF ENGINEERS
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GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX
1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL



DISCIPLINE
STATE OF TEXAS
JOHN R. KUBALA
106120
04/24/2024
Kubala Engineers
F-23612

CLIENT: GALVESTON ISD

DATE: 01/31/2024	PROJECT NUMBER: 230063
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No.	Description	Date
1	ISSI #1	04/24/2024

ISSUE FOR PROPOSAL

BUILDING NUMBER

GENERAL NOTES



STRUCTURAL SUBMITTALS (NO LEEDS)

SPECIFICATION SECTION	ITEM	SUBMITTAL
03 10 00	FORMWORK SHOP DRAWINGS	FOR INFORMATION ONLY SIGNED AND SEALED
03 10 00	MANUFACTURER'S PRODUCT DATA FOR APPROVAL	FOR APPROVAL
03 10 00	CONSTRUCTION JOINT LAYOUT	FOR INFORMATION ONLY
03 20 00	STEEL REINFORCING SHOP DRAWINGS	FOR APPROVAL
03 20 00	EMBEDDED METAL ASSEMBLY SHOP DRAWINGS	FOR APPROVAL
03 20 00	MANUFACTURER'S PRODUCT INFORMATION FOR BAR SUPPORTS	FOR APPROVAL
03 20 00	MILL TEST CERTIFICATE OF STEEL REINFORCING	FOR INFORMATION ONLY
03 20 00	QUALIFICATION DATA	FOR INFORMATION ONLY
03 20 00	WELDING CERTIFICATES	FOR INFORMATION ONLY
03 30 00	CONCRETE MIX DESIGN	FOR APPROVAL
03 30 00	SLAB ON GRADE AND COMPOSITE SLAB CONSTRUCTION JOINT LAYOUT AND POUR SEQUENCE	FOR APPROVAL
03 30 00	MATERIAL CERTIFICATES FOR CONCRETE RELATED PRODUCTS	FOR APPROVAL
03 30 00	PRODUCT DATA	FOR INFORMATION ONLY
03 30 00	QUALIFICATION DATA	FOR INFORMATION ONLY
03 30 00	MATERIAL TEST REPORTS	FOR INFORMATION ONLY
03 30 00	FLOOR SURFACE FLATNESS	FOR INFORMATION ONLY
03 30 00	FIELD QUALITY CONTROL REPORTS	FOR INFORMATION ONLY
03 30 00	MINUTES OF PREINSTALLATION CONFERENCE	FOR INFORMATION ONLY
04 22 00	CMU WALL SHOP DRAWINGS	FOR APPROVAL
04 22 00	MIX DESIGN (MORTAR AND GROUT)	FOR APPROVAL
04 22 00	MASONRY COMPRESSIVE STRENGTH	FOR APPROVAL
04 22 00	SAMPLES FOR SELECTION AND VERIFICATION	FOR APPROVAL
04 22 00	MATERIAL CERTIFICATES	FOR INFORMATION ONLY
04 22 00	COLDHOT WEATHER PROCEDURES	FOR INFORMATION ONLY
04 22 00	QUALIFICATION DATA	FOR INFORMATION ONLY
05 12 00	STRUCTURAL STEEL FRAMING SHOP DRAWINGS	FOR APPROVAL
05 12 00	STEEL CONNECTION CALCULATIONS	FOR INFORMATION ONLY SIGNED AND SEALED
05 12 00	WELDING PROCEDURE SPECIFICATION	FOR INFORMATION ONLY
05 12 00	WELDING CERTIFICATES	FOR INFORMATION ONLY
05 12 00	MILL TEST CERTIFICATE OF STRUCTURAL STEEL	FOR INFORMATION ONLY
05 12 00	PAINT COMPATIBILITY CERTIFICATE	FOR INFORMATION ONLY
05 12 00	QUALIFICATION DATA	FOR INFORMATION ONLY
05 31 13	STEEL FLOOR DECK SHOP DRAWINGS	FOR APPROVAL
05 31 13	PRODUCT CERTIFICATE	FOR INFORMATION ONLY
05 31 13	PRODUCT TEST REPORT	FOR INFORMATION ONLY
05 31 23	STEEL ROOF DECK SHOP DRAWINGS	FOR APPROVAL
05 31 23	PRODUCT CERTIFICATE	FOR INFORMATION ONLY
05 31 23	PRODUCT TEST REPORT	FOR INFORMATION ONLY
05 40 00	COLD FORMED METAL STUD SHOP DRAWINGS	FOR APPROVAL
05 40 00	COLD FORMED METAL STUD CALCULATIONS	FOR INFORMATION OLY SIGNED AND SEALED
05 40 00	PRODUCT TEST REPORT	FOR INFORMATION ONLY
05 40 00	RESEARCH REPORT	FOR INFORMATION ONLY
05 40 00	WELDING CERTIFICATE	FOR INFORMATION ONLY
05 50 00	METAL FABRICATION LADDERS	FOR APPROVAL/ SIGNED AND SEALED
05 50 00	METAL FABRICATION TOILET PARTITION SUPPORT	FOR APPROVAL/ SIGNED AND SEALED
05 50 00	METAL FABRICATION MEDICAL EQUIPMENT SUPPORT	FOR APPROVAL/ SIGNED AND SEALED
06 10 00	PRODUCT DATA	FOR INFORMATION ONLY
06 10 00	FASTENER PATTERNS	FOR APPROVAL
06 10 00	MATERIAL CERTIFICATES	FOR INFORMATION ONLY
06 10 00	EVALUATION REPORTS	FOR INFORMATION ONLY
06 15 00	WOOD DECK PRODUCT DATA	FOR APPROVAL
06 15 00	WOOD DECK SHOP DRAWINGS	FOR APPROVAL
06 15 00	RESEARCH EVALUATION REPORTS	FOR INFORMATION ONLY
06 16 00	WOOD SHEATHING PRODUCT DATA	FOR APPROVAL
06 16 00	EVALUATION REPORTS	FOR INFORMATION ONLY
06 18 00	GLUE LAMINATED TIMBER SHOP DRAWINGS	FOR APPROVAL
06 18 00	GLUE LAMINATED TIMBER CONNECTIONS	FOR INFORMATION ONLY SIGNED AND SEALED
06 18 00	TIMBER PRODUCT DATA	FOR INFORMATION ONLY
06 18 00	RESEARCH EVALUATION REPORTS	FOR INFORMATION ONLY
31 63 29	PIER REINFORCEMENT SHOP DRAWINGS	FOR APPROVAL
31 63 29	PIER CONCRETE MIX DESIGN	FOR APPROVAL
31 63 29	PIER DRILLING LOG	FOR INFORMATION ONLY

DELEGATED STRUCTURAL DESIGN OF COMPONENTS:

- A STRUCTURAL COMPONENT IS AN INDIVIDUAL STRUCTURAL MEMBER DESIGNED TO BE PART OF A STRUCTURAL SYSTEM. A LIST OF STRUCTURAL COMPONENTS THAT ARE TO BE DESIGNED BY THE COMPONENT SUPPLIER'S ENGINEERS IS PROVIDED IN THESE PLANS AND SPECIFICATIONS.
- A COMPONENT'S DELEGATED ENGINEER AND RESPONSIBLE CHARGE, SHALL BE A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
- ALL DRAWINGS AND CALCULATIONS FOR COMPONENTS IN QUESTION, OR THEIR ASSEMBLY INTO STRUCTURAL SYSTEMS SHALL REQUIRE THE SEAL AND SIGNATURE OF THE DELEGATED ENGINEER WHO PREPARED THEM.
- THE DESIGN OF PRE-ENGINEERED SYSTEMS SPECIFIED IN THE CONTRACT DOCUMENTS WHICH ARE DESIGNED/ENGINEERED BY THE SYSTEM SUPPLIER IS THE SOLE RESPONSIBILITY OF THE SUPPLIER AND ITS DESIGN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMITTALS OF SUCH SYSTEMS TO THE STRUCTURAL ENGINEER OF RECORD SHALL BE REVIEWED FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO THE ARRANGEMENT AND/OR SIZES OF MEMBERS SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS AND THE SUPPLIER'S INTERPRETATION OF THE DESIGN INFORMATION INCLUDED IN THE CONTRACT DOCUMENTS. SUCH REVIEW BY THE STRUCTURAL ENGINEER OF RECORD SHALL NOT IMPLY ANY RESPONSIBILITY FOR THE ACTUAL DESIGN OF SUCH SYSTEMS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DIMENSIONAL ACCURACY AND CONFORMANCE WITH THE INFORMATION CONTAINED IN CONTRACT DOCUMENTS.
- SEE APPLICABLE SECTIONS OF GENERAL NOTES AND SPECIFICATIONS FOR THE APPROPRIATE DESIGN RESPONSIBILITIES OF THE SUPPLIER AND ITS LICENSED ENGINEER.

SPECIAL INSPECTIONS:

SPECIAL INSPECTION WORK AND THE FINAL LETTER OF COMPLIANCE HAVE NOT BEEN INCLUDED IN THE STRUCTURAL ENGINEER'S SCOPE OF SERVICES. THE OWNER SHALL BE RESPONSIBLE FOR OBTAINING THE SERVICES OF THE SPECIAL INSPECTOR AND THE TESTING LABORATORY. SPECIAL INSPECTIONS CAN BE PROVIDED BY AN INDEPENDENT SPECIAL INSPECTOR WHO IS APPROVED BY THE BUILDING AUTHORITY OR THE ENGINEER OF RECORD. THE SPECIAL INSPECTION WORK DOES NOT INCLUDE THE TESTING LABORATORY SERVICES AS CALLED FOR ON THE DRAWINGS. ARRANGEMENTS FOR SPECIAL INSPECTIONS SHOULD BE MADE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE OWNER IF SPECIAL INSPECTIONS ARE REQUIRED ON THE APPROVED PERMIT DRAWINGS AND FOR NOTIFYING THE TESTING LABORATORY AND SPECIAL INSPECTOR IN A TIMELY MANNER BEFORE CONSTRUCTION OPERATIONS CONTINUE. THE CONTRACTOR SHALL NOT PROCEED WITH ANY WORK REQUIRING INSPECTIONS WITHOUT THE TESTING LABORATORY'S OR SPECIAL INSPECTOR'S PRESENCE. THE STRUCTURAL ENGINEER WILL NOT PROVIDE A FINAL LETTER OF COMPLIANCE AFTER THE WORK IS COMPLETE UNLESS HE HAS REVIEWED ALL SPECIAL INSPECTIONS/TESTING LABORATORY TEST RESULTS.

CHAPTER 17 OF THE 2018 INTERNATIONAL BUILDING CODE, INCLUDING ANY LOCAL AMENDMENTS, REQUIRES SPECIAL INSPECTION ON THE FOLLOWING ITEMS:

- | | |
|--|--|
| SOILS (SECTION 1705.6) | CAST-IN-PLACE DEEP FOUNDATIONS (SECTION 1705.8) |
| CONCRETE CONSTRUCTION (SECTION 1705.3) | HELICAL PILE FOUNDATIONS (SECTION 1705.9) |
| STEEL (SECTION 1705.2) | FABRICATED ITEMS (SECTION 1705.10) |
| MASONRY (SECTION 1705.4) | SPECIAL INSPECTIONS FOR WIND RESISTANCE (SECTION 1705.11) |
| WOOD (SECTION 1705.5) | SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE (SECTION 1705.12) |
| DRIVEN DEEP FOUNDATIONS (SECTION 1705.7) | TESTING FOR SEISMIC RESISTANCE (SECTION 1705.13) |
| | SPECIAL INSPECTIONS FOR WINDSTORM CERTIFICATION PER TD |



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 JOHN R. KUBALA
 11713 98th Street
 Houston, TX 77055
 LEAD ENGINEER
 11713 98th Street

**GALVESTON ISD
 KERMIT COURVILLE STADIUM
 NEW PRESS BOX**

1429 27TH ST.
 GALVESTON, TX, 77550
 ISSUE FOR PROPOSAL



KEY PLAN

NORTH PLAN TRUE

DISCIPLINE
 STATE OF TEXAS
 JOHN R. KUBALA
 106120
 04/24/2024
Kubala Engineers
 F-23612

CLIENT GALVESTON ISD		
DATE 01/31/2024	PROJECT NUMBER 230063	
DRAWING HISTORY		
No.	Description	Date
1	ASI #1	04/24/2024

ISSUE FOR PROPOSAL

BUILDING NUMBER

GENERAL NOTES

S-012



GENERAL FOUNDATION NOTES:

1. THE FOUNDATION DESIGN AND SUBSURFACE INFORMATION IS BASED ON THE GEOTECHNICAL INVESTIGATION REPORT AS FOLLOWS:
 GEOTECHNICAL CONSULTANT: BRAUN INTERTEC
 GEOTECHNICAL REPORT NUMBER: B2208085
 GEOTECHNICAL REPORT LOCATION: (COURVILLE STADIUM) 1429 27TH STREET, GAVESTON, TEXAS 77550
 DATE OF REPORT: 08/22/2022
2. ALL RECOMMENDATIONS THEREIN THAT RELATE TO THE WORK SHOWN ON THESE DRAWINGS SHALL BE USED. FOR ANY CONFLICTS BETWEEN THE GEOTECHNICAL REPORT AND THESE DRAWINGS, THE GC SHALL ISSUE A REQUEST FOR INFORMATION/CLARIFICATION.
3. GEOTECHNICAL REPORT IS AVAILABLE TO THE GENERAL CONTRACTOR UPON REQUEST TO THE OWNER. THE INFORMATION INCLUDED THEREIN MAY BE USED BY THE GENERAL CONTRACTOR FOR HIS GENERAL INFORMATION ONLY. THE ARCHITECT AND ENGINEER WILL NOT BE RESPONSIBLE FOR THE ACCURACY OR RELIABILITY OF SUCH DATA THEREIN.
4. PREPARED GRADE AREA UNDER ALL BUILDING SLABS AND GRADE BEAMS SHALL BE COVERED WITH A 10 MIL WATER VAPOR BARRIER MEETING THE REQUIREMENTS OF ASTM E 1745 (LATEST EDITION), CLASS A OR BETTER WITH MAXIMUM WATER PERMEANCE OF 0.01 PERMS WHEN TESTED IN ACCORDANCE WITH ASTM E96. THE RETARDER BARRIERS SHALL BE INSTALLED AND LAPPED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM E1643 (LATEST EDITION). PENETRATIONS SHALL BE SEALED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
5. WHERE VOID FORMS ARE REQUIRED, THESE FORMS SHALL BE CAPABLE OF SUPPORTING THE WEIGHT OF THE WET CONCRETE AND REINFORCEMENT. AFTER PLACEMENT ON THE SUBGRADE, THE FORMS SHALL BE TAPED AT ALL JOINTS. 1/8" THICK HARDBOARD SHALL BE LAID OVER THE TOP OF THE VOID FORMS PRIOR TO PLACEMENT OF THE REINFORCEMENT. AT LARGE AREAS AS REQUIRED, VOID FORMS SHALL BE SECURED TO THE SUBGRADE PER MANUFACTURER'S RECOMMENDATIONS. VOID FORMS SHALL BE WAX COATED FOR MOISTURE PROTECTION, RECTANGULAR IN PROFILE, AND EQUAL TO THE WIDTH OF THE ADJACENT GRADE BEAM. SOIL RETAINERS ARE REQUIRED. GRADE BEAMS TO BE FORMED EACH SIDE.

EARTHWORK STRUCTURAL TESTING AND INSPECTIONS:

1. UNLESS NOTED OTHERWISE BY THE OWNER OR ARCHITECT, THE GENERAL CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER OR OTHER AUTHORIZED INSPECTOR DURING EARTHWORK OPERATIONS, AND KEEP A COMPETENT TRAINED TECHNICIAN ASSIGNED TO THE PROJECT ON SITE DURING OPERATIONS. SERVICES PROVIDED SHALL INCLUDE:
 - A. OBSERVE STRIPPING OPERATIONS AND EVALUATE THE REQUIRED STRIPPING DEPTH DURING THESE OPERATIONS.
 - B. OBSERVE PROOFROLLING OPERATIONS AFTER SITE STRIPPING AND DETERMINE IF ANY SOFT SPOTS NEED TO BE UNDERCUT TO FIRM SOILS, REPLACED WITH SELECT FILL AND RECOMPACTED.
 - C. VERIFY THAT THE SUBGRADE IS SCARIFIED, MOISTURE CONDITIONED, AND LIME STABILIZED (IF REQUIRED) PER SUBGRADE PREPARATION.
 - D. THE SELECT FILL PAD MATERIAL SHALL BE TESTED FOR UNIFORMITY AND A MOISTURE DENSITY CURVE SHALL BE ESTABLISHED. SELECT FILL MATERIAL SHALL BE AS INDICATED ON SUBGRADE PREPARATION.
 - E. VERIFY THAT THE SELECT FILL IS PLACED IN EIGHT INCH LOOSE LIFTS AND COMPACTED PER SUBGRADE PREPARATION.
 - F. SELECT FILL MATERIAL SHALL BE TESTED DURING PLACEMENT OF EACH LIFT FOR THE ATTERBERG LIMITS IN ACCORDANCE WITH ASTM D4318-08 METHOD B 'STANDARD TEST METHOD FOR LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS' TO VERIFY THAT THE SELECT FILL MATERIAL IS IN ACCORDANCE WITH THE ORIGINALLY APPROVED SELECT FILL MATERIAL. PROVIDE TESTS PER SUBGRADE PREPARATION.
 - G. PROVIDE FIELD MOISTURE TESTS FOR THE FULL DEPTH OF THE PAD PER SUBGRADE PREPARATION.
 - H. OBSERVE THE EXCAVATION DAILY AND ENSURE THAT THE CONTRACTOR MAINTAINS A CLEAN EXCAVATION THAT IS FREE OF WATER 100% OF THE TIME. CONTRACTOR SHALL PROVIDE PUMPS AS REQUIRED TO REMOVE WATER AT ALL TIMES.
 - J. OBSERVE GRADING OPERATIONS TO ENSURE THAT PROPER DRAINAGE AWAY FROM THE BUILDING PAD IS PROVIDED.

SUBGRADE PREPARATION:

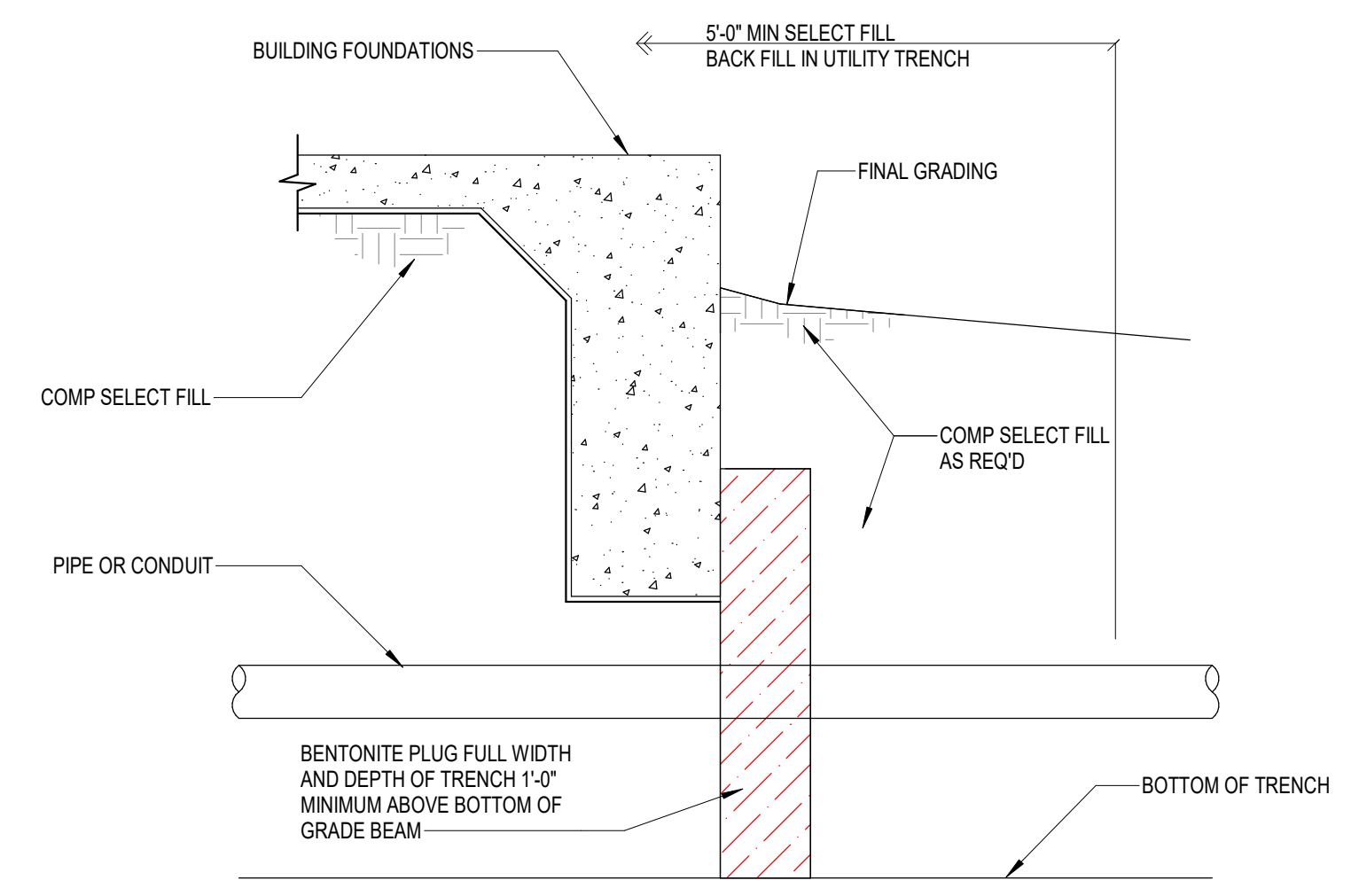
1. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND BE THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION GIVEN THEREIN. ALL SUBGRADE PREPARATION, FILL, FILL PLACEMENT, AND FOUNDATION CONSTRUCTION SHALL BE PERFORMED IN STRICT COMPLIANCE WITH THE STRUCTURAL DOCUMENTS AND THE GEOTECHNICAL REPORT, AND SHALL BE OBSERVED, TESTED, AND APPROVED BY THE PROJECT'S GEOTECHNICAL ENGINEER-OF-RECORD (GER) PRIOR TO PROCEEDING WITH FOUNDATION CONSTRUCTION.
2. SITE PREPARATION FOR THE BUILDING PAD SHALL CONSIST OF THE REMOVAL OF EXISTING PAVEMENT, TOPSOIL, VEGETATION, ORGANIC MATTER, AND OTHER DEBRIS/UNSATURABLE SURFACE MATERIAL AS NECESSARY TO PROVIDE THE REQUIRED AMOUNT OF FILL UNDER THE BUILDING AND EXTENDING OUT BEYOND THE PERIMETER OF THE BUILDING AS SHOWN IN THE SITE PREPARATION DETAIL. PROPER SITE DRAINAGE SHOULD BE MAINTAINED DURING CONSTRUCTION SO THAT PONDING OF SURFACE RUNOFF DOES NOT OCCUR AND CAUSE CONSTRUCTION DELAYS AND/OR INHIBIT SITE ACCESS.
3. THE SUBGRADE SHALL BE PROOFROLLED WITH A HEAVY RUBBER-TIRED VEHICLE (STATIC WEIGHT OF AT LEAST 20 TONS AND WITH TIRE PRESSURES OF AT LEAST 30 PSI). THE CONTRACTOR SHALL MAKE AT LEAST TWO COMPLETE PASSES OVER THE AREA WITH THE SECOND PASS PERPENDICULAR TO THE FIRST PASS. AREAS OF THE SUBGRADE THAT ARE OBSERVED TO BE SOFT OR WEAK SHALL BE OVEREXCAVATED AND REPLACED WITH PROPERLY COMPACTED SOILS EXHIBITING SIMILAR CLASSIFICATION, MOISTURE CONTENT, AND DENSITY AS THE ADJACENT IN-SITU SOILS. PROOFROLLING SHOULD BE PERFORMED UNDER THE DIRECT OBSERVATION OF THE GEOTECHNICAL ENGINEER OR HIS/HER REPRESENTATIVE.
4. PREPARE THE BUILDING SUBGRADE SOILS IN DIRECT ACCORDANCE WITH THE RECOMMENDATIONS LISTED IN THE 'EARTHWORK' AND 'FLOOR SLABS' SECTIONS OF THE GEOTECHNICAL REPORT. SUBGRADE SHALL BE SCARIFIED AND MOISTURE CONDITIONED TO A DEPTH OF SIX (6) INCHES WHERE NO EXISTING SELECT FILL IS PRESENT. WHERE EXISTING FILL IS PRESENT, THE FULL DEPTH OF FILL UP TO 2'-0" DEEP SHALL BE REMOVED AND REPLACED WITH PROPERLY COMPACTED AND MOISTURE CONDITIONED SELECT FILL AND THEN RECOMPACTED TO BETWEEN 95 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEST (ASTM D698). THE MOISTURE CONTENT SHALL BE BETWEEN OPTIMUM AND +4 PERCENT OF THE OPTIMUM MOISTURE CONTENT. PROVIDE A MINIMUM OF FOUR (4) FIELD DENSITY TESTS PER LIFT OR ONE (1) FOR EVERY 2,500 SQUARE FEET WHOEVER IS GREATER. IF WET WEATHER CONDITIONS ARE ENCOUNTERED AND WILL NOT ALLOW FOR ADEQUATE SUBGRADE COMPACTION EFFORTS, THEN THE SUBGRADE SHALL BE LIME STABILIZED WITH 3 PERCENT BY VOLUME HYDRATED LIME IN ACCORDANCE WITH THE LIME ASSOCIATION RECOMMENDATIONS. THE OPTIMUM LIME CONTENT SHALL BE DETERMINED AT THE TIME OF CONSTRUCTION BY A LIME SERIES TEST TO DETERMINE THE REQUIRED OPTIMUM LIME CONTENT NECESSARY TO ACHIEVE A pH OF 12.4 WHILE ACHIEVING A PLASTICITY INDEX OF LESS THAN 20. PRIOR TO PROCEEDING WITH THE SELECT FILL PLACEMENT, THE LIME STABILIZED SUBGRADE SHALL BE TESTED AND APPROVED BY THE TESTING LABORATORY.
5. THE FINAL BUILDING PAD SHALL BE TESTED AND APPROVED BY THE PROJECT'S GER PRIOR TO INSTALLATION OF ANY FOUNDATION AND/OR SLAB-ON-GRADE RELATED CONSTRUCTION. IF THE BUILDING PAD HAS BEEN INSTALLED MORE THAN TWO MONTHS PRIOR TO THE PLACEMENT OF THE VAPOR RETARDER, PROVIDE FIELD MOISTURE TESTS FOR THE FULL DEPTH OF THE PAD 96 HOURS PRIOR TO PLACEMENT OF THE WATER VAPOR BARRIER TO ENSURE THAT THE FILL MOISTURE CONTENT HAS BEEN MAINTAINED PRIOR TO CONCRETE PLACEMENT. MOISTURE CONTENTS SHALL BE TAKEN AT 12 INCH VERTICAL INTERVALS WITH A MINIMUM OF TWO TESTS PER BORING AT A RATE OF ONE (1) BORING FOR EVERY 2,500 SQUARE FEET OF PAD WITH A MAXIMUM OF TEN (10). IF THE PAD TESTS SHOW MOISTURE CONTENT OUT OF THE SPECIFIED RANGES, THE TOP SIX (6) INCHES TO ONE FOOT OF MATERIAL SHALL BE REMOVED, MOISTURE CONDITIONED AND RECOMPACTED TO BETWEEN 95 AND 100 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR DENSITY TEST (ASTM D698). THE MOISTURE CONTENT SHALL BE BETWEEN OPTIMUM AND +2 PERCENT OF THE OPTIMUM MOISTURE CONTENT FOR SELECT FILL.
6. CONTRACTOR SHALL MAINTAIN A CLEAN EXCAVATION THAT IS FREE OF WATER 100% OF THE TIME. CONTRACTOR SHALL PROVIDE PUMPS AS REQUIRED TO REMOVE ANY WATER AT ALL TIMES.
7. BACKFILL AGAINST THE OUTSIDE FACE OF THE GRADE BEAMS SHOULD CONSIST OF SELECT FILL USED TO PREPARE THE BUILDING PAD. THE SELECT FILL SHOULD BE UNIFORMLY COMPACTED TO AT LEAST 95 PERCENT OF THE STANDARD EFFORT (ASTM D 698) MAXIMUM DRY DENSITY AT A MOISTURE CONTENT WITHIN OPTIMUM AND +2% OF OPTIMUM MOISTURE CONTENT.
8. A MINIMUM OF 2.5 FT. OF SELECT FILL MATERIAL IS REQUIRED UNDER THE FLOOR SLAB, AND SHALL EXTEND A MINIMUM OF 5'-0" BEYOND THE BUILDING PERIMETER. THE FINAL EXTERIOR GRADE ADJACENT TO THE STRUCTURE SHOULD BE SLOPED TO PROMOTE EFFECTIVE DRAINAGE AWAY FROM THE STRUCTURE.
9. EXERCISE CARE IN PLACEMENT AND COMPACTION OF FILL IN ALL LEAVE-OUTS. ALL FILL SHALL BE PLACED IN LOOSE LIFTS AND COMPACTED AT ELEVATED MOISTURE CONTENT AS DETAILED IN THE PROJECT'S GEOTECHNICAL REPORT. ANY REINFORCING SHALL BE APPROPRIATELY MANIPULATED TO ENSURE PROPER COMPACTION. VAPOR RETARDER SHALL BE SUITABLY PLACED AND LAPPED.

SITE DRAINAGE:

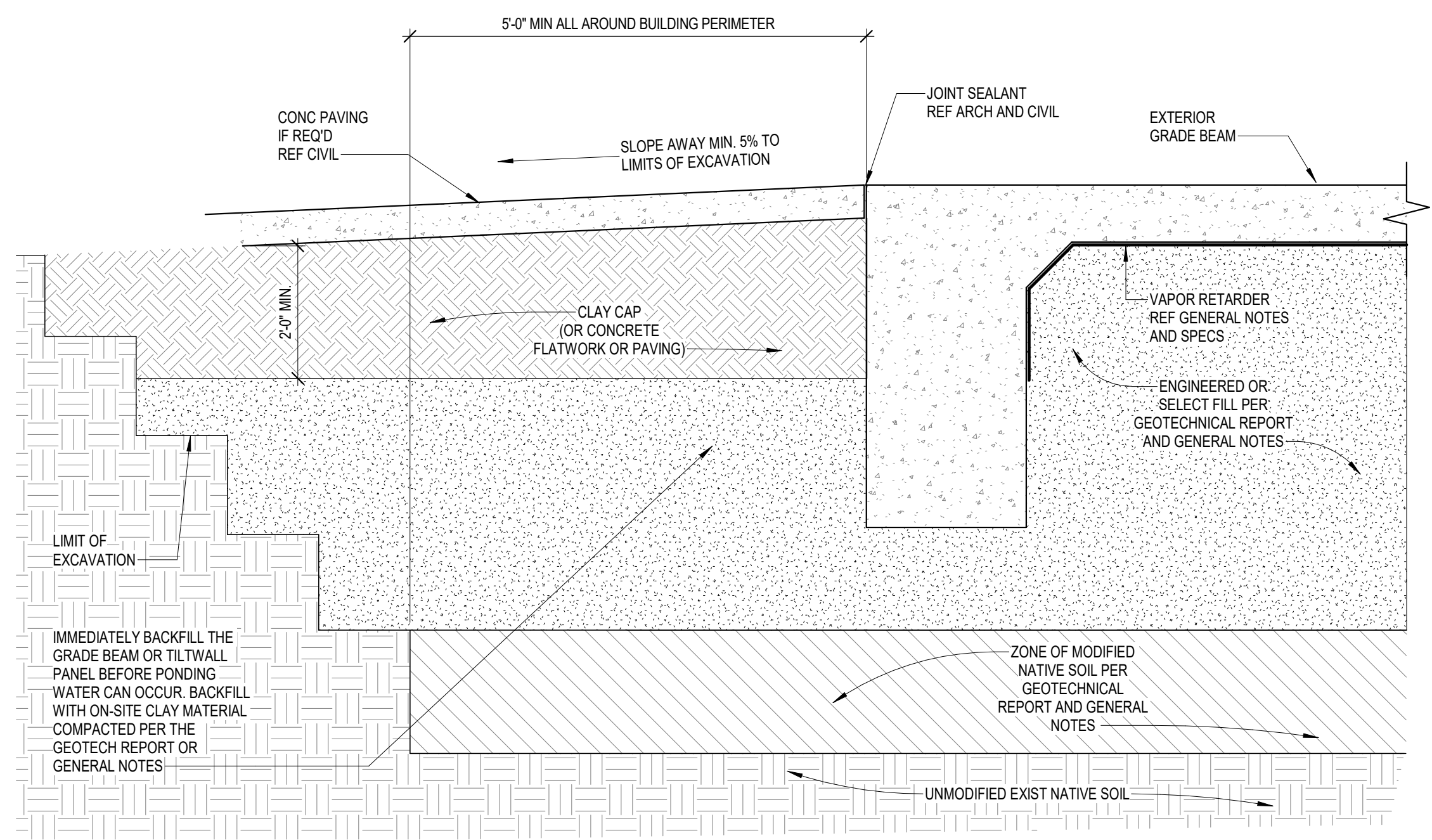
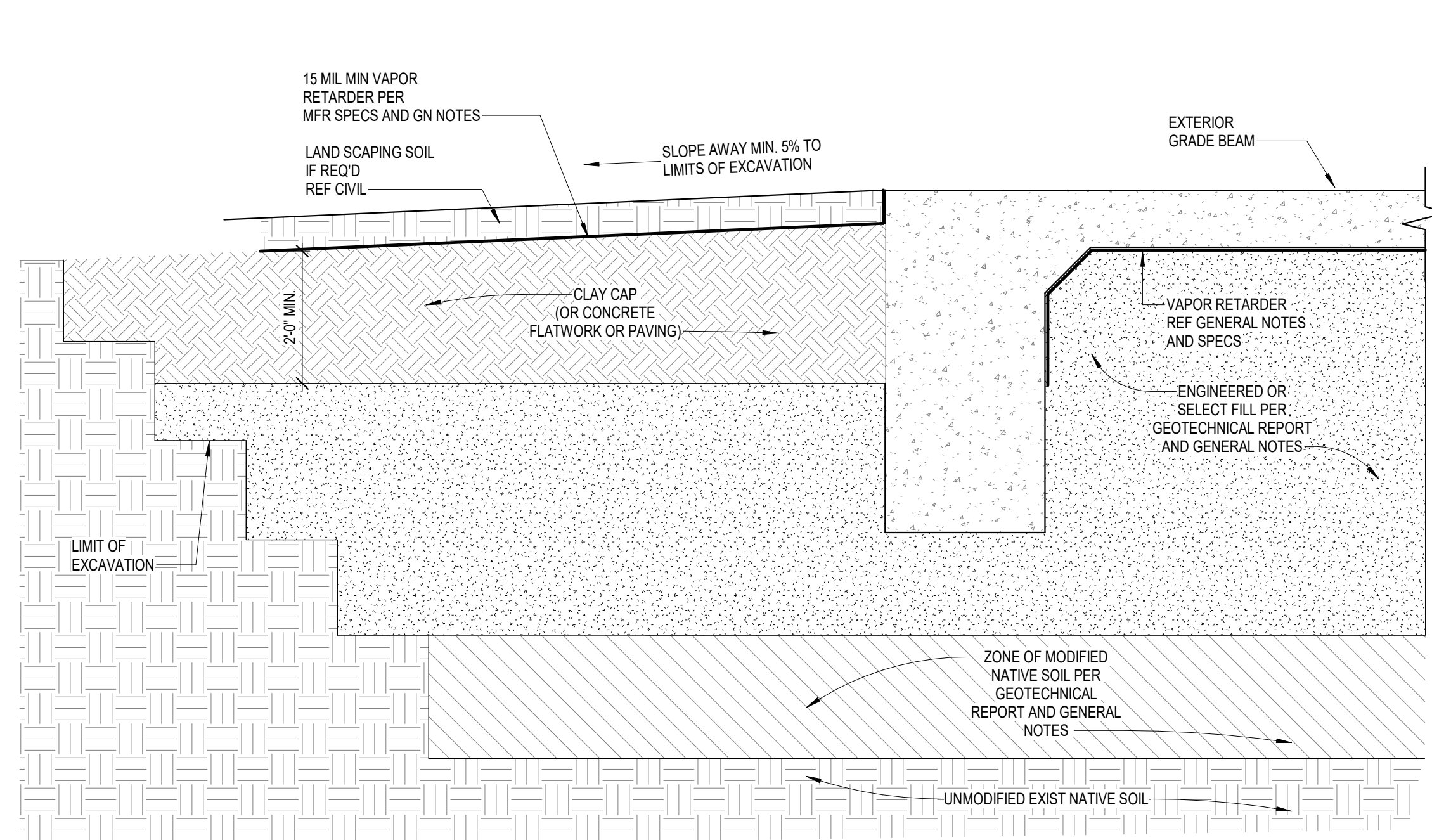
1. THE SITE SHALL BE GRADED TO PROVIDE POSITIVE DRAINAGE AWAY FROM ALL BUILDINGS AND SLABS. WATER SHALL NOT BE ALLOWED TO POND ADJACENT TO THE FOUNDATION.
2. WATER FROM ROOF DRAINS AND DOWNSPOUTS SHALL BE COLLECTED AND PIPED AWAY FROM THE BUILDING.
3. TREES AND VEGETATION SHALL NOT BE ALLOWED WITHIN A DISTANCE EQUAL TO THREE QUARTERS THEIR ULTIMATE HEIGHT AWAY FROM THE BUILDING.

NOTES:

1. PIPES OR CONDUITS SHALL BE PLACED IN THE SELECT FILL MATERIAL. ALL PENETRATIONS TO THE VAPOR BARRIER SHALL BE SEALED PER THE MANUFACTURER'S RECOMMENDATIONS
2. BACKFILL DIRECTLY AROUND PIPE SHALL BE CEMENT STABILIZED SAND (1-SACK MIX). THE REMAINDER OF THE TRENCH INFILL MAY BE SELECT FILL ONLY IF IT CAN BE COMPACTED AND TESTED BY THE TESTING LAB. OTHERWISE, THE GC SHALL USE CEMENT STABILIZED SAND



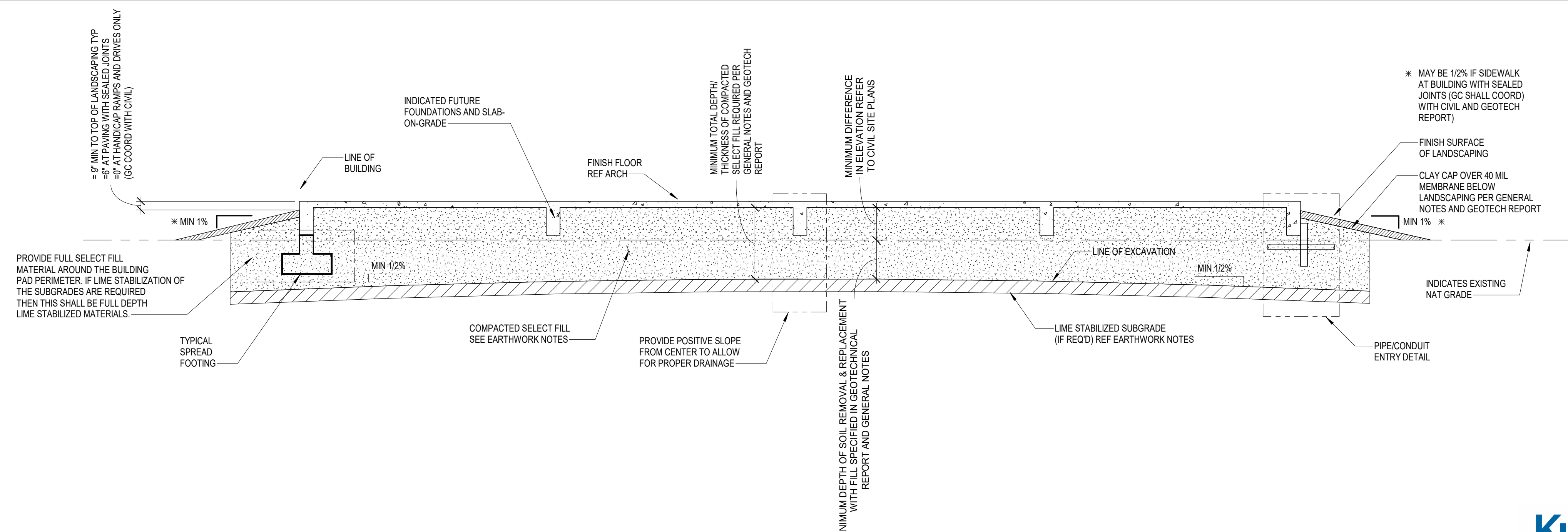
1 PIPE/CONDUIT ENTRY DETAIL
NO SCALE



SITE NOTES:

1. ROOF RUNOFF AND SURFACE DRAINAGE SHALL BE COLLECTED AND DISCHARGED AWAY FROM THE STRUCTURES TO PREVENT WETTING OF THE FOUNDATION. SOILS, ROOF GUTTERS SHALL BE INSTALLED AND CONNECTED TO DOWNSPOUTS AND PIPES DIRECTING ROOF RUNOFF AT LEAST 10 FEET AWAY FROM THE BUILDINGS, OR DISCHARGED ON TO POSITIVELY SLOPED IMPERVIOUS FLATWORK SUCH AS SIDEWALKS AND/OR PAVING AREAS.
2. SPRINKLER MAINS AND SPRAY HEADS SHALL BE LOCATED AT LEAST 5 FEET AWAY FROM THE BUILDING SUCH THAT THEY CANNOT BECOME A POTENTIAL POINT SOURCE OF WATER DIRECTLY ADJACENT TO THE BUILDING. WATERING OF VEGETATION SHALL BE PERFORMED IN A TIMELY AND CONTROLLED MANNER AND PROLONGER WATERINGS SHALL BE AVOIDED. LANDSCAPED IRRIGATION ADJACENT TO THE FOUNDATION STRUCTURE SHALL BE MINIMIZED OR ELIMINATED. SPECIAL CARE SHALL BE TAKEN SUCH THAT UNDERGROUND UTILITIES DO NOT DEVELOP LEAKS WITH TIME.

3 TYPICAL EXTERIOR GB BACKFILL DETAIL
NO SCALE



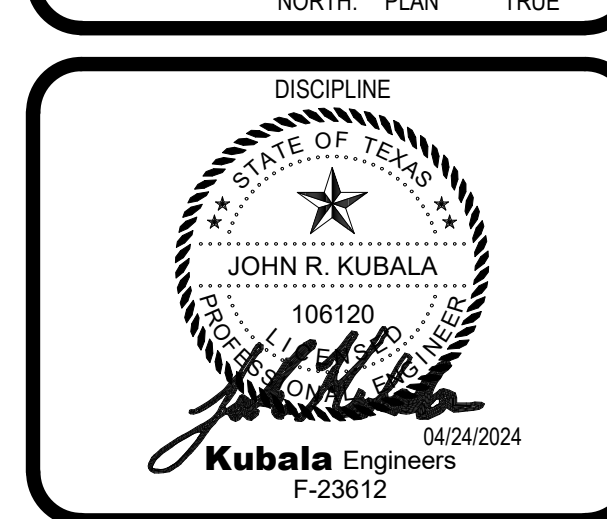
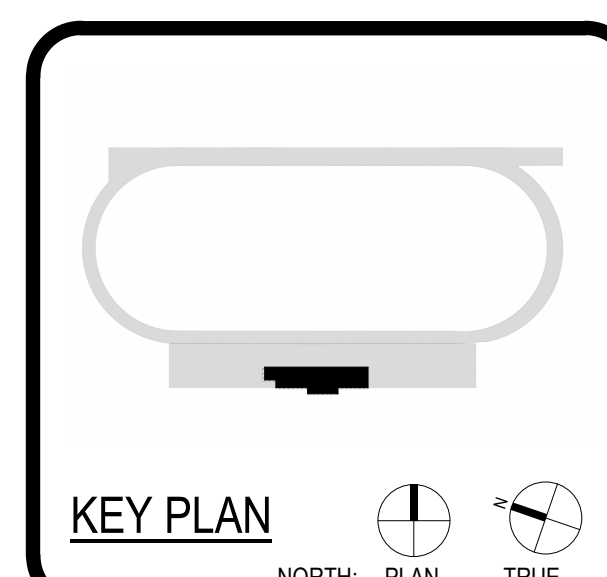
4 TYPICAL BUILDING PAD PREPARATION DETAIL
NO SCALE



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TX REG. NO. F-23612

GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX



CLIENT: GALVESTON ISD
DATE: 01/31/2024 PROJECT NUMBER: 230063
DRAWING HISTORY
No. Description Date
1 ASI #1 04/24/2024

ISSUE FOR PROPOSAL
BUILDING NUMBER

PRESS BOX FOUNDATION PLAN

S-101

FOUNDATION PLAN NOTES

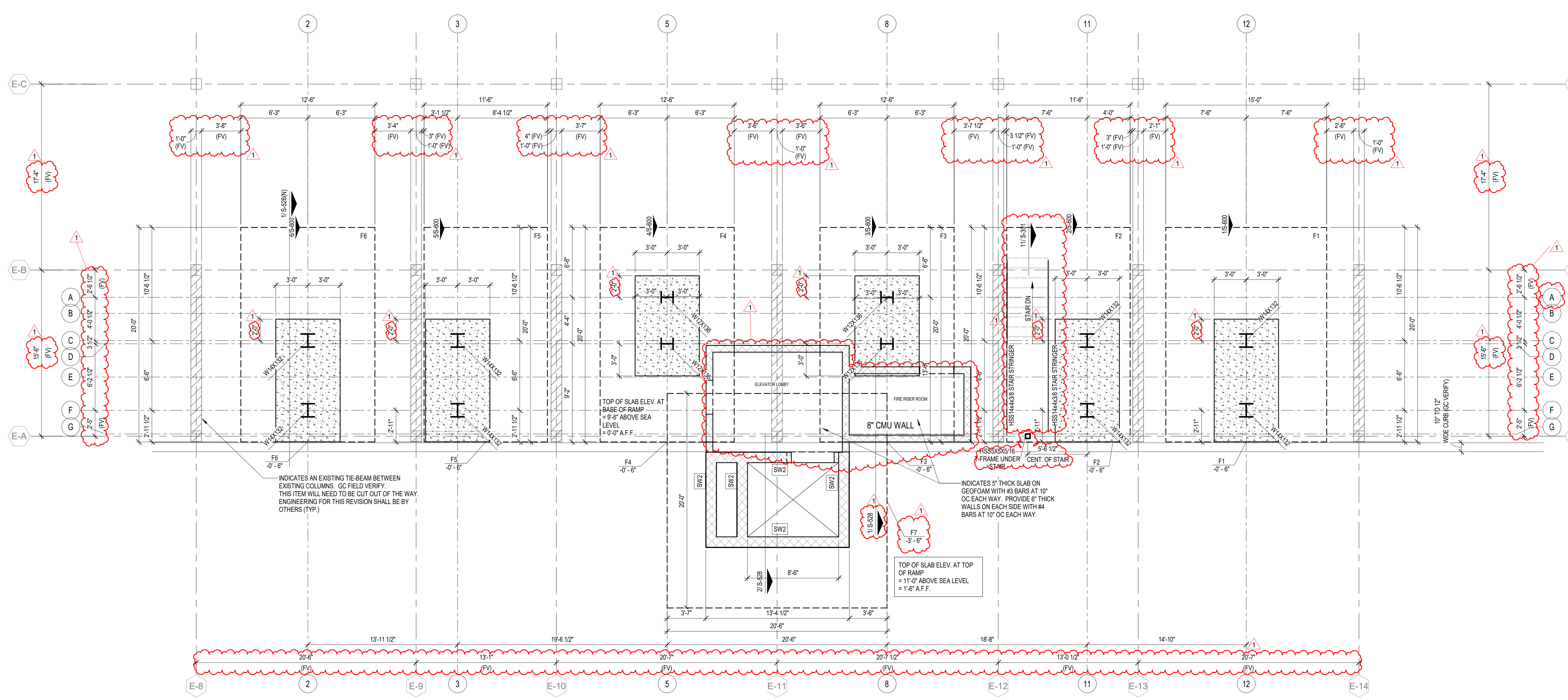
- REFER TO PLAN FOR TOP OF STRUCTURAL CONCRETE ELEVATIONS (TOSS CELL ALL ELEVATIONS SHOWN ON THE PLAN ARE BASED ON A LEVEL ONE REFERENCE ELEVATION = 0'-0". THIS REFERENCE ELEVATION IS EQUIVALENT TO THE LEVEL ONE MEAN SEA LEVEL ELEVATION + REF CIVIL SHOWN IN THE CIVIL AND ARCHITECTURAL DRAWINGS AND IS NOT INTENDED TO ESTABLISH THE ACTUAL SEA LEVEL ELEVATION OF ANY PORTION OF THE STRUCTURE.
- 5" THICK CONCRETE SLAB ON GRADE REINFORCED WITH #3@10" OC EACH WAY OR #4@12" OC EACH WAY. ON A 1/2" CHAIRS SPACED AT 30" OC EACH WAY. PLACE THE SLAB ON 15 MIL WATER VAPOR BARRIER OVER COMPACTED SELECT FILL (SOIL REPORT) FOR SLAB JOINT DETAILS REFER TO 13S-300 AND 2S-300.
- TOP OF INTERIOR/EXTERIOR PLINTH ELEVATION SHALL BE = -1'-0" UON. TOP OF INTERIOR PIER ELEVATION WITHOUT PLINTH SHALL BE = -1'-0" UON. TOP OF INTERIOR PIER ELEVATION WITH PLINTH SHALL BE = -3'-6" UON. TOP OF INTERIOR PIER ELEVATION WITH GRADE BEAM SHALL BE = -3'-6" UON. TOP OF EXTERIOR PIER ELEVATION SHALL BE = -3'-6" UON.
- REFER TO ARCHITECTURAL DRAWINGS FOR EXTENTS AND DIMENSIONS OF RAISED OR DEPRESSED SLAB AREAS, SLOPES, CURBS, AND DRAINS. REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS.
- GC COORDINATE ALL PENETRATIONS AND UNDERGROUND UTILITIES WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
- CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
- GC COORDINATE ALL SLAB EDGE DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
- PROVIDE (2) - #5 x 5'-0" LONG BAR IN SLAB AT RE-ENTRANT CORNERS, TYPICAL INCLUDING RE-ENTRANT CORNERS AROUND THE PERIMETER OF THE BUILDINGS, FLOOR RECESSES AND OPENINGS.
- GC COORDINATE ALL THE SIZE AND EXTENT OF ALL BRICK LEDGES SHOWN ON PLAN OR DETAILS WITH ARCHITECTURAL DRAWINGS.
- REF 50-xx SERIES DRAWINGS FOR GENERAL NOTES AND TYP DETAILS.
- REF 53-xx SERIES DRAWINGS FOR FOUNDATION AND SLAB-ON-GRADE DETAILS.
- REF 54-xx SERIES DRAWINGS FOR CMU DETAILS.
- REF 55-xx SERIES DRAWINGS FOR STEEL DETAILS.
- REF 56-xx SERIES DRAWINGS FOR STEEL BRACE ELEVATIONS AND DETAILS.
- REFER TO ARCH AND PLUMBING DWGS FOR THE SIZE, NUMBER AND LOCATION OF ALL THE TRENCHES, AND FLOOR DRAINS. REF 10S-301 FOR TRENCH DETAIL AND REF 4S-301 FOR FLOOR DRAIN DETAIL.
- AT INTERIOR CMU WALL LOCATIONS, WHERE THE GRADE BEAM IS NOT SHOWN, PROVIDE SLAB TURNDOWN PER DETAIL 4S-313 TYPICAL. AT INTERIOR GLAZING LOCATIONS, WHERE THE GRADE BEAM IS NOT SHOWN, PROVIDE DETAIL 13S-301 TYPICAL.
- PIERS/FOOTINGS WITHOUT CENTERLINES SHOWN ON PLANS, SECTIONS AND/OR DETAILS SHALL BE LOCATED AS FOLLOWS:
A. COLUMNS AND PILASTERS: CENTERLINE OF THE COLUMN.
B. GRADE BEAMS AND WALLS: CENTERLINE OF THE GRADE BEAM OR WALL.
C. ALONG THE LENGTH OF GRADE BEAMS AND WALLS: INTERMEDIATE PIERS/FOOTINGS SHALL BE SPACED EQUALLY BETWEEN PIERS/FOOTINGS THAT ARE DIMENSIONALLY SET ON PLAN OR AS NOTED ABOVE.
D. PIERS SUPPORTING SLABS ON CARTON FORMS, UNLESS NOTED OTHERWISE, PIERS NOT DIMENSIONED SHALL BE SPACED EQUALLY BETWEEN PIERS THAT ARE DIMENSIONALLY SET ON PLAN.
- GC TO COORDINATE THE LOCATION OF ALL CONC CURBS WITH ARCH DWGS. REF 7S-301 FOR DETAIL TYP.
- GC COORDINATE THE LOCATION OF ALL CANOPY COLUMNS WITH ARCH DRAWINGS.
- ALL CMU SHOWN ON PLAN ARE PART OF THE STRUCTURAL LATERAL FORCE RESISTING SYSTEM AND SUPPORTS GRAVITY LOADS. NON-STRUCTURAL CMU EXISTS IN OTHER LOCATION (REF ARCH DWGS). A 3" VERT CONTROL JOINT SHALL BE PROVIDED WHEREVER STRUCTURAL AND NON-STRUCTURAL CMU ABUT.

GC PLEASE NOTE:
1. WHERE CMU WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, FOR STRUCTURAL SHEARWALLS, THE WALLS SHALL BE 12" CMU MIN AT ELEVATOR TOWER AND 8" MIN FOR OTHER CONDITIONS. REINFORCE 12" WALLS WITH (1) - #6 BAR AT 8" OC STAGGERED EACH FACE, (1) #6 BAR PER CELL, STAGGERED ALONG EACH FACE, CELL TO CELL FOR FULL LENGTH OF WALL.
2. ALL CMU WALLS SHALL BE FULLY GROUTED FOR FULL HEIGHT.
ALL WALLS ARE USED FOR LATERAL SHEAR WALLS AND SHALL HAVE REINFORCED BOND BEAMS WITH (2) - #6 BARS RUNNING HORIZONTALLY AT 4'-0" OC VERTICALLY FOR FULL HEIGHT OF WALL. BOND BEAM BARS SHALL HAVE 180 DEGREE HOOK AT EA END AROUND THE LAST VERTICAL BAR (TYP).
ALL CMU WALL REINFORCING MAY BE STANDARD DEFORMED REINFORCEMENT BARS (GRADE 60KSI).
3. WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE, THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINTS FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GC PLEASE NOTE:
1. WHERE CAST IN PLACE CONCRETE WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, THE WALLS SHALL BE 12" THICK AND REINFORCED WITH (1) - #6 BAR AT 16" OC EACH FACE VERTICALLY @ #6 BARS EVERY 16" AND (2) - #5 HORIZONTAL BARS EACH FACE AT 8" OC. HORIZONTAL BARS SHALL HAVE A 180 DEGREE HOOK AT EACH END OF EACH WALL AND SHALL WRAP AROUND THE LAST VERTICAL BARS (TYP). ALL CAST-IN-PLACE WALL REINFORCING SHALL BE THE SAME CHROM-X #100 REINFORCING STEEL USED WITHIN THE BUILDING COLUMN CONCRETE MEETING THE REQUIREMENTS OF ASTM A1035-CM GRADE 100.
2. ALL CAST-IN-PLACE CONCRETE SHEARWALLS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF WALL (TYP).
3. WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE, THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINT FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GENERAL CONTRACTOR PLEASE NOTE:
ALL REINFORCING FOR THE CONCRETE COLUMNS SHALL UTILIZE SPECIALIZED CHROM-X #100 REINFORCING STEEL. THIS STEEL IS SPECIALLY FORMULATED TO HAVE HIGH CORROSION RESISTANCE IN SEA SPRAY AND MARINE ENVIRONMENTS. THIS REINFORCING SHALL HAVE A MINIMUM $f_y = 100KSI$ AND SHALL MEET ASTM 1035-C5.
(TYPICAL FOR ALL STRUCTURAL REINFORCED CONCRETE COLUMNS)
ALL CAST-IN-PLACE CONCRETE COLUMNS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF MEMBER (TYP).

HOT DIPPED GALVANIZED STEEL
PLEASE NOTE: ALL EXPOSED STRUCTURAL STEEL BOTH OUTSIDE OF THE BUILDING ENVELOPE AND WITHIN THE BUILDING BUT NOT WITHIN AIR CONDITIONED SPACE SHALL BE HOT DIP GALVANIZED. ANY FIELD WELDING SHALL BE REQUIRED TO BE COATED WITH 2 COATS OF A ZINC RICH RUST INHIBITING PAINT, COLOR TO BE SELECTED BY ARCH (SHERMAN WILLIAMS BRAND OR APPROVED EQUAL)



1 PRESS BOX FOUNDATION PLAN
1: 58



NON-COMPOSITE FLOOR DECK AND METAL ROOF DECK LEGEND

NON-COMPOSITE FLOOR DECK TYPES

FLOOR DECK TYPE 1
0.522 NON-COMPOSITE DECK WITH 4 1/2" NWC ON 9/16" STEEL DECK-22 GA (GALV) (500) - (5" TOTAL SLAB DEPTH). SLAB SHALL BE REINFORCED WITH WWR MIN 6#5-W2.9XW2.5, U.N.C.

METAL ROOF DECK TYPES

ROOF DECK TYPE 2
1.5 WR 20 GA (GALV.) MTL. DECK.

NOTES

- STEEL DECK SHALL BE PLACED WITH TWO-SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER-OF-RECORD.
- REF SLABS FORM DECK (AT FLOORS) GENERAL NOTES AND METAL DECK SCHEDULE ON SHEET S-504 FOR MORE INFORMATION REGARDING FLOOR DECK REQUIREMENTS.
- REF METAL ROOF DECKING GENERAL NOTES AND METAL DECK SCHEDULE ON SHEET S-504 FOR MORE INFORMATION REGARDING ROOF DECK REQUIREMENTS.

HOT DIPPED GALVANIZED STEEL

PLEASE NOTE: ALL EXPOSED STRUCTURAL STEEL BOTH OUTSIDE OF THE BUILDING ENVELOPE AND WITHIN THE BUILDING BUT NOT WITHIN AIR CONDITIONED SPACE SHALL BE HOT DIP GALVANIZED. ANY FIELD WELDING SHALL BE REQUIRED TO BE COATED WITH 2 COATS OF A ZINC RICH RUST INHIBITING PAINT, COLOR TO BE SELECTED BY ARCH (SHERMAN WILLIAMS BRAND OR APPROVED EQUAL)

GC PLEASE NOTE:

- WHERE CMU WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, FOR STRUCTURAL SHEARWALLS, THE WALLS SHALL BE 12" CMU MIN AT ELEVATOR TOWER AND 8" MIN FOR OTHER CONDITIONS. REINFORCE 12" WALLS WITH (1) #6 BAR AT 8" OC STAGGERED EACH FACE (1 #6 BAR PER CELL STAGGERED ALONG EACH FACE. CELL TO CELL FOR FULL LENGTH OF WALL).
- ALL CMU WALLS SHALL BE FULLY GROUVED FOR FULL HEIGHT. ALL WALLS ARE USED FOR LATERAL SHEAR WALLS AND SHALL HAVE REINFORCED BOND BEAMS WITH (2) #5 BARS RUNNING HORIZONTALLY AT 4'-0" OC VERTICALLY FOR FULL HEIGHT OF WALL. BOND BEAM BARS SHALL HAVE 180 DEGREE HOOK AT EA. END AROUND THE LAST VERTICAL BAR (TYP). ALL CMU WALL REINFORCING MAY BE STANDARD DEFORMED REINFORCEMENT BARS (GRADE 60KSI).
- WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINTS FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GC PLEASE NOTE:

- WHERE CAST IN PLACE CONCRETE WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, THE WALLS SHALL BE 12" THICK AND REINFORCED WITH (1) #6 BAR AT 18" OC EACH FACE VERTICALLY (2 #6 BARS EVERY 18" AND (2) #5 HORIZONTAL BARS EACH FACE AT 18" OC. HORIZONTAL BARS SHALL HAVE A 180 DEGREE HOOK AT EACH END OF EACH WALL AND SHALL WRAP AROUND THE LAST VERTICAL BARS (TYP). ALL CAST-IN-PLACE WALL REINFORCING SHALL BE THE SAME CHROM-X 9100 REINFORCING STEEL USED WITHIN THE BUILDING COLUMN CONCRETE MEETING THE REQUIREMENTS OF ASTM A1035-CM GRADE 100.
- ALL CAST-IN-PLACE CONCRETE SHEARWALLS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF WALL (TYP).
- WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINT FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GENERAL CONTRACTOR PLEASE NOTE: *****
ALL REINFORCING FOR THE CONCRETE COLUMNS SHALL UTILIZE SPECIALIZED CHROM-X 9100 REINFORCING STEEL. THIS STEEL IS SPECIALLY FORMULATED TO HAVE HIGH CORROSION RESISTANCE IN SEA SPRAY AND MARINE ENVIRONMENTS. THIS REINFORCING SHALL HAVE A MINIMUM F_y = 100KSI AND SHALL MEET ASTM 1035-CS (TYPICAL FOR ALL STRUCTURAL REINFORCED CONCRETE COLUMNS)
ALL CAST-IN-PLACE CONCRETE COLUMNS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF MEMBER (TYP.)

- COMPOSITE FLOOR PLAN NOTES**
- REF PLAN FOR TOP OF SLAB ELEVATION.
 - ALL ELEVATIONS ARE BASED ON TOSC EL = 0'-0"
 - STEEL DECK SHALL BE PLACED WITH A TWO-SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD.
 - COMPOSITE BEAM CONSTRUCTION IS UNSHORED.
 - UNLESS SHOWN OTHERWISE, STEEL BEAMS OR JOISTS ARE CENTERED ON AND EQUALLY SPACED BETWEEN COLUMN CENTERLINES.
 - TOS EL = BOTTOM OF DECK.
 - ALL STRUCTURAL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
 - REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
 - PROVIDE ADDITIONAL (5) #6 CONT. BARS IN COMPOSITE SLAB ALONG FULL PERIMETER OF SLAB AND AROUND OPENINGS. EXTEND BARS 10'-0" PAST CORNERS OF OPENINGS. PROVIDE STD HOOK AT TERMINATION ENDS. ALL LAPS IN REINFORCING SHALL BE WITH MECHANICAL COUPLERS OR 25'-0" LAP SPLICE (TYP).
 - CONTRACTOR TO VERIFY ALL SLAB EDGE DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
 - INDICATES WIND BRACE BEAM BOTTOM FLANGE BRACING PER 205-806 TYP ALONG ENTIRE GRID AT ALL BRACE FRAMES. AT LOCATIONS WHERE BEAM SPACING EXCEEDS 4'-0" OC REF 105-505.
 - REF 80-xx SERIES DRAWINGS FOR GENERAL NOTES AND TYP DETAILS.
 - REF 84-xx SERIES DRAWINGS FOR CMU DETAILS.
 - REF 85-xx SERIES DRAWINGS FOR STEEL FRAMING DETAILS.
 - REF 86-xx SERIES DRAWINGS FOR STEEL BRACE ELEVATIONS AND DETAILS.
 - ALL CMU SHOWN ON PLAN ARE PART OF THE STRUCTURAL LATERAL FORCE RESISTING SYSTEM AND SUPPORTS GRAVITY LOADS. NON-STRUCTURAL CMU EXISTS IN OTHER LOCATION (REF ARCH DWGS). A 3/8" VERT CONTROL JOINT SHALL BE PROVIDED WHEREVER STRUCTURAL AND NON-STRUCTURAL CMU ABUT.
 - INDICATES PLAN DIAGONAL STRUT W16x15. COPE WEB AT BEAMS AND OVERLAP FLANGE 4" WELD TO TOP FLANGE AND COLUMNS WITH 5/16" FILLET WELD 3-SIDES (TYP.)
 - GC SHALL COORDINATE LOCATION OF FOLDING PARTITIONS WITH ARCH DRAWINGS. REF DETAIL 4-1.
 - REF DETAIL 4-1 FOR ADDITIONAL REIN UNDER INTERIOR CMU WALLS PARALLEL TO DECK SPAN. GC TO COORDINATE THE LOCATION OF ALL CMU WALLS WITH ARCH DRAWINGS.

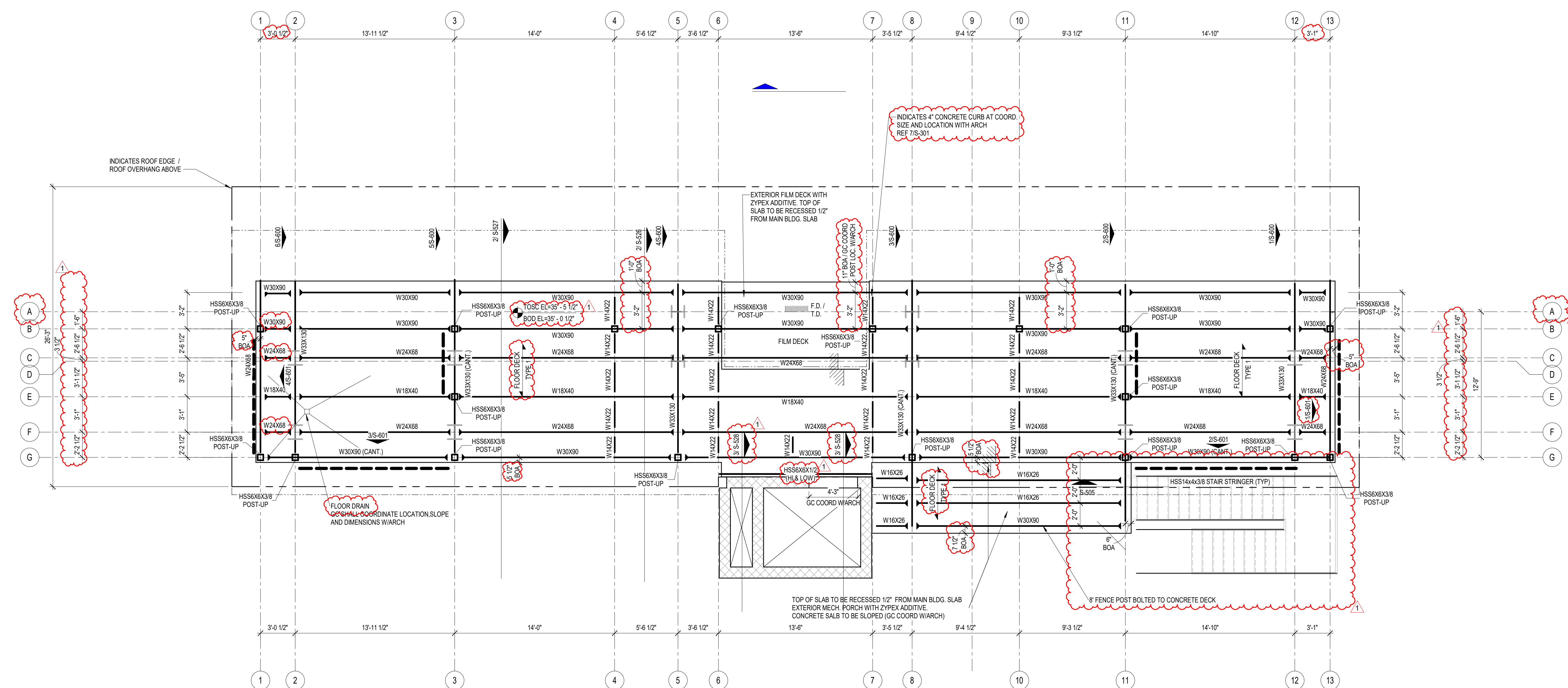


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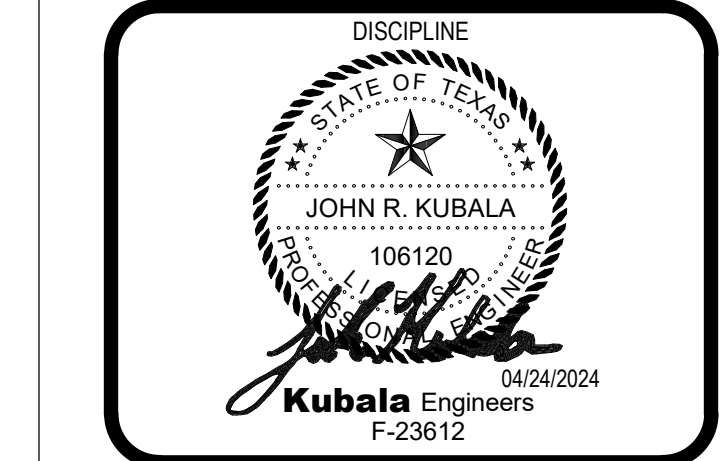
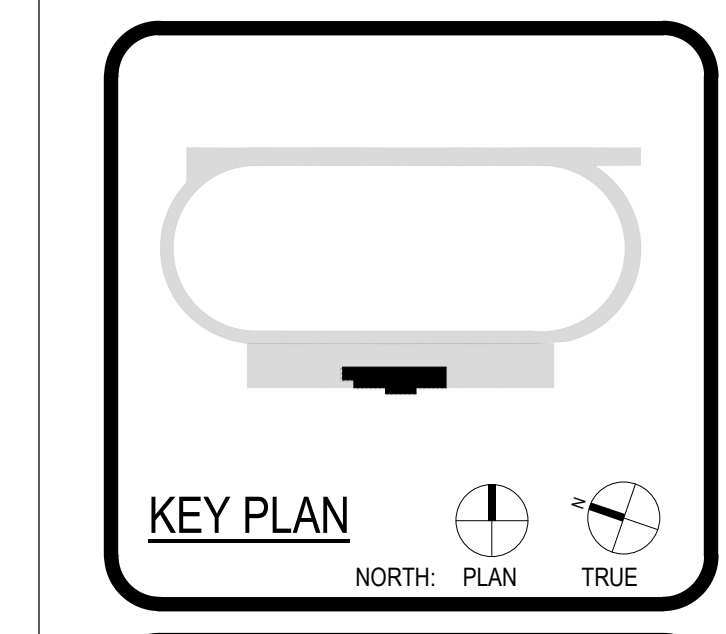
PROJECT
GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX

1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL



**GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX**

1429 27TH ST.
GALVESTON, TX, 77550
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CLIENT: GALVESTON ISD

DATE: 01/31/2024 PROJECT NUMBER: 230063

No.	Description	Date
1	AS#1	04/24/2024

ISSUE FOR PROPOSAL

BUILDING NUMBER

**PRESS BOX LEVEL 1
FRAMING PLAN**



NON-COMPOSITE FLOOR DECK AND METAL ROOF DECK LEGEND

NON-COMPOSITE FLOOR DECK TYPES

FLOOR DECK TYPE 1:
0.6222 NON-COMPOSITE DECK WITH 4 1/2" NWC ON #16 STEEL DECK-22 GA (GALV) (590) - (5" TOTAL SLAB DEPTH). SLAB SHALL BE REINFORCED WITH WWR MIN 6#6-W2.9M2.9, U.N.C.

METAL ROOF DECK TYPES

ROOF DECK TYPE 2:
1.5 WR20 GA (GALV.) MTL DECK.

NOTES:

- STEEL DECK SHALL BE PLACED WITH TWO-SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER-OF-RECORD.
- REF SLABS FORM DECK (AT FLOORS) GENERAL NOTES AND METAL DECK SCHEDULE ON SHEET S-504 FOR MORE INFORMATION REGARDING FLOOR DECK REQUIREMENTS.
- REF METAL ROOF DECKING GENERAL NOTES AND METAL DECK SCHEDULE ON SHEET S-504 FOR MORE INFORMATION REGARDING ROOF DECK REQUIREMENTS.

FLOOR & ROOF DECK TYPE LEGEND
12" = 1'-0"

HOT DIPPED GALVANIZED STEEL

PLEASE NOTE: ALL EXPOSED STRUCTURAL STEEL BOTH OUTSIDE OF THE BUILDING ENVELOPE AND WITHIN THE BUILDING BUT NOT WITHIN AIR CONDITIONED SPACE SHALL BE HOT DIP GALVANIZED. ANY FIELD WELDING SHALL BE REQUIRED TO BE COATED WITH 2 COATS OF A ZINC RICH RUST INHIBITING PAINT, COLOR TO BE SELECTED BY ARCH (SHERMAN WILLIAMS BRAND OR APPROVED EQUAL)

GC PLEASE NOTE:

- WHERE CMU WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, FOR STRUCTURAL SHEARWALLS, THE WALLS SHALL BE 12" CMU MIN AT ELEVATOR TOWER AND 8" MIN FOR OTHER CONDITIONS. REINFORCE 12" WALLS WITH (1) - #6 BAR AT 8" OC STAGGERED EACH FACE (1 #6 BAR PER CELL STAGGERED ALONG EACH FACE, CELL TO CELL FOR FULL LENGTH OF WALL).
- ALL CMU WALLS SHALL BE FULLY GROUTED FOR FULL HEIGHT. ALL WALLS ARE USED FOR LATERAL SHEAR WALLS AND SHALL HAVE REINFORCED BOND BEAMS WITH (2) - #6 BARS RUNNING HORIZONTALLY AT 4'-0" OC VERTICALLY FOR FULL HEIGHT OF WALL. BOND BEAM BARS SHALL HAVE 180 DEGREE HOOK AT EA. END AROUND THE LAST VERTICAL BAR (TYP). ALL CMU WALL REINFORCING MAY BE STANDARD DEFORMED REINFORCEMENT BARS (GRADE 60KS), (TYPICAL FOR ALL STRUCTURAL REINFORCED CONCRETE COLUMNS).
- WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINTS FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GC PLEASE NOTE:

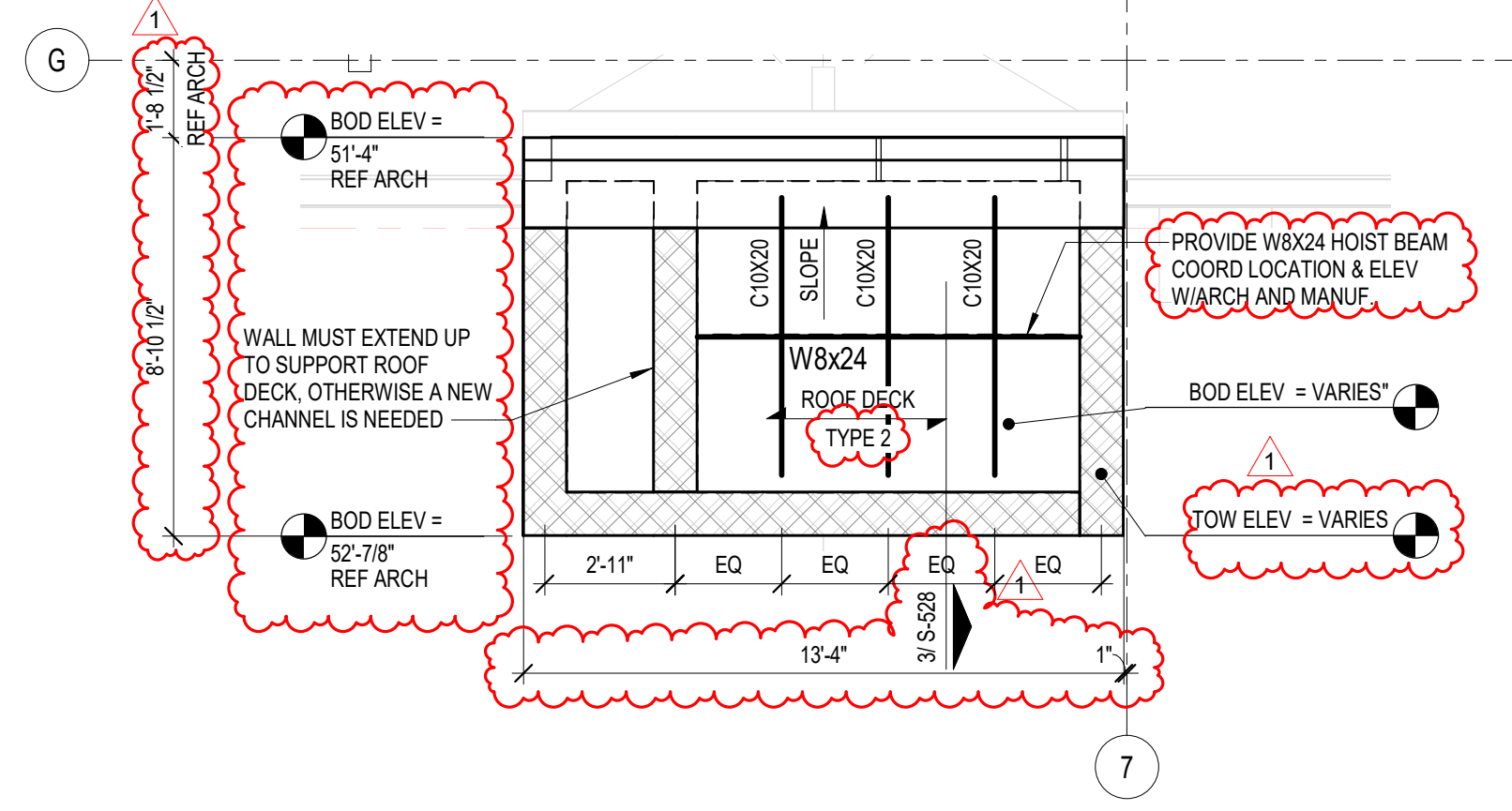
- WHERE CAST IN PLACE CONCRETE WALLS ARE USED, BOTH INSIDE AND OUTSIDE OF THE BUILDING, THE WALLS SHALL BE 12" THICK AND REINFORCED WITH (1) - #6 BAR AT 18" OC EACH FACE VERTICALLY (2 #6 BARS EVERY 18") AND (2) - #5 HORIZONTAL BARS EACH FACE AT 8" OC. HORIZONTAL BARS SHALL HAVE A 180 DEGREE HOOK AT EACH END OF EACH WALL AND SHALL WRAP AROUND THE LAST VERTICAL BARS (TYP). ALL CAST-IN-PLACE WALL REINFORCING SHALL BE THE SAME CHROM-X #100 REINFORCING STEEL USED WITHIN THE BUILDING COLUMN CONCRETE MEETING THE REQUIREMENTS OF ASTM A1035-CM GRADE 100.
- ALL CAST-IN-PLACE CONCRETE SHEARWALLS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF WALL (TYP).
- WHERE WALLS TRANSITION ACROSS A STEEL COLUMN OR TO ANOTHER WALL TYPE THE GC SHALL BE RESPONSIBLE FOR FLASHING AND WATERPROOFING OF THE JOINT FOR FLOOD WATERS UP TO 14'-0" DEEP AGAINST THE BUILDING (TYP).

GENERAL CONTRACTOR PLEASE NOTE:

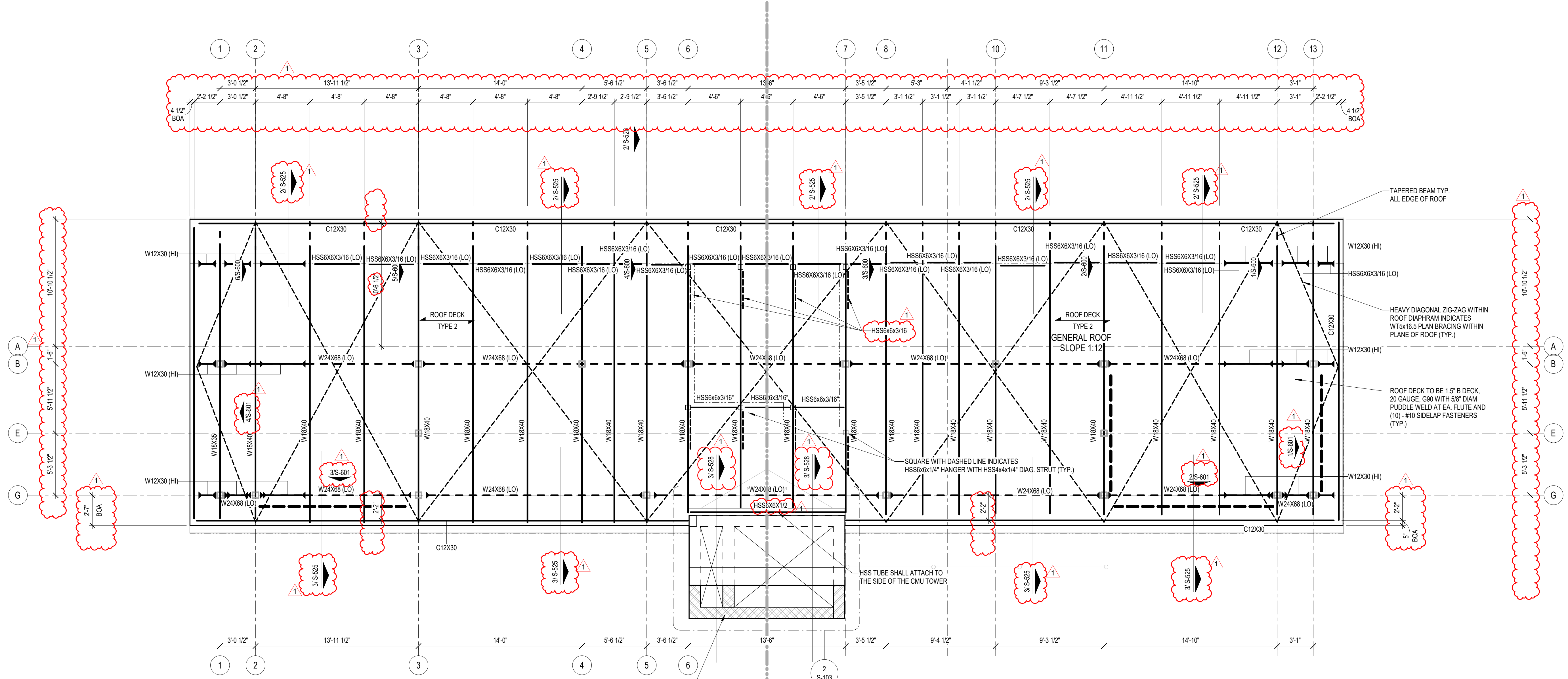
ALL REINFORCING FOR THE CONCRETE COLUMNS SHALL UTILIZE SPECIALIZED CHROM-X #100 REINFORCING STEEL. THIS STEEL IS SPECIALLY FORMULATED TO HAVE HIGH CORROSION RESISTANCE IN SEA SPRAY AND MARINE ENVIRONMENTS. THIS REINFORCING SHALL HAVE A MINIMUM Fy = 100ksi AND SHALL MEET ASTM 1035-C5 (TYPICAL FOR ALL STRUCTURAL REINFORCED CONCRETE COLUMNS)

ALL CAST-IN-PLACE CONCRETE COLUMNS SHALL ALSO HAVE THE XYPEX C-500 ADDITIVE FOR FULL HEIGHT OF MEMBER (TYP.)

- ROOF PLAN NOTES**
- REF PLAN FOR TOP OF SLAB ELEVATION.
 - ALL ELEVATIONS ARE BASED ON TOS EL = 0'-0".
 - TOP OF ROOF STRUCTURE IS SLOPED FOR DRAINAGE. REF ELEVATIONS NOTED ON PLAN. SLOPES SHALL BE UNIFORM BETWEEN COLUMN CENTERLINES UNLESS SHOWN OTHERWISE.
 - JOIST MFR NOTE: BRIDGING NOT SHOWN ON PLAN. MANUFACTURER SHALL PROVIDE STANDARD BRIDGING COMPLYING WITH SJI SPECIFICATIONS OF LATEST ADOPTION. TYPICAL FOR GRAVITY AND UPLIFT LOADS SUPERIMPOSED ON ALL JOISTS. DIAGONAL BRIDGING SHALL BE PROVIDED BETWEEN ADJACENT JOISTS WHENEVER BOTTOM CHORD HORIZONTAL BRIDGING IS CONTINUOUS.
 - REFER TO THE ROOF DIAPHRAGM CONNECTION SCHEDULE FOR DECK ATTACHMENT TO ROOF FRAMING MEMBERS AND DECK FASTENER TYPE, SIZE AND SPACING.
 - REF ARCH FOR TOP OF WALL ELEVATIONS.
 - DETAILING OF ALL MEMBER CONNECTIONS TO THE SUPPORTS SHALL BE PERFORMED TO SATISFY LATEST OSHA ERECTION REQUIREMENTS.
 - TOS EL = BOTTOM OF DECK.
 - UNLESS SHOWN OTHERWISE, STEEL BEAMS OR JOISTS ARE CENTERED ON AND EQUALLY SPACED BETWEEN COLUMN CENTERLINES.
 - ALL STRUCTURAL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
 - GC COORDINATE ALL PENETRATIONS AND UNDERGROUND UTILITIES WITH MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
 - CONTRACTOR TO VERIFY ALL SLAB EDGE DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
 - INDICATES WIND BRACE BEAM BOTTOM FLANGE BRACING PER 405-607, TYP ALONG ENTIRE GRID AT ALL BRACE FRAMES. NOT SHOWN IN PLAN FOR CLARITY.
 - REF DETAIL 128-509 WHERE ROOF TOP EQUIPMENT SHALL BE DESIGNED FOR AN ADDITIONAL CONCENTRATED DEAD LOAD (AS SHOWN ON PLAN AT ANY POINT ALONG JOIST SPAN). GC SHALL COORDINATE WITH MEP, ARCHITECTURE AND EQUIPMENT CUTSHEETS FOR FINAL WEIGHT, DIMENSIONS, LOCATION, ETC.
 - INDICATES PIPING RUN, REF MEP DRAWINGS; GC SHALL COORDINATE GIVEN LOAD WITH JOIST MANUFACTURER. REF SHEET S-011.
 - REF 80.xx SERIES DRAWINGS FOR GENERAL NOTES AND TYP DETAILS.
 - REF 84.xx SERIES DRAWINGS FOR CMU DETAILS.
 - REF 85.xx SERIES DRAWINGS FOR STEEL FRAMING DETAILS.
 - REF 86.xx SERIES DRAWINGS FOR STEEL BRACE ELEVATIONS AND DETAILS.
 - INDICATES COLUMN BRACING PER 405-607, TYPICAL AT COLUMNS UNBRACED BY FRAMING MEMBER ABOUT THEIR LOCAL AXIS. NOT ALL SHOWN ON PLAN FOR CLARITY.
 - INDICATES PLAN DIAGONAL STRUT WTSx15. COPE WEB AT BEAMS AND OVERLAP FLANGE 4". WELD TO TOP FLANGE AND COLUMNS WITH 5/16" FILLET WELD 3-SIDES (TYP.)
 - UPLIFT X-BRIDGING NOT SHOWN ON PLANS FOR CLARITY. GC SHALL PROVIDE X-BRIDGING BETWEEN ADJACENT WF BEAMS AT ROOF PER DETAIL 118-505
 - GC SHALL COORDINATE LOCATION OF FOLDING PARTITIONS WITH ARCH DRAWINGS. JOISTS ABOVE FOLDING PARTITION SHALL BE DESIGNED FOR ADD'L DL = 3 X X K (PERP) OR DL = 0.3 KLF (PARALLEL) (DEF. FROM DL SHALL NOT EXCEED 30"). END JOISTS ABOVE FOLDING PARTITION SHALL BE DESIGNED FOR STACKED END LOAD EQUAL TO 8000 POUNDS (DEF. FROM DL SHALL NOT EXCEED 30"). REF DETAIL 118-508.



2 ELEV SHAFT ROOF FRAMING PLAN
1/4" = 1'-0"



INDICATES 12" CMU BEING USED AS LOAD-BEARING AND SHEARWALLS. CMU SHALL BE REINFORCED WITH (1) - #7 VERTICAL BAR AT 8" OC STAGGERED BETWEEN EACH FACE. PROVIDE (2) - #5 CONT. BARS IN BOND BEAMS AT 4'-0" OC VERTICALLY. HORIZONTAL BARS SHALL TERMINATE IN A STANDARD 90 DEGREE HOOK OR 180 DEGREE HOOK TO HOOK AROUND VERTICAL BARS AT EA. END OF EACH WALL SEGMENT. PROVIDE (2) - #6 HOOKED DOWELS AT EACH BOND BEAM AT EACH CORNER (TYP.)

1 PRESS BOX ROOF FRAMING PLAN
1 : 56



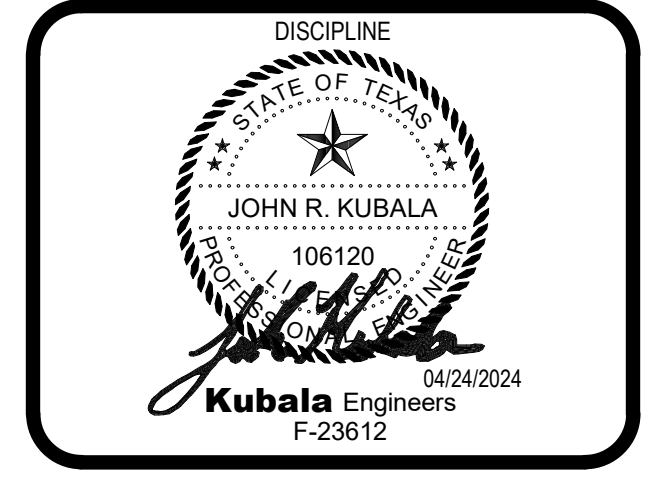
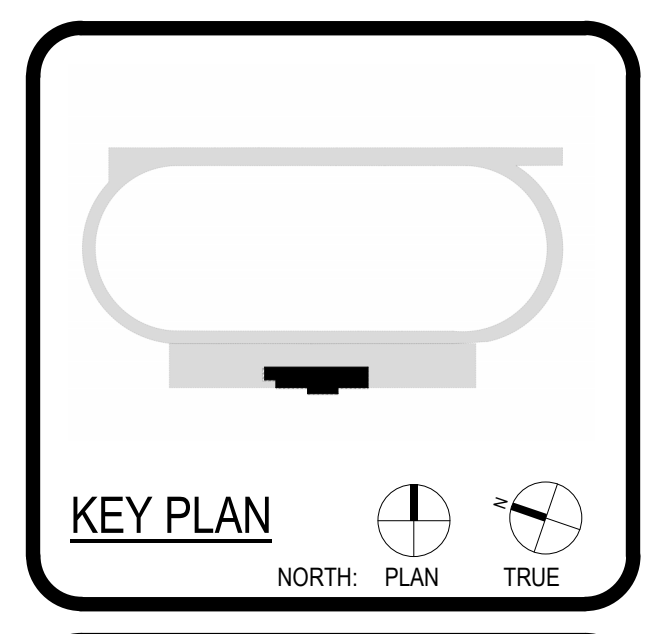
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**GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX**

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GALVESTON, TX, 77550

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CLIENT: GALVESTON ISD

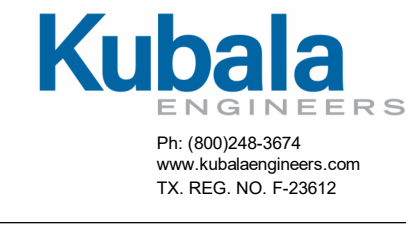
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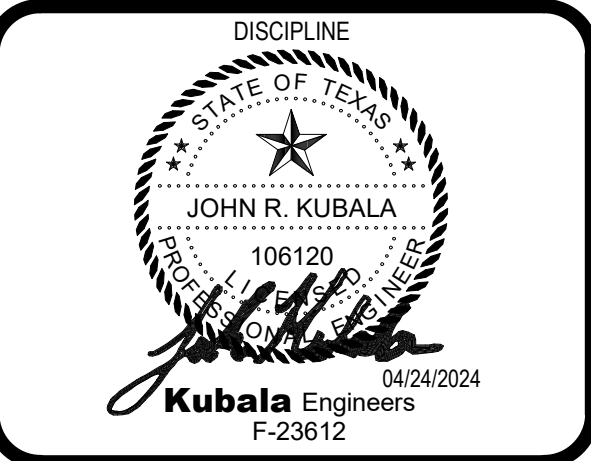
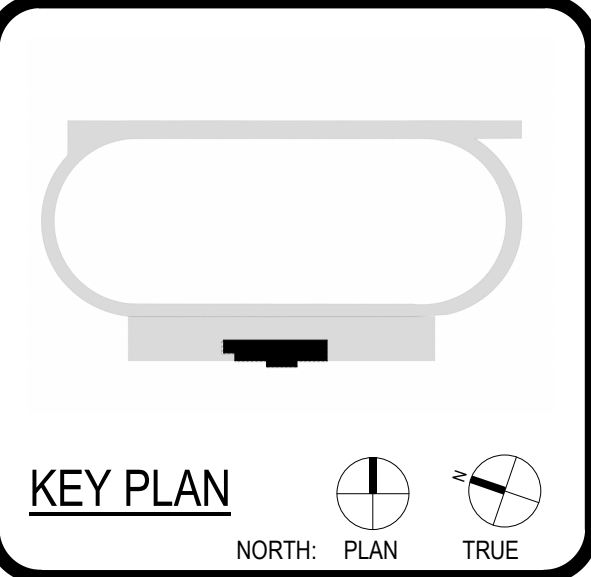
No.	Description	Date
1	AS#1	04/24/2024

ISSUE FOR PROPOSAL

BUILDING NUMBER

PRESS BOX ROOF FRAMING PLAN

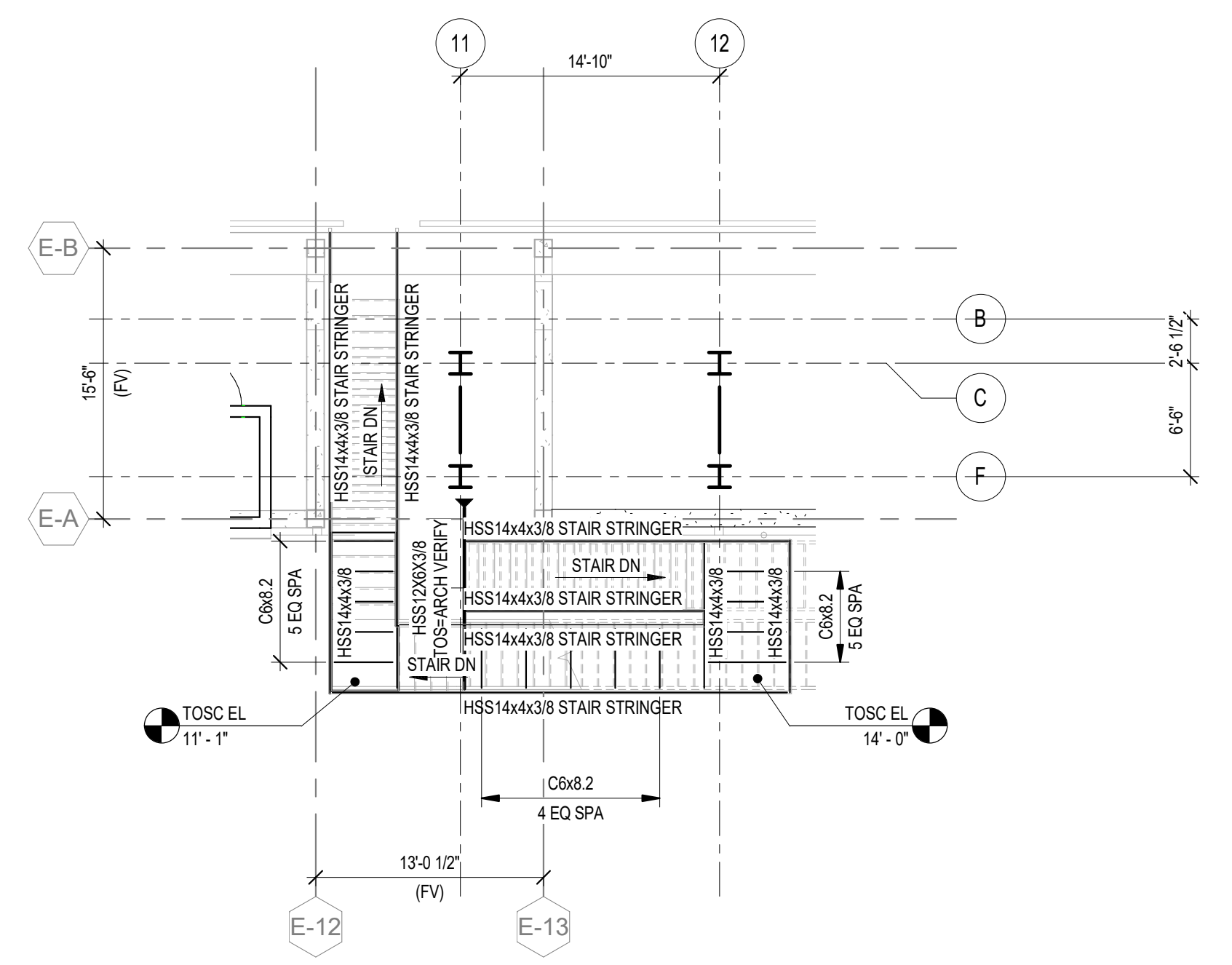




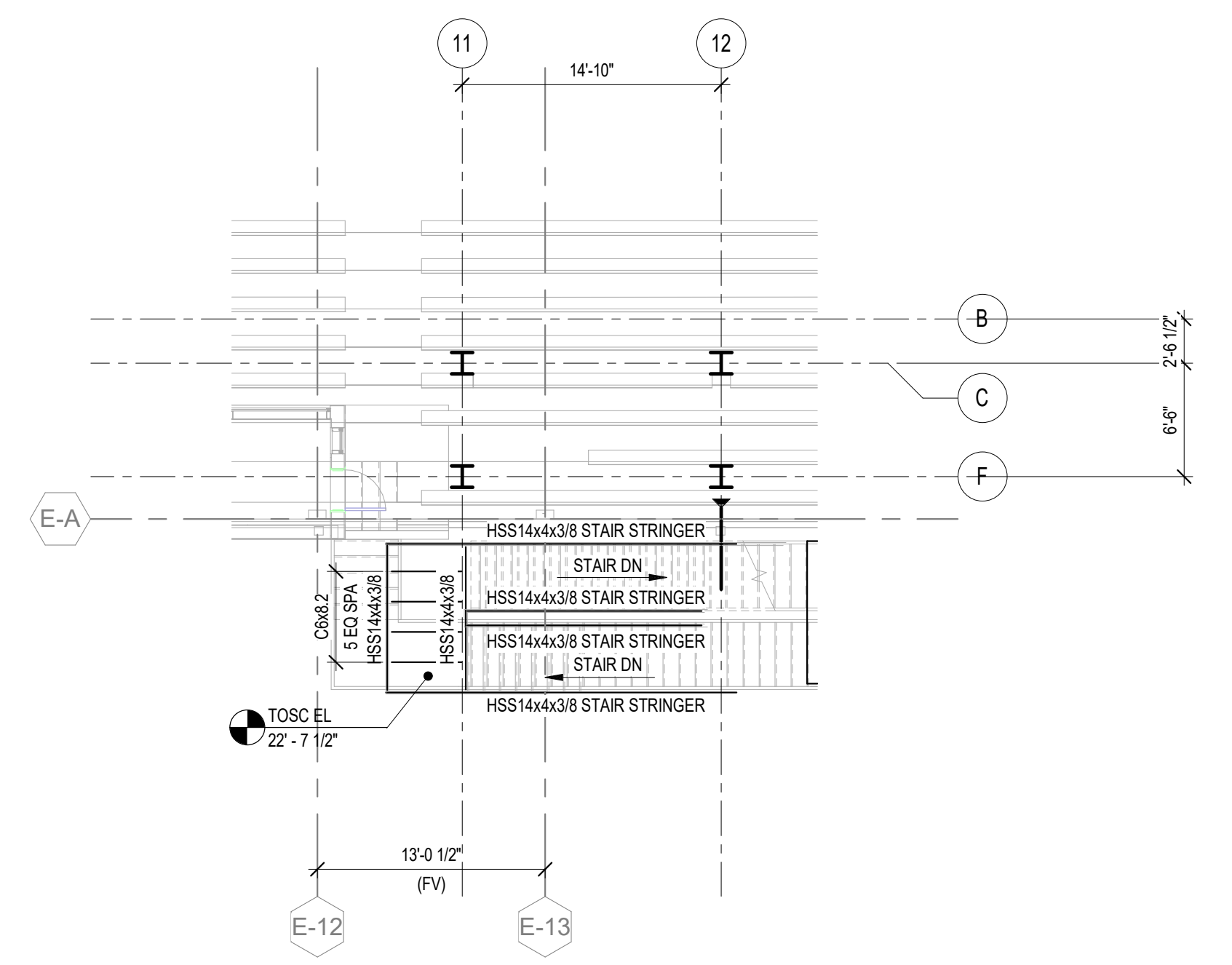
CLIENT: GALVESTON ISD		
DATE: 01/31/2024	PROJECT NUMBER: 230063	
DRAWING HISTORY		
No.	Description	Date
1	ASI #1	04/24/2024

ISSUE FOR PROPOSAL
 BUILDING NUMBER

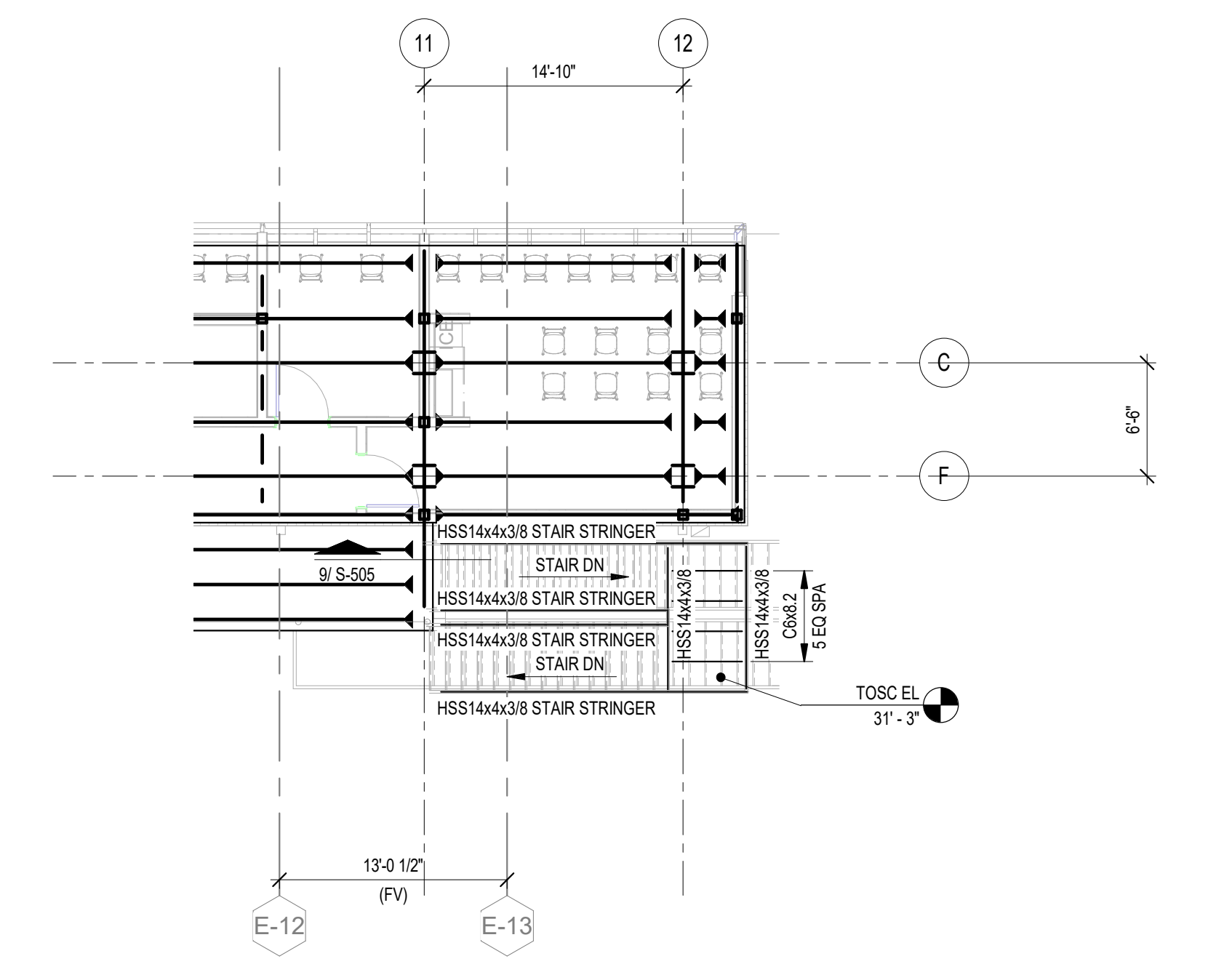
STAIR FRAMING PLANS
S-104



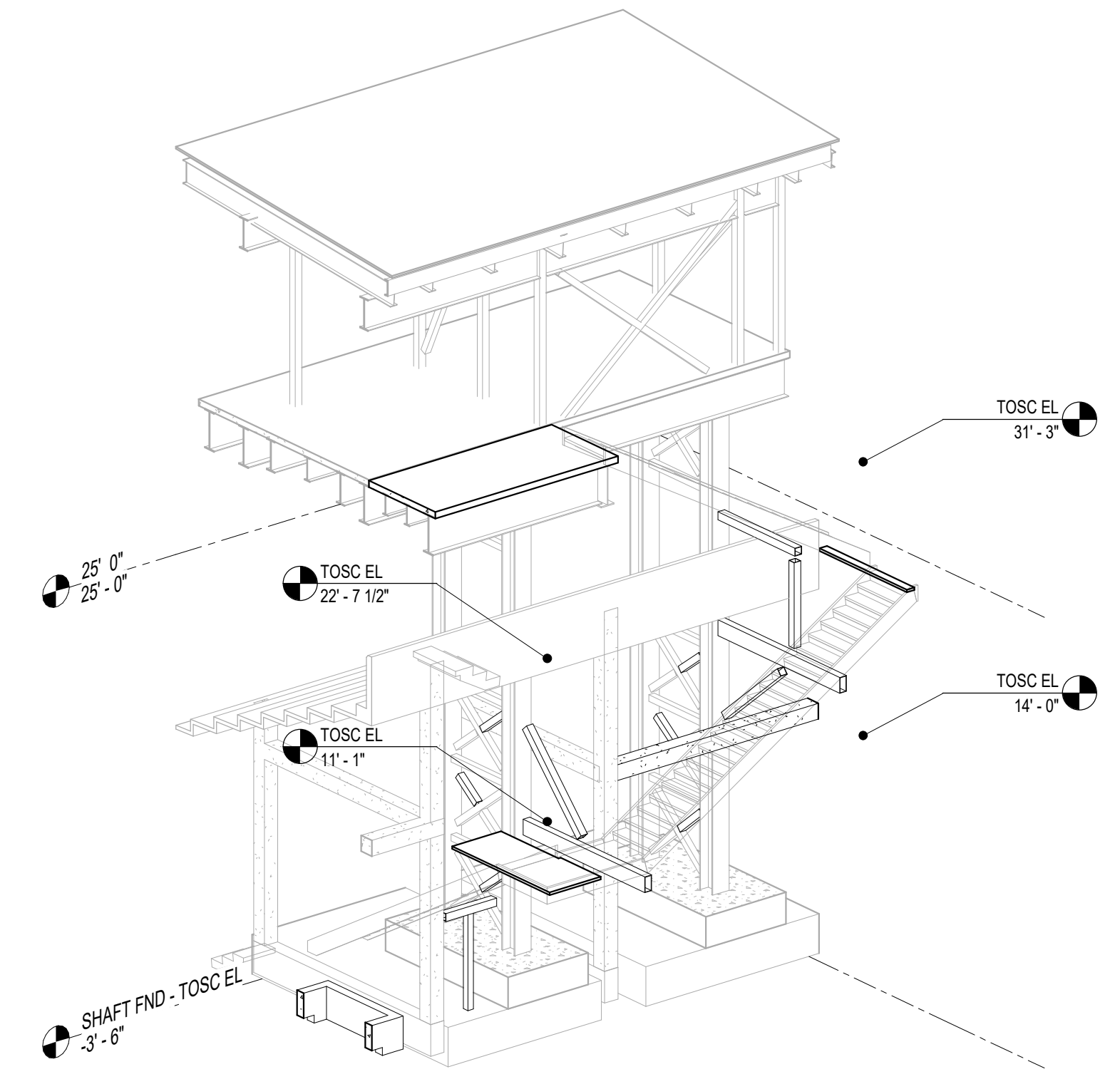
1 STAIR FRAMING PLAN
 1/8" = 1'-0"



2 STAIR FRAMING PLAN
 1/8" = 1'-0"



3 STAIR FRAMING PLAN
 1/8" = 1'-0"



4 STAIR 3D VIEW

CONCRETE MIX:

1. CONCRETE SPECIFICATIONS SHALL BE AS FOLLOWS:

Table with columns: USAGE, 28 DAY STRENGTH (PSI), MAX AGGREGATE SIZE (IN), SLUMP (IN), MAX W/C RATIO, MAX CURE DENSITY (PCF), CEMENT TYPE, MAX ALLOWABLE % FLY ASH (REF CMS), % AIR ENTRAINMENT. Rows include Footings, Slab-on-Grade, Grade Beams and Plinths, Columns, Beams, Walls, Basement Walls, Normal Weight Slab on Non-Composite Metal Deck, Slab for Equipment Pads, Non-Composite Topping Slab.

- ** SPECIAL GC NOTE FOR CONCRETE WHICH IS PART OF A DEPRESSED AREA. SEE CONCRETE MIX NOTE 10.
2. PORTLAND CEMENT SHALL BE TYPE I OR TYPE II (CONFORM TO ASTM C150), EXCEPT AS FOLLOWS:
MASS CONCRETE ONLY CEMENT TYPE II
NORMAL WEIGHT AGGREGATE SHALL CONFORM TO ASTM C33. LIGHT WEIGHT AGGREGATE SHALL CONFORM TO ASTM C330. ALL AGGREGATE SHALL BE FROM A SINGLE SOURCE.
3. FLY ASH WILL NOT BE PERMITTED IN ARCHITECTURALLY EXPOSED CONCRETE. FLY ASH MAY BE USED ELSEWHERE, WITHIN THE SPECIFIED PROPORTION LIMITS, BUT THE CONTRACTOR SHALL FIRST VERIFY COMPATIBILITY WITH CURING COMPOUNDS, SEALERS, BOND BREAKER, FLOORING ADHESIVES AND OTHER MATERIALS PROPOSED TO BE IN CONTACT WITH THE CONCRETE.
4. CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR REVIEW A MINIMUM OF 7 DAYS PRIOR TO THE START OF THE WORK FOR ENGINEER AND OWNERS TESTING LABORATORY APPROVAL, PRIOR TO THE PLACEMENT OF CONCRETE. MIX DESIGNS MUST INDICATE CONFORMANCE WITH ACI 318-LATEST EDITION, CHAPTER 5, SECTION 5.3.
5. AT THE POINT OF DISCHARGE SLUMP TESTS, CONFORMING TO ASTM C143, SHALL BE TAKEN. SEE CONCRETE NOTE NO. 5 BELOW FOR RATE OF TESTS.
6. AIR CONTENT TESTS CONFORMING TO ASTM C173 (VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE; ASTM C231 PRESSURE METHOD FOR NORMAL WEIGHT CONCRETE) SHALL BE TAKEN FOR EACH DAY'S POUR FOR ALL TYPES OF AIR-ENTRAINED CONCRETE BEING USED.
7. CONCRETE TEMPERATURE SHALL BE TESTED HOURLY WHEN THE AIR TEMPERATURE IS 40 DEG F AND BELOW, 80 DEG F AND ABOVE AND EACH TIME A SET OF COMPRESSION TEST SPECIMENS ARE MADE.
8. ONE SET OF FOUR COMPRESSION TEST SPECIMENS CONFORMING TO ASTM C31 SHALL BE MOLDED AND STORED FOR LABORATORY-CURED SPECIMENS. COMPRESSIVE STRENGTH TESTS SHALL CONFORM TO ASTM C39 AND SHALL CONSIST OF ONE SET FOR EACH DAY'S POUR EXCEEDING 5 CU YDS. PLUS ADDITIONAL SETS FOR EACH 50 CU YDS. MORE THAN THE FIRST 25 CU YDS OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY. ONE SPECIMEN SHALL BE TESTED AT 7 DAYS, TWO SPECIMENS SHALL BE TESTED AT 28 DAYS, AND ONE SPECIMEN SHALL BE RETAINED FOR LATER TESTING AS REQUIRED.
9. VERIFY THAT POST INSTALLED ANCHORS ARE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
10. SPECIAL GC NOTE FOR CONCRETE WHICH IS PART OF A DEPRESSED AREA.

A. AT ALL DEPRESSIONS GREATER THAN OR EQUAL TO MINUS 6 INCHES FROM THE MAIN FINISH FLOOR THE GC SHALL PROVIDE XYPEX C-1000 ADMIXTURE (AT AN ASSUMED MINIMUM RATE OF 15 LBS/CU YD) INTO ALL OF THE CONCRETE. THE ADMIXTURE SHALL BE PLACED INTO THE MIX AT THE TIME OF BATCHING AT THE PLANT. DO NOT ADD DRY BAK MIX TO THE WET CONCRETE TRUCK ON-SITE. THE GC SHALL STRICTLY FOLLOW ALL MANUFACTURER'S INSTRUCTIONS FOR ADDITION, USE, HANDLING, ETC. THE ADD ADMIXTURE SHALL BE ADDED TO ALL OF THE CONCRETE WHICH MAKES UP ANY DEPRESSION GREATER THAN OR EQUAL TO MINUS 6 INCHES FROM THE MAIN FINISH FLOOR, THIS INCLUDES BUT IS NOT LIMITED TO: DEPRESSED SLABS (FULL THICKNESS), ALL VERTICAL STEM WALLS (FULL THICKNESS) AND/OR ANY GRADE BEAMS (FULL DEPTH AND THICKNESS WHICH FORM ANY PORTION OF THE VERTICAL DROP AND ALL HORIZONTAL SLAB FOR THE DEPRESSED AREA. THIS DOES NOT REDUCE OR REVISE ANY WATERPROOFING TREATMENTS. LAYERS THAT ARE SUBSTRATES REQUIRED BY THE STRUCTURAL, ARCHITECTURAL, AND/OR OTHER CONSULTANT DRAWINGS. THIS IS IN ADDITION TO THOSE CURRENT MEASURES. THE COST FOR THIS ADD ADMIXTURE SHALL BE ACCOUNTED FOR WITHIN THE BASE BID AND SHALL INCLUDE, BUT IS NOT LIMITED TO ALL AREAS ARCHITECTURALLY LABELED AS FOLLOWS: ORCHESTRA PIT, ALL ELEVATOR PITS, AND THE BELOW GRADE FLY LOFT ROOFING PIT. PLEASE NOTE XYPEX C-1000 HAS BEEN CHOSEN AS A NEUTRAL MIX ADDITIVE THAT IS NOT INTENDED TO CHANGE THE CURRENTLY PLANNED CONCRETE SET TIME. IF FOR SOME REASON THE SET TIME IS DESIRED TO BE INCREASED OR DECREASED XYPEX DOES HAVE ALTERNATIVE FORMULATIONS WHICH MAY BE USED. HOWEVER, THE GC MUST GET WRITTEN APPROVAL FROM THE EOR PRIOR TO ANY CHANGE IN THE XYPEX C-1000 FORMULATION. THE XYPEX C-1000 IS AN ADDITIONAL MOISTURE INTRUSION MITIGATION MEASURE THAT IS REQUIRED IN ADDITION TO THE SPECIFIED WATERSTOPS NOTED IN THE OTHER GENERAL NOTES AND DETAILS RELATED TO CONCRETE COLD-JOISTS AND OTHER CONCRETE TRANSITIONS OF PLANE.

CAST-IN-PLACE CONCRETE:

- 1. CONCRETE SUPPLIER SHALL BE AWARE OF CEMENTS THAT CAN CAUSE LATE ETTRINGITE FORMATION IN THE CEMENT PASTE AND BE PREPARED TO SHOW THAT THE CEMENTS USED WILL NOT CAUSE THIS PROBLEM.
2. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE, ACI 301 AND ACI 318, LATEST EDITIONS.
3. NO HORIZONTAL JOINTS WILL BE PERMITTED IN CONCRETE EXCEPT WHERE THEY NORMALLY OCCUR OR WHERE SHOWN ON THE DETAILS. VERTICAL JOINTS SHALL OCCUR AT CENTER SPANS OR AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.
4. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE, ACI 301 AND ACI 318, LATEST EDITIONS.
5. ALL BASE PLATES AND ANCHOR BOLTS SHALL BE PROTECTED WITH 3" (MIN.) OF CONCRETE. ANCHOR BOLTS SHALL BE FABRICATED FROM FULL BODIED ASTM F1554, GRADE 36 LOW CARBON STEEL RODS HAVING THE SAME DIAMETER AS THE BOLT DIAMETER AND USING CUT THREADS. ROLLED THREADS ARE NOT ACCEPTABLE. BOLTS SHALL BE SET USING RIGID TEMPLATES.
6. AT HORIZONTAL CONCRETE FRAMING, FORMS SHALL NOT BE REMOVED UNTIL THE CONCRETE HAS REACHED 70 PERCENT OF THE 28-DAY COMPRESSIVE STRENGTH INDICATED. FLOOR SLABS AND BEAMS SHALL REMAIN SHORED UNTIL THE UPPER MOST LEVEL OF CONCRETE HAS REACHED 70 PERCENT OF THE 28-DAY COMPRESSIVE STRENGTH. FOR BUILDINGS WITH MORE THAN THREE STORES IN HEIGHT, SHORING SHALL BE MAINTAINED FOR THREE LEVELS BELOW, UNTIL THE UPPER MOST LEVEL HAS REACHED 70 PERCENT OF THE SPECIFIED 28-DAY COMPRESSIVE STRENGTH.
7. ALL CONDUITS AND PIPES EMBEDDED IN CONCRETE SHALL COMPLY WITH ALL PROVISIONS SPECIFIED IN ACI 318 SECTION 8.3, WITH THE FOLLOWING SPECIFIC REQUIREMENTS:

- A. THE MAXIMUM OUTSIDE DIAMETER OF THE CONDUITS AND PIPES SHALL BE 1 1/2". NONE PERMITTED IN SLABS THINNER THAN 4" 1/2".
B. THE MINIMUM CLEAR DISTANCE BETWEEN CONDUITS AND PIPES SHALL BE 6".
C. NONE PERMITTED IN SLABS ON-GRADE WHICH WILL BE PERMANENTLY EXPOSED OR SCHEDULED TO RECEIVE THIN SET TILE. PLACE ALL PIPES AND CONDUITS IN THE FILL BENEATH THE VAPOR RETARDER, RE-COMPACT AS SPECIFIED.
D. IN NON-EXPOSED SLABS ON-GRADE, LIMIT SIZE TO 1" O.D. IN 5" SLAB SPACE 12" APART AND TIE TO UNDERSIDE OF REINFORCING MAT. WHERE LINES CONVERGE AT SOURCE, DOUBLE UP THE SLAB REINFORCING IN THE CONVERGENCE ZONE AND 3'-0" BEYOND. PLACE ALL LARGER LINES IN THE FILL BENEATH THE VAPOR RETARDER.
E. NONE PERMITTED IN COLUMNS WITHOUT PRIOR APPROVAL.
F. DO NOT DISPLACE REINFORCING STEEL FROM ITS PROPER POSITION.

- 8. PROVIDE SHEAR KEYS IN ALL CONSTRUCTION JOINTS IN BEAMS AND WALLS. IN ACCORDANCE WITH THE TYPICAL CONCRETE DETAILS.
9. PLACE WATERSTOPS IN ALL EXTERIOR CONSTRUCTION JOINTS BELOW GRADE AND ELSEWHERE AS CALLED FOR.
10. FLOORS ARE NOT DESIGNED TO SUPPORT FORMWORK AND WET CONCRETE WEIGHT OF NEXT LEVEL. CONTRACTOR SHALL DESIGN AND PROVIDE RE-SHORING TO PREVENT OVERSTRESSING THE STRUCTURE.
11. SET FORMS TO FOLLOW SLOPES AND GRADES DEFINED ON PLAN, KEEPING MEMBER DEPTHS CONSTANT AT DEPTHS DETAILED OR SCHEDULED, UNLESS NOTED OTHERWISE. SLOPE UNIFORMLY BETWEEN ELEVATIONS GIVEN. BUILD IN CAMBER WHERE SPECIFIED.
12. CONSTRUCTION JOINTS PERMITTED ONLY WHERE INDICATED ON DRAWINGS. WHERE NOT SPECIFICALLY INDICATED ON DRAWINGS, LOCATE THE JOINTS AS FOLLOWS:
A. LOCATE JOINTS NOT INDICATED TO LEAST MINOR STRENGTH AND APPEARANCE OF STRUCTURE. LOCATE VERTICAL JOINTS IN MIDDLE THIRD OF SPANS OF NON-POST-TENSIONED SLABS, BEAMS OR GIRDERS, UNLESS A BEAM INTERSECTS A GIRDER AT MIDDLE. LOCATE IN MIDDLE. LOCATE VERTICAL JOINTS IN GIRDERS TWICE WIDTH OF BEAM. LOCATE VERTICAL JOINTS WITHIN THE END THIRD OF SPANS OF POST-TENSIONED CONTINUOUS SLABS, BEAMS OR GIRDERS WHERE TENDON PROFILE IS AT OR NEAR THE CENTROID OF THE CONCRETE CROSS SECTION.
B. LOCATE HORIZONTAL JOINTS IN ALL SANDING. PLACE ALL UNDERSIDE OF SUPPORTED ELEMENTS AT THE TOP OF THE WALL OR COLUMN AND AT THE TOP OF FOOTINGS OR FLOOR SLABS AT THE BOTTOM OF THE WALL OR COLUMN. ROUGHEN SURFACE OF HORIZONTAL OR NEARLY HORIZONTAL CONSTRUCTION JOINTS SO THAT AGGREGATE SHALL BE UNIFORMLY LEAVING NO LAFANCE, LOOSENED PARTICLES OR DAMAGED CONCRETE.
C. REFER TO PLANS FOR ALL SUPPORTED SLABS.
D. JOINTS ARE NOT ALLOWED BETWEEN PLASTERS AND BEAM/WALL THAT ARE MONOLITHIC.
E. SUBMIT CONSTRUCTION JOINT LAYOUT PLANS FOR APPROVAL BY THE ENGINEER PRIOR TO CONSTRUCTION.

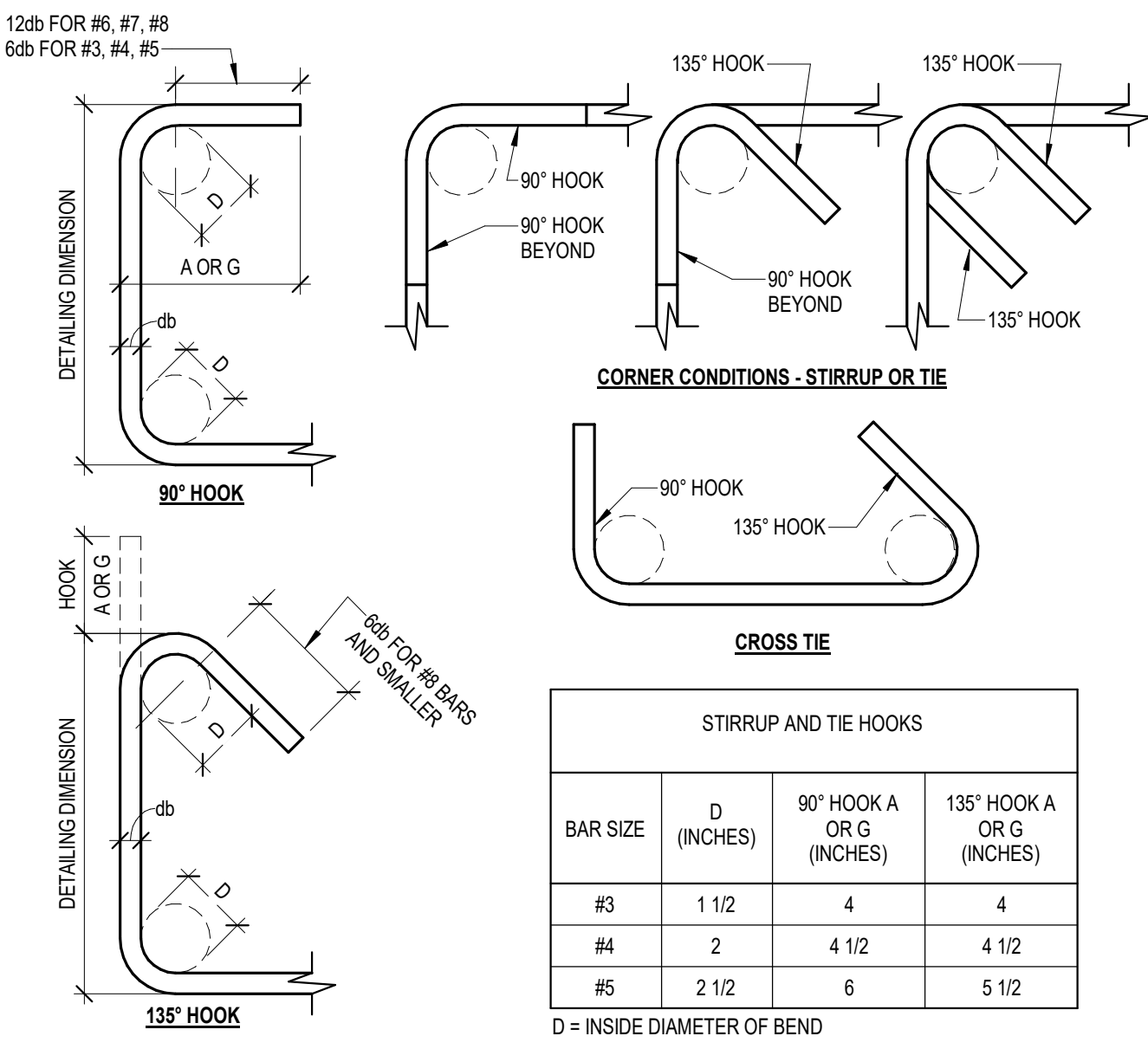
- 13. "GC NOTE"
A. AT ALL CONCRETE COLD JOINTS OR TRANSITIONS BETWEEN PLANES, VERTICAL TO VERTICAL, POURS, HORIZONTAL TO HORIZONTAL, POURS, HORIZONTAL TO VERTICAL, POURS, AND VERTICAL TO HORIZONTAL, POURS, THE GC SHALL PROVIDE A CONTINUOUS WATER STOP WITHIN EACH JOINT. WATERSTOP MANUFACTURERS AND TYPES SHALL BE AS APPROVED IN THE SPECS AND THE GENERAL NOTES. ALL WATER STOPS MUST BE APPROPRIATE FOR THE CONDITION BASED ON THE MANUFACTURER'S DATA. THE GC SHALL BE IN STRICT COMPLIANCE WITH ALL MANUFACTURER'S USE, HANDLING, AND INSTALLATION INSTRUCTIONS. AT A MINIMUM, THE GC SHALL ASSUME A DUMBBELL-TYPE WATER STOP WITH WINGS THAT EXTEND TO EACH SIDE OF THE CONCRETE FOR EACH JOINT UNLESS OTHERWISE NOTED WITHIN THE CDS TO BE AN ALTERNATE ACCEPTABLE WATER STOP.
B. ADDITIONALLY, CONCRETE USED AT A SLAB DEPRESSION WITH A DEPTH GREATER THAN 6 INCHES, THE CONCRETE ADDITIVE XYPEX C-1000 SHALL BE INCLUDED IN THE CONCRETE MIX. REFER TO CONCRETE MIX SCHEDULE FOR CONCRETE MIXES WHICH MAY REQUIRE XYPEX C-1000 AND CONCRETE MIX NOTE 10 FOR ADDITIONAL INFORMATION.

CONCRETE REINFORCEMENT:

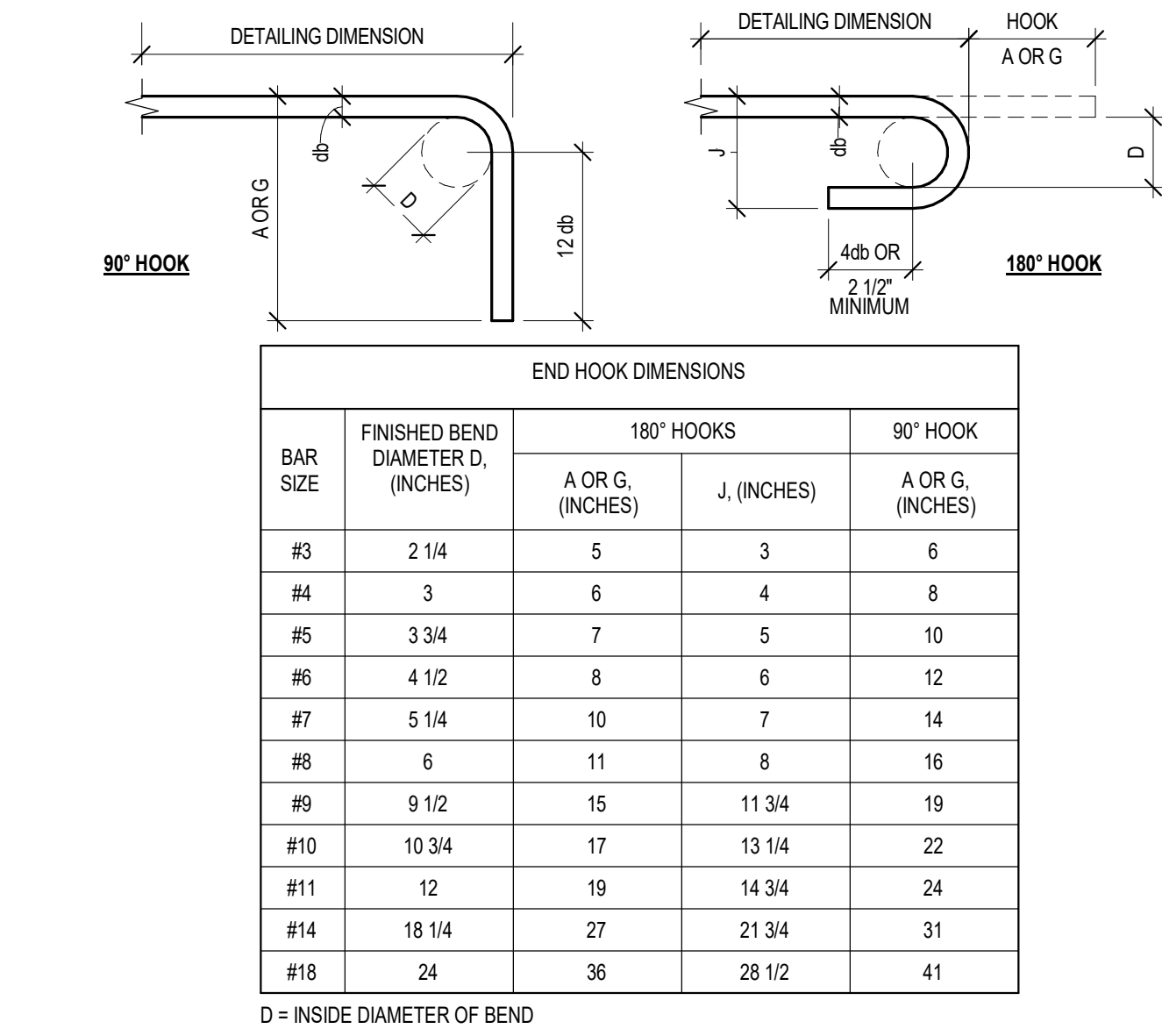
- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615. BARS SHALL BE NEW OR RECYCLED DOMESTIC BILLET STEEL OF A DOMESTIC MANUFACTURE. REINFORCING BARS SIZE #3 THROUGH #11 SHALL BE GRADE 60. REINFORCING BARS SIZE #11 THROUGH #18 SHALL BE GRADE 75.
2. DEFORMED BAR ANCHORS SHALL CONFORM TO ASTM A498, GRADE 70.
3. ALL WELDED WIRE FABRIC SHALL BE SMOOTH WIRE FABRIC CONFORMING TO ASTM A185, AND SHALL BE FURNISHED IN FLAT SHEETS.
4. CONCRETE COVERAGE AROUND REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 SECTION 7.7, LATEST EDITION, AND MEET REQUIREMENTS BELOW. THE REINFORCING STEEL DETAILER SHALL ADJUST REINFORCING STEEL CASE SIZES AT INTERSECTING REINFORCING MEMBERS AS REQUIRED TO ALLOW CLEARANCE FOR INTERSECTING BARS. SLAB ON GRADE REINFORCEMENT SHALL BE SUPPORTED AT EVERY THIRD BAR, NOT TO EXCEED 45-INCH INTERVALS.

Table with columns: FOOTINGS/PIERS, GRADE BEAMS, SLAB ON GRADE, SLAB BOTTOMS OVER VOID FORM, COLUMNS NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND (TYP INTERIOR CONDITIONS). Rows show 3 IN, 3 IN BOT, 2 IN SIDES (3" IF CAST AGAINST SOIL), 2 IN TOP, 2 IN TOP, 1 5/8".

- 5. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL CONFORM TO ACI 315, LATEST EDITION. ALL HOOKED BARS SHOWN SHALL HAVE STANDARD HOOKS, U.N.O.
6. REINFORCING SHALL NOT BE WELDED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
7. BOTTOM REINFORCING BARS SHALL BE SPICED AT SUPPORTS AND CONTINUOUS TOP BARS SHALL BE SPICED AT MID-SPAN.
8. ALL CONTINUOUS REINFORCEMENT SHALL BE LAPPED 56 BAR DIAMETERS AT SLAB LOCATIONS.
9. WHERE BAR TYPES FROM THE BAR BENDING DIAGRAM ARE SPECIFIED, PROVIDE BARS ACCORDINGLY. OTHERWISE, DETAIL BARS IN BEAMS, COLUMNS, SLABS, AND WALLS AS FOLLOWS:
A. RUN TOP AND BOTTOM BARS CONTINUOUS, WITH SPLICES AND HOOKS AS DESCRIBED BELOW.
B. PROVIDE STANDARD 90 DEGREE HOOK ON TOP BARS AT CANTILEVER ENDS.
C. SPLICE TOP AND INTERMEDIATE BARS AT THE CENTER LINE BETWEEN MEMBER SUPPORTS, UNLESS NOTED OTHERWISE.
D. SPLICE BOTTOM BARS DIRECTLY OVER MEMBER SUPPORTS, UNLESS NOTED OTHERWISE.
E. CENTER BARS NOTED AS 'AT SUPPORTS', OVER MEMBER SUPPORTS, AND CENTER BARS NOTED AS 'BTWN. SUPPORTS', BETWEEN SUPPORTS.
F. PLACE BARS NOTED AS '2ND LAYER' BELOW THE PRIMARY TOP BARS (OR ABOVE THE PRIMARY BOTTOM BARS) AND PROVIDE #11 SPACER BARS PLACED AT INTERVALS OF 4'-0" BETWEEN THE TWO LAYERS OF BARS.
G. ALL BAR SPLICES IN BEAMS, AND SLABS SHALL BE 30 BAR DIAMETERS, EXCEPT THAT SPLICES IN HORIZONTAL WALL BARS AND INTERMEDIATE BEAM BARS SHALL BE 36 BAR DIAMETERS.
H. PROVIDE CORNER BARS FOR EACH HORIZONTAL BAR AT THE INSIDE AND OUTSIDE FACES OF INTERSECTING BEAMS OR WALLS. REFER TO TYPICAL CORNER BAR DETAIL ON.



1 TYPICAL STIRRUP AND TIE HOOK TYPES NO SCALE



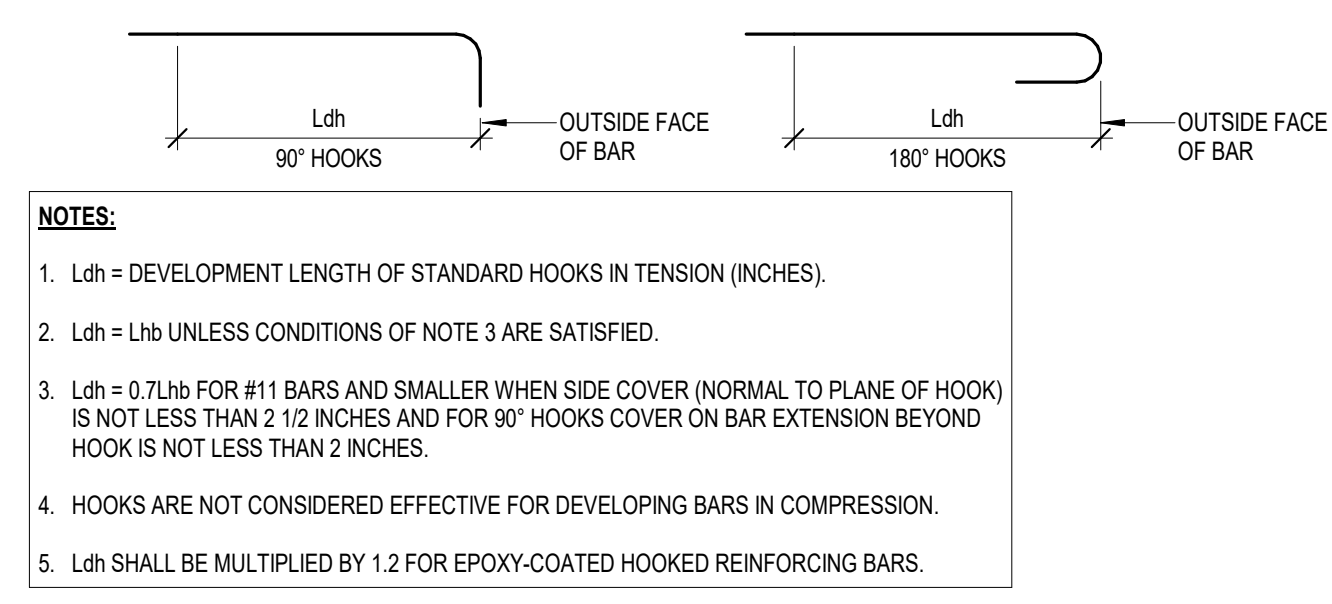
3 TYPICAL END HOOK TYPE NO SCALE

Table titled 'BEAM AND GIRDER TENSION DEVELOPMENT AND LAP SPLICE LENGTHS'. Columns include BAR SIZE, LAP CLASS, BOTTOM BARS, OTHER BARS, BOTTOM BARS, OTHER BARS, BOTTOM BARS, OTHER BARS. Rows include bar sizes #3 through #11 for concrete strengths fc = 3000 PSI, fc = 4000 PSI, and fc = 5000 PSI.

- NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR BEAMS AND GIRDERS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED 'CLASS A' LAP SPLICE LENGTH.
4. A BOTTOM BAR IS DEFINED AS ANY BAR THAT DOES NOT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
5. OTHER BARS INCLUDE TOP BARS, FACE BARS, AND ALL OTHER BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
6. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS OF BOTTOM BARS BY 1.5 AND THE TABULATED SPLICE LENGTHS OF OTHER BARS BY 1.3.
7. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE 'CLASS A' SPLICE LENGTH OF THE LARGER BAR.
8. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

5 TYPICAL BEAM AND GIRDER TENSION DEVELOPMENT AND LAP SPLICE LENGTHS NO SCALE

Table titled 'DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION'. Columns include BAR SIZE, fc=3000 PSI, fc=4000 PSI, fc=5000 PSI, BAR SIZE. Rows include bar sizes #3 through #18.



- NOTES:
1. Ldh = DEVELOPMENT LENGTH OF STANDARD HOOKS IN TENSION (INCHES).
2. Ldh = Ldb UNLESS CONDITIONS OF NOTE 3 ARE SATISFIED.
3. Ldh = 0.7Ldb FOR #11 BARS AND SMALLER WHEN SIDE COVER (NORMAL TO PLANE OF HOOK) IS NOT LESS THAN 2 1/2 INCHES AND FOR 90° HOOKS COVER ON BAR EXTENSION BEYOND HOOK IS NOT LESS THAN 2 INCHES.
4. HOOKS ARE NOT CONSIDERED EFFECTIVE FOR DEVELOPING BARS IN COMPRESSION.
5. Ldh SHALL BE MULTIPLIED BY 1.2 FOR EPOXY-COATED HOOKED REINFORCING BARS.

7 TYPICAL DEVELOPMENT LENGTHS OF STANDARD HOOKS IN TENSION NO SCALE

Table titled 'SLAB TENSION DEVELOPMENT AND LAP SPLICE LENGTHS'. Columns include BAR SIZE, LAP CLASS, BOTTOM BARS, OTHER BARS, BOTTOM BARS, OTHER BARS, BOTTOM BARS, OTHER BARS. Rows include bar sizes #3 through #9 for concrete strengths fc = 3000 PSI, fc = 4000 PSI, and fc = 5000 PSI.

- NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR SLABS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED 'CLASS A' LAP SPLICE LENGTH.
4. A BOTTOM BAR IS DEFINED AS ANY BAR THAT DOES NOT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR.
5. OTHER BARS INCLUDE TOP BARS AND ALL OTHER BARS THAT HAVE MORE THAN 12" OF FRESH CONCRETE BELOW THE BAR FOR TOP REINFORCEMENT IN SLABS THAT ARE 12" THICK OR LESS. TABULATED SPLICE LENGTHS FOR BOTTOM BARS SHALL BE USED.
6. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS OF BOTTOM BARS BY 1.5 AND THE TABULATED SPLICE LENGTHS OF OTHER BARS BY 1.3.
7. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE 'CLASS A' SPLICE LENGTH OF THE LARGER BAR.

2 TYPICAL SLAB TENSION DEVELOPMENT AND LAP SPLICE LENGTHS NO SCALE

Table titled 'WALL HORIZONTAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS'. Columns include BAR SIZE, LAP CLASS, fc = 3000 PSI, fc = 4000 PSI, fc = 5000 PSI. Rows include bar sizes #3 through #9.

- NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR WALL HORIZONTAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL VERTICAL BARS AND OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED 'CLASS A' LAP SPLICE LENGTH.
4. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.3.
5. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE 'CLASS A' SPLICE LENGTH OF THE LARGER BAR.

4 TYPICAL WALL HORIZONTAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS NO SCALE

Table titled 'WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS'. Columns include BAR SIZE, LAP CLASS, CATEGORY 1, CATEGORY 2, CATEGORY 1, CATEGORY 2, CATEGORY 1, CATEGORY 2, CATEGORY 1, CATEGORY 2. Rows include bar sizes #4 through #9.

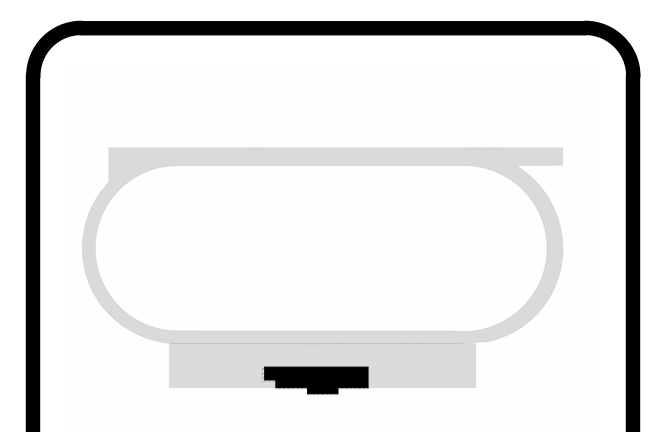
- NOTES:
1. ALL SPLICE LENGTHS ARE IN INCHES.
2. THIS TABLE SHALL BE USED FOR WALL VERTICAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL HORIZONTAL BARS AND OTHER MEMBERS.
3. THE TENSION DEVELOPMENT LENGTH (Ld) IS EQUAL TO THE SCHEDULED 'CLASS A' LAP SPLICE LENGTH.
4. FOR DETERMINING THE BAR CATEGORY, 'db' IS DEFINED AS THE DIAMETER OF THE LARGER BAR BEING SPICED.
5. THE 'CATEGORY 1' LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR GREATER THAN 3db.
6. THE 'CATEGORY 2' LAP LENGTH SHALL BE USED WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS LESS THAN 3db AND GREATER THAN 2db.
7. WHEN THE CLEAR SPACING BETWEEN BARS AT THE SPLICE IS EQUAL TO OR LESS THAN 2db, SPLICES SHALL BE STAGGERED SO THAT NO MORE THAN 50% OF BARS ARE SPICED AT ANY GIVEN LOCATION. REFER TO TYPICAL CLEAR SPACING CRITERIA OF LAP SPICED BARS, STAGGERED SPLICES FOR CRITERIA TO DETERMINE CLEAR SPACING 'S' FOR BARS AT STAGGERED SPLICES.
8. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.5.
9. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE 'CLASS A' SPLICE LENGTH OF THE LARGER BAR.

6 TYPICAL WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS NO SCALE



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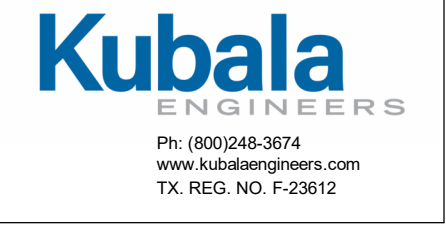
KEY PLAN NORTH PLAN TRUE

DISCIPLINE JOHN R. KUBALA 106120 04/24/2024 Kubala Engineers F-23612

Table with columns: CLIENT GALVESTON ISD, DATE 01/31/2024, PROJECT NUMBER 230063, DRAWING HISTORY.

ISSUE FOR PROPOSAL BUILDING NUMBER

GENERAL CONCRETE AND STL REIN NOTES AND TYP DETAILS



SLAB-ON-GRADE:

- INSTALL VAPOR RETARDER IN ACCORDANCE WITH SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS IMMEDIATELY BELOW SLAB. PROPERLY TAPE ALL SEAMS PER MANUFACTURER'S RECOMMENDATIONS AND REMOVE ANY REMOVED STANDING WATER ON THE SURFACE OF VAPOR RETARDER PRIOR TO CONCRETE PLACEMENT.
- PROVIDE (2) #5 x 5'-0" LONG BAR IN SLAB AT RE-ENTRANT CORNERS. TYPICAL, INCLUDING RE-ENTRANT CORNERS AROUND THE PERIMETER OF THE BUILDINGS, FLOOR RECESSES AND OPENINGS.
- PROVIDE CONTROL JOINTS ALONG COLUMN LINES AND AT 15'-0" ON CENTER, EACH WAY, MAXIMUM, UNLESS NOTED OTHERWISE. ALSO, PROVIDE ADDITIONAL CONTROL JOINTS AT RE-ENTRANT CORNERS. SEE TYPICAL DETAILS.
- JOINT FILLER SHALL BE USED ON ALL JOINTS THAT DO NOT GET COVERED BY AN ARCHITECTURAL FLOOR COVERING. ACCEPTABLE PRODUCTS INCLUDE EDGE PRO 90. STRICTLY FOLLOW MANUFACTURER-WRITTEN RECOMMENDATIONS FOR INSTALLATION. DELAY JOINT FILLING AS LONG AS PRACTICAL TO ALLOW FOR MAXIMUM CONCRETE SHRINKAGE.
- DO NOT PLACE HEAVY EQUIPMENT OR CONSTRUCTION STORAGE LOADS ON THE SLAB UNLESS CALCULATIONS PREPARED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE IN WHICH THE PROJECT IS LOCATED HAVE BEEN SUBMITTED, VERIFYING THE ADEQUACY OF THE SLAB.
- TESTING LAB WILL TEST ALL PLACEMENTS IN ACCORDANCE WITH ASTM E 1155 TO CONFIRM F-NUMBERS AND ADEQUACY OF FINISHING PROCEDURES.
- THE PERFORMANCE OF THE SLAB IS HIGHLY DEPENDENT ON HOW WELL THE CONTRACTOR FOLLOWS THE BUILDING PAD PREPARATION RECOMMENDATIONS AS SPECIFIED IN THE GEOTECHNICAL REPORT AND EARTHWORK SECTION OF THE GENERAL NOTES. THE CONTRACTOR AND OWNER SHOULD CONSULT WITH THE GEOTECHNICAL ENGINEER IF THERE ARE ANY QUESTIONS CONCERNING DESIGN, CONSTRUCTION METHODS, PERFORMANCE AND RISKS INVOLVED IN THE CONSTRUCTION OF A GRADE SUPPORTED SLAB.
- SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS.

GEO-FOAM WITH CONCRETE TOPPING:

- ALL EPS GEO-FOAM SHALL BE PER ASTM D8817 AND TYPE EPS XI OR BETTER.
- UNLESS OTHERWISE NOTED, ALL GEO-FOAM SHALL BE FINISHED WITH 4" THICK CONCRETE TOPPING SLAB WITH #3 AT 12" O.C. EACH WAY CENTERED IN THE SLAB.
- CONSTRUCTION JOINTS SHALL BE FOLLOWED PER PLANS. ADJACENT POURS SHOULD BE SCHEDULED MINIMUM 4 DAYS APART.
- ALL GEO-FOAM SHALL BE ADHERED TO THE BASE SLAB AND TO EACH OTHER WITH A NON-SOLVENT ADHESIVE WITH A MINIMUM SHEAR STRENGTH OF 5 PSF.

TOOLED EDGE
(1) #3 CONTINUOUS EACH SIDE OF JOINT SUPPORTED INDEPENDENTLY FROM DOWEL BAR
WHERE JOINT FILLER IS SPECIFIED, PREPARE EDGE ON EACH SIDE OF JOINT WITH 1/8" RADIUS SAWCUT JOINT, AND PROVIDE JOINT FILLER
#5 x 2'-0" @ 24" OC
CAP END
ENCASE ONE END W/ PVC OR RUBBER SLEEVE
CONTINUOUS KEYWAY (REMOVE METAL KEYWAY PRIOR TO SECOND POUR)
VAPOR BARRIER

NOTES:
1. AT CONTRACTOR'S OPTION, CONTRACTION JOINT MAY BE USED IN PLACE OF CONSTRUCTION JOINT. LOCATIONS SHALL BE PROPOSED BY THE CONTRACTOR AND APPROVED BY THE ARCHITECT.
2. CONTRACTION JOINT MAY BE SAW CUT TO 1/4 OF THE SLAB DEPTH WITHIN 6 HOURS AFTER SLAB PLACEMENT.

1 SLAB CONSTRUCTION JOINT
NO SCALE

TOP COVER SHALL BE 1/4"-1/2" MIN. INCREASE AS REQUIRED BY OTHER DETAILS
SAWCUT DEPTH TO REF. JOINT REQUIREMENTS
1/8"
PROVIDE JOINT FILLER ONLY IF SPECIFIED
VAPOR BARRIER

NOTES:
1. PROVIDE JOINT AT EVERY COLUMN LINE AND IN BETWEEN THE COLUMN LINES AT A MAXIMUM SPACING OF 36 TIMES THE SLAB THICKNESS (NOT TO EXCEED 16 FEET), UNO.
2. GC SHALL SUBMIT JOINT LAYOUT TO THE ARCHITECT FOR APPROVAL.

2 CONTRACTION (CONTROL) JOINT
NO SCALE

L1 1/4" x 1/4" GALV. FRAME WITH 1/2" x 4" HSA AT 6" O.C.
#4 DWLS @ 12" OC ALL AROUND PIT
1"x1/4" BEARING BARS AT 2'-0" OC WITH 9/8" CROSS RODS AT 2' OC GALVANIZED SUMP GRATE
WATER VAPOR BARRIER
2'-4" x 1'-4" (AT SINGLE WASHER BASIN)
5'-3" x 1'-4" (AT DOUBLE WASHER BASIN)
8'-0" x 1'-4" (AT THREE WASHER BASIN)
#4 @ 12" OC EW
COMP SELECT FILL
VERIFY LOCATION WITH ARCHITECTURAL DRAWINGS

3 DRAIN TROUGH DETAIL
NO SCALE

REF ARCH
SLOPE
WATER VAPOR BARRIER
FLOOR DRAIN REF PLUMBING REF MANUF FOR DIMENSIONS
DRAIN WIDTH (H)
COMP SELECT FILL
(2) #5 EW
VERIFY LOCATION WITH ARCHITECTURAL DRAWINGS

4 FLOOR DRAIN DETAIL
NO SCALE

NOTE:
1. GC SHALL COORDINATE DEPTH AND LOCATION OF ALL FLOOR DEPRESSIONS WITH THE ARCHITECTURAL DRAWINGS.

(1) #3 CONT. @ EDGES
REF PLAN FOR REINF
TOSC ELEV REF PLAN
MAINTAIN SPECIFIED SLAB THICKNESS AT DEPRESSIONS (TYP)
8" MIN

SLAB RECESS LESS THAN 3" DEEP

#4 DOWEL TO MATCH SPACING OF SLAB ON GRADE REINF
1'-6" TYP
TOSC ELEV REF PLAN
MAINTAIN SPECIFIED SLAB THICKNESS AT DEPRESSIONS (TYP)
1'-2" MIN

SLAB RECESS 3" TO 10" DEEP

NOTE:
1. GC SHALL COORDINATE CURB SIZES AND LOCATIONS WITH ARCH DRAWINGS.

CURB WIDTH GREATER THAN 8"
#4 HORIZ AT 12" OC EF - TYP
#3 AT 12" OC - TYP
STANDARD HOOK IN THE DIRECTION OF THE LOWEST LAYER REINF - TYP

CURB WIDTH GREATER THAN 8"
#4 HORIZ AT 12" OC EF - TYP
#3 AT 12" OC - TYP
FINISH FLOOR
STANDARD HOOK IN THE DIRECTION OF THE LOWEST LAYER REINF - TYP

8" OR LESS
#4 CONT AT HOOK AND @ 12" OC

#3 @ 12" OC

7 TYPICAL CURB DETAILS
NO SCALE

INTERRUPTED SLAB REINFORCEMENT (HOOK TOP BARS)
PROVIDE 1-#4 TOP AND 1-#4 BOTTOM CORNER BARS (INNER LAYER)
B/2+L/8+2"
A
INTERRUPTED SLAB REINFORCEMENT (HOOK TOP BARS)
PROVIDE HALF OF INTERRUPTED REINFORCEMENT PLUS ONE ADDITIONAL BAR OF SAME SIZE ON EACH SIDE OF OPENING

NOTES:
1. L₈ EQUALS BAR "CLASS B" TENSION LAP SPICE LENGTH, UNLESS NOTED OTHERWISE.
2. SLAB OPENINGS ARE ONLY PERMITTED WHERE INDICATED ON THE STRUCTURAL PLANS OR AS APPROVED BY THE STRUCTURAL ENGINEER.

8 TYPICAL SLAB OPENING REINFORCEMENT
NO SCALE

NOTE:
1. PROVIDE EXTRA HORIZ REINF AT EACH SIDE OF DRAIN EQUAL TO REINFORCING INTERRUPTED BY DRAIN DEPRESSION

REF ARCH FOR FINAL PROFILE
2'-6" DIA
REF PLAN FOR SLAB REINF AND SLAB NOTES
VAPOR RETARDER
VERIFY LOCATION WITH ARCHITECTURAL DRAWINGS

5 TYPICAL SLAB RECESS AT RESTROOM DRAIN
NO SCALE

#4 AT 12" OC CONT WITH (3) #5 CONT
MAINTAIN SPECIFIED SLAB THICKNESS AT DEPRESSIONS (TYP)
CONTINUOUS WATERSTOP
2'-0" TYP
TOSC ELEV REF PLAN
1'-0"
1'-0"
6"
SLAB RECESS 10" TO 36" DEEP

6 TYPICAL SLAB DEPRESSION DETAILS
NO SCALE

1 1/2" TYP
DEEP CONT KEY
#4 DOWELS AT 16" O.C. WITH #4 CONT WHERE NOT SHOWN
3'-0"

12 TYPICAL WALL TO SLAB CONSTRUCTION JOINT
NO SCALE

NOTE:
1. FABRICATOR TO LOCATE THESE BARS ON THE SHOP DRAWINGS.

PROVIDE 36x3'-0" DIAGONAL BARS 1" BELOW TOP OF SLAB
GRADE BEAM CONSTRUCTION JOINT OR RECESS
45.00"
BLOCKOUTS
CORNERS

9 TYPICAL RE-ENTRANT CORNER BARS DETAIL
NO SCALE

SEE MANUF FOR EMBEDS AND DIMENSIONS
1'-0" TYP
DRAIN ASSEMBLY AND SLOPE REF ARCH AND PLUMBING
PROVIDE HOOK WHERE SLAB IS NOT PRESENT
(4) #4 CONT AT DRAIN
#4 AT 12" OC
DEPTH OF TRENCH DRAIN 4" MIN
3" CLR
3" CLR
WIDTH OF TRENCH DRAIN 1'-1/2" MIN
VERIFY LOCATION WITH ARCHITECTURAL DRAWINGS

10 TYPICAL TRENCH DRAIN AT SLAB-ON-GRADE
NO SCALE

NOTE:
1. CONTRACTOR TO COORDINATE LOCATIONS OF SIFFENER BEAMS TO ALIGN WITH LOCATIONS OF STAIR STRINGERS & LANDING POSTS. COORDINATE WITH STAIR MANUFACTURER AS REQUIRED.

STAIR LANDING POST
L4x4x3/8x1'-0" WITH (2) 3/4" DIA EXP ANCHORS AT 8" O.C. (EMBED = 4 3/4") PLACE ANGLE AT INSIDE FACE OF STRINGER
5/16
PAVING BY OTHERS
1'-0"
THICKENED PAVING BY OTHERS
2'-0"

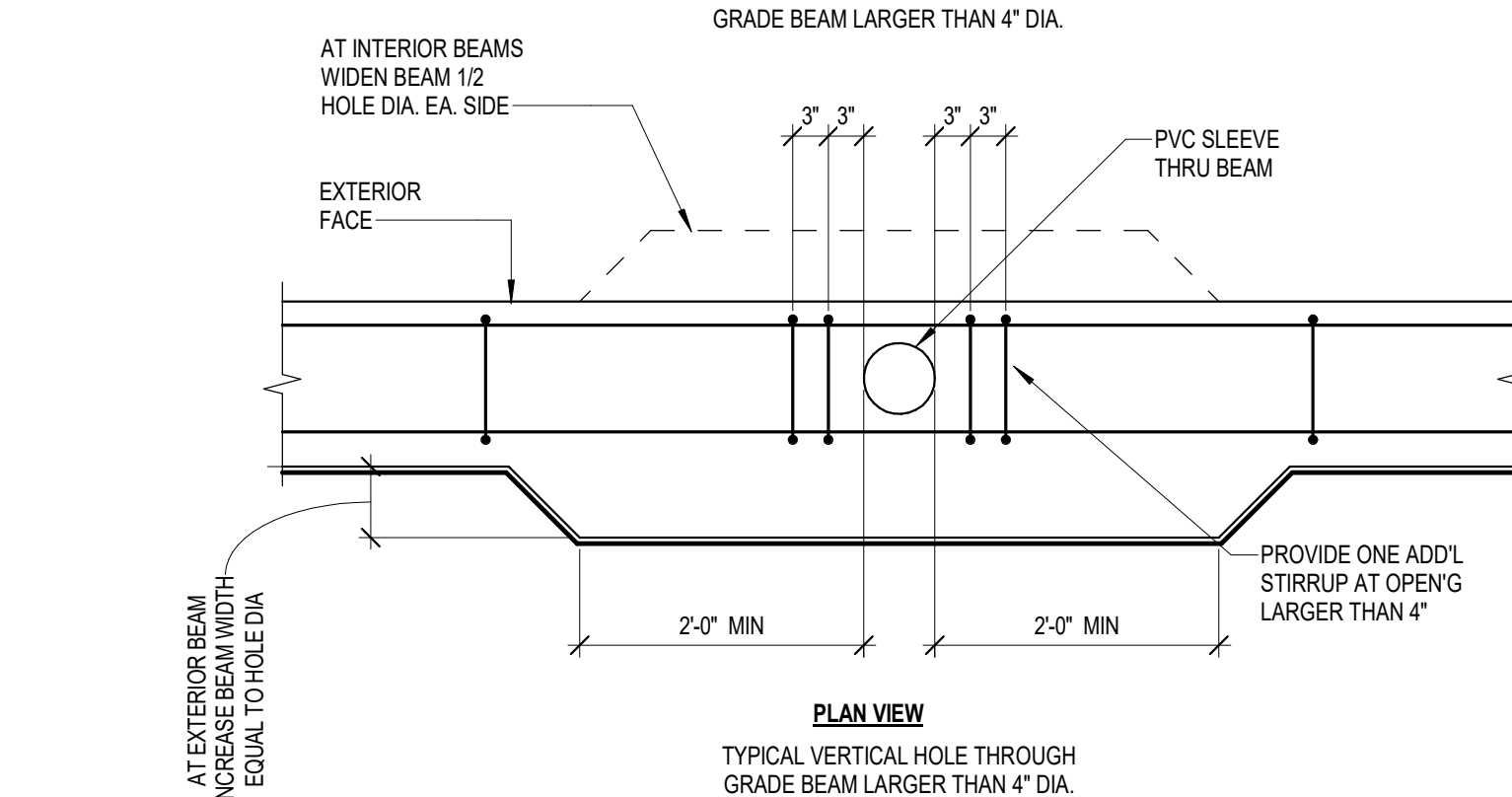
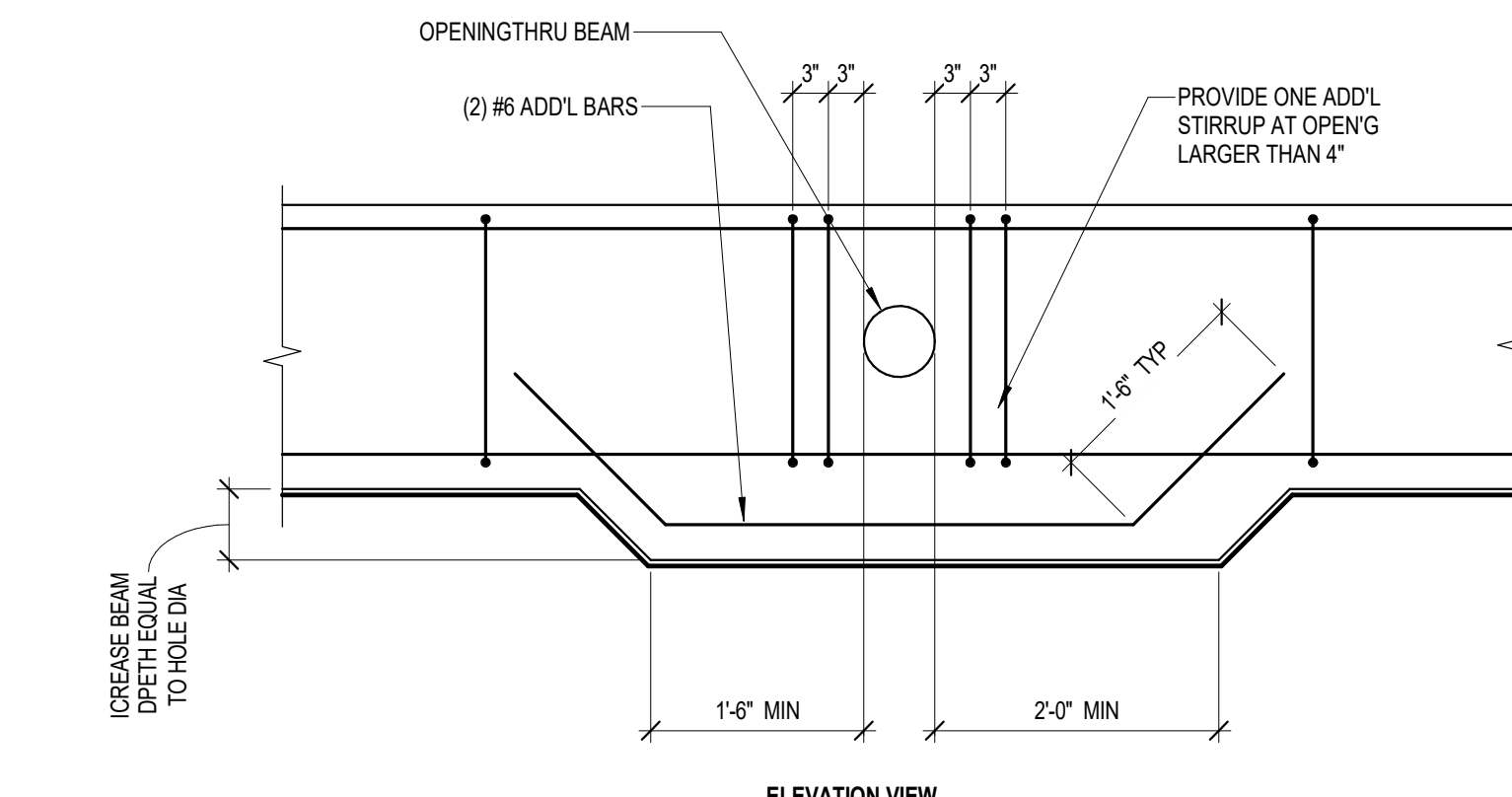
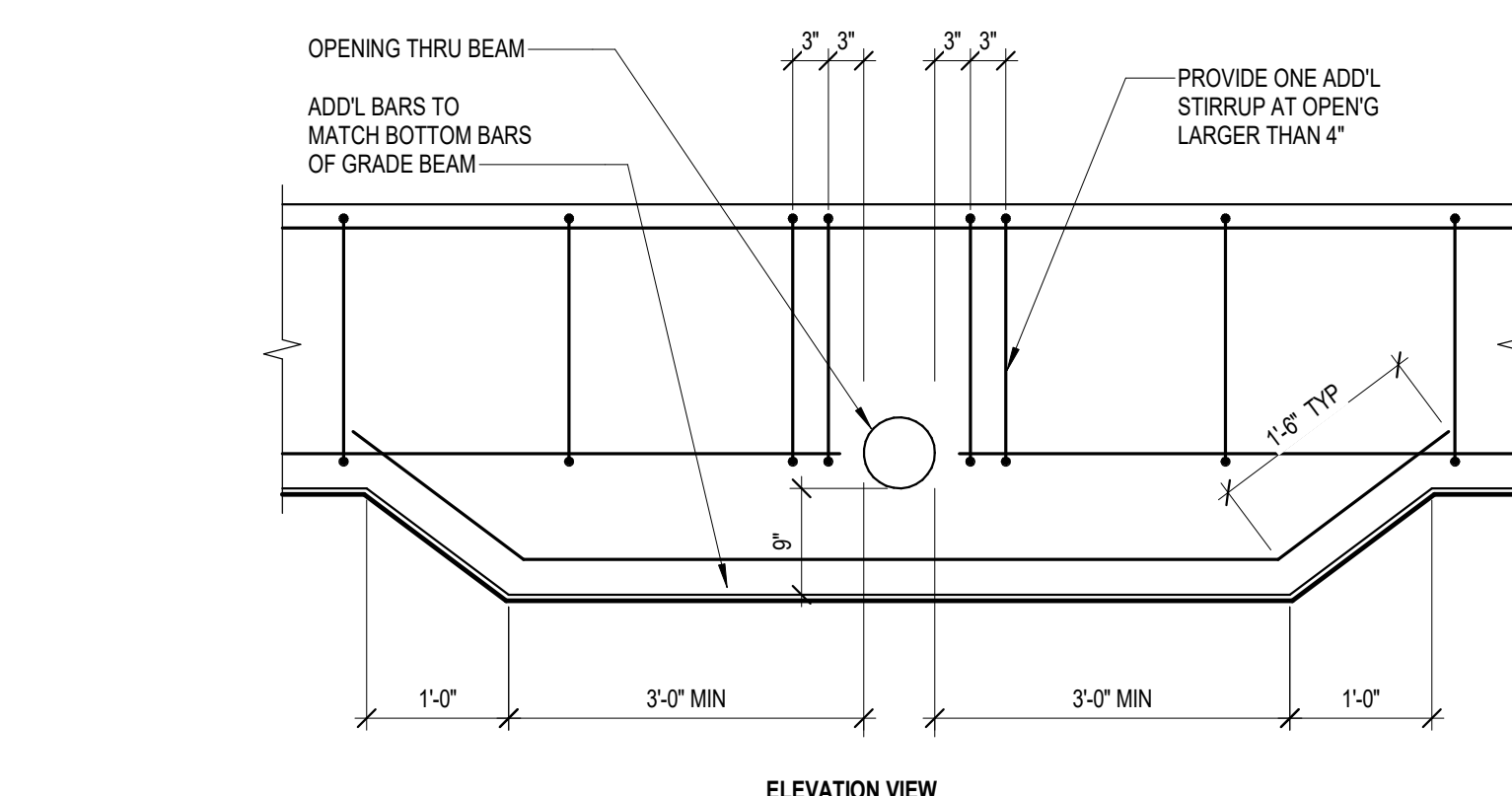
11 TYPICAL SLAB STIFFENER UNDER STAIR STRINGER
NO SCALE

USE STANDARD 90 DEGREE HOOK OR 180 DEGREE HOOK WHERE BEAM OR WALL IS NOT PRESENT BELOW THE SLAB
#4 CONT AT CORNER WHERE NO OTHER REINF IS PRESENT OR SPECIFIED
DOWELS REF SPECIFIC DETAILS, USE #4 AT 12" OC WHERE DOWELS ARE NOT SPECIFICALLY NOTED
#4 CONT AT DOWEL CORNERS (EACH FACE) WHERE NO OTHER REINF IS PRESENT OR SPECIFIED
2'-0" MIN BEYOND FACE OF BEAM OR WALL
2'-0" MIN BEYOND FACE OF BEAM OR WALL
2'-0" MIN INTO BEAM OR WALL
2'-0" MIN INTO BEAM OR WALL
2'-0" MIN BEYOND FACE OF BEAM OR WALL
2'-0" MIN BEYOND FACE OF BEAM OR WALL

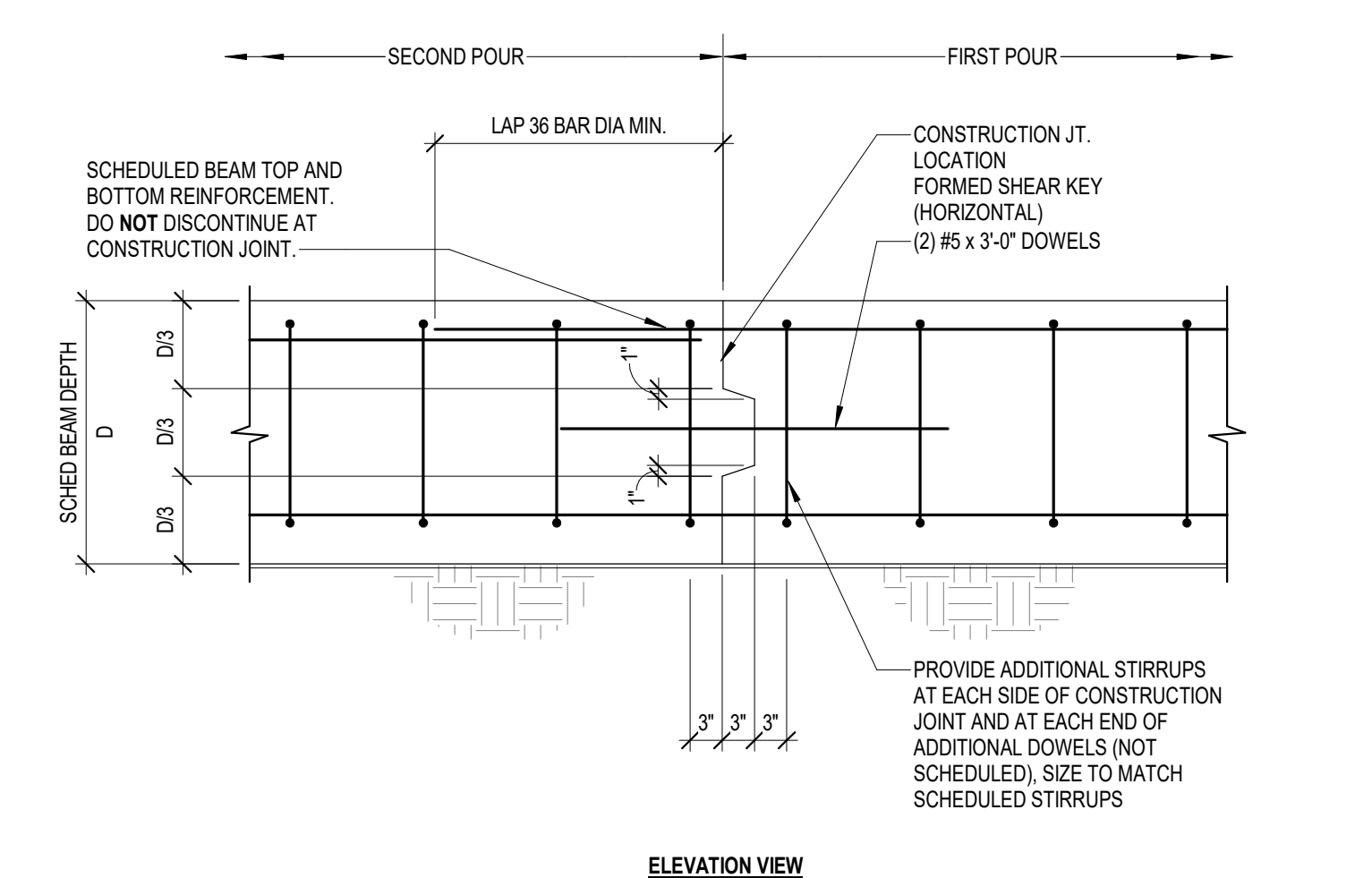
13 TYPICAL SLAB DOWEL
NO SCALE

GRADE BEAM:

- GRADE BEAM DIMENSIONS AND/OR LOCATIONS SHALL NOT BE ALTERED WITHOUT APPROVAL OF THE ENGINEER OF RECORD. SIDES OF GRADE BEAMS SHALL BE FORMED. EARTH FORMING OF GRADE BEAMS IS NOT ALLOWED UNLESS GRADE BEAMS MAY BE EARTH FORMED SO LONG AS THE SIDES ARE PLUMB AND SOUND AND ANY PORTION OF THE BEAM/SLAB THAT EXTENDS ABOVE GRADE/ EXPOSED IS BOARD FORMED. THE WALLS MUST NOT SLOUGH OFF MORE THAN 1/2" OUT OF PLANE OF THE PLUMB LINE. WHERE THIS OCCURS A BOARD FORM MAY BE LAID IN THE EARTH TO SMOOTH THE SIDE LOCALLY AT THE IMPERFECTION TO MAINTAIN WALL FLATNESS TOLERANCE.
- GRADE BEAMS SHALL BE POURED MONOLITHICALLY AROUND CORNERS AND AT INTERSECTIONS. SEE TYPICAL GRADE BEAM CONSTRUCTION JOINT DETAIL FOR ACCEPTABLE CONSTRUCTION JOINT LOCATIONS.
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION, SIZE, AND ELEVATION AND INCLUDE IN HIS CONTRACT PRICE ALL REQUIRED HORIZONTAL PENETRATIONS THROUGH CONCRETE BEAMS WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. WHERE BEAM PENETRATIONS ARE REQUIRED BUT ARE NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS, CONTRACTOR SHALL SUBMIT DRAWINGS SHOWING DIMENSIONS AND LOCATIONS OF ALL REQUIRED PENETRATIONS FOR REVIEW AND APPROVAL.



4 TYPICAL GRADE BEAM PENETRATION DETAILS NO SCALE



- NOTES:**
- LOCATE JOINTS IN MIDDLE THIRD OF BEAM SPAN BETWEEN SUPPORTS.
 - ACCEPTABLE JOINT LOCATIONS ARE AS FOLLOWS:
 - FOR BEAMS NOT SUPPORTING INTERSECTING BEAMS, PLACE JOINT WITHIN MIDDLE THIRD OF SPAN.
 - FOR BEAMS SUPPORTING INTERSECTING BEAMS, CHECK WITH STRUCTURAL ENGINEER FOR JOINT LOCATIONS AND DWEL REQUIREMENTS.
 - FOR JOINT LOCATIONS OTHER THAN WITHIN MIDDLE THIRD OF SPAN, CONTRACTOR SHALL COORDINATE REQUIRED ADDITIONAL REINFORCEMENT WITH THE ENGINEER ON THE SHOP DRAWINGS.
 - JOINTS MAY NOT OCCUR IN THE FIRST SPAN OF A BEAM LINE OR IN ANY SPAN WHICH IS LESS THAN 8 FEET.
 - GENERAL CONTRACTOR SHALL SUBMIT DESIRED CONSTRUCTION JOINT LAYOUT AS A SHOP DRAWING FOR APPROVAL A MINIMUM OF TWO WEEKS PRIOR TO POUR.

7 TYPICAL GRADE BEAM CONSTRUCTION JOINT NO SCALE

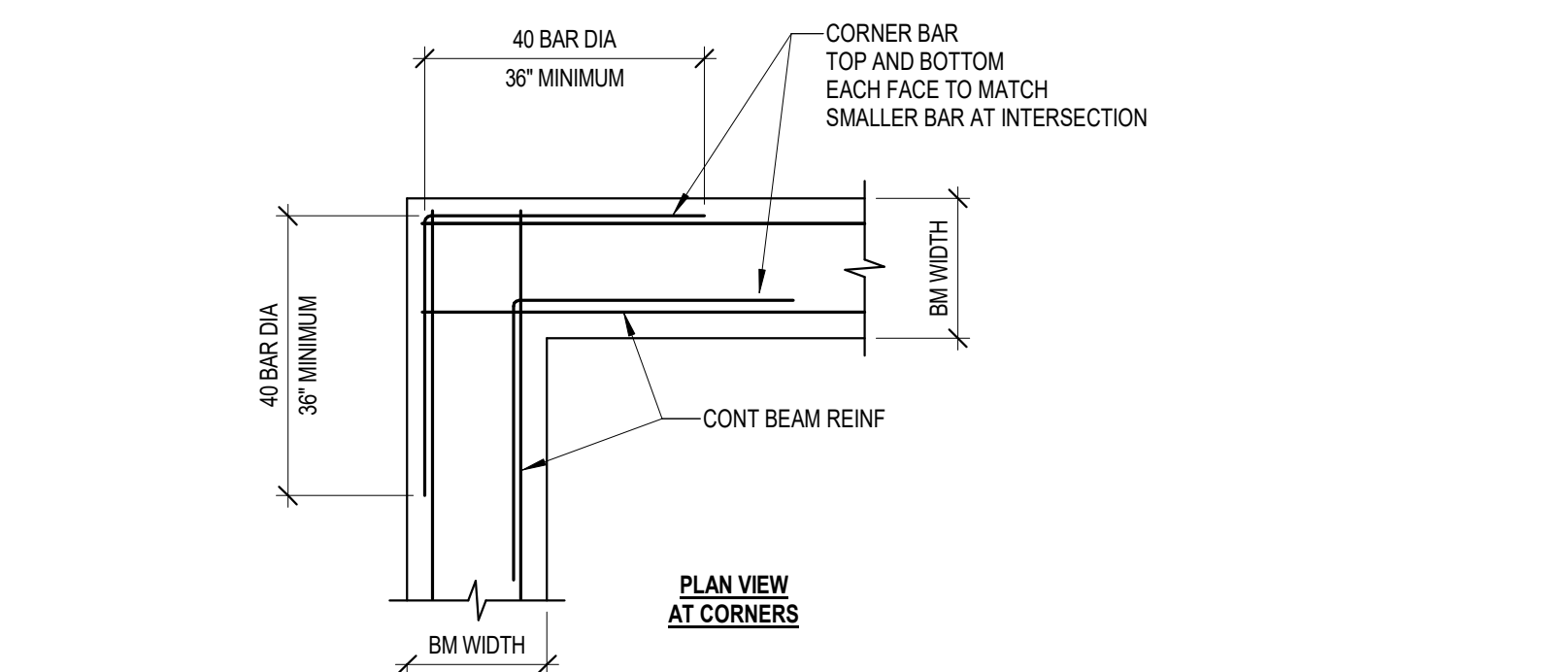
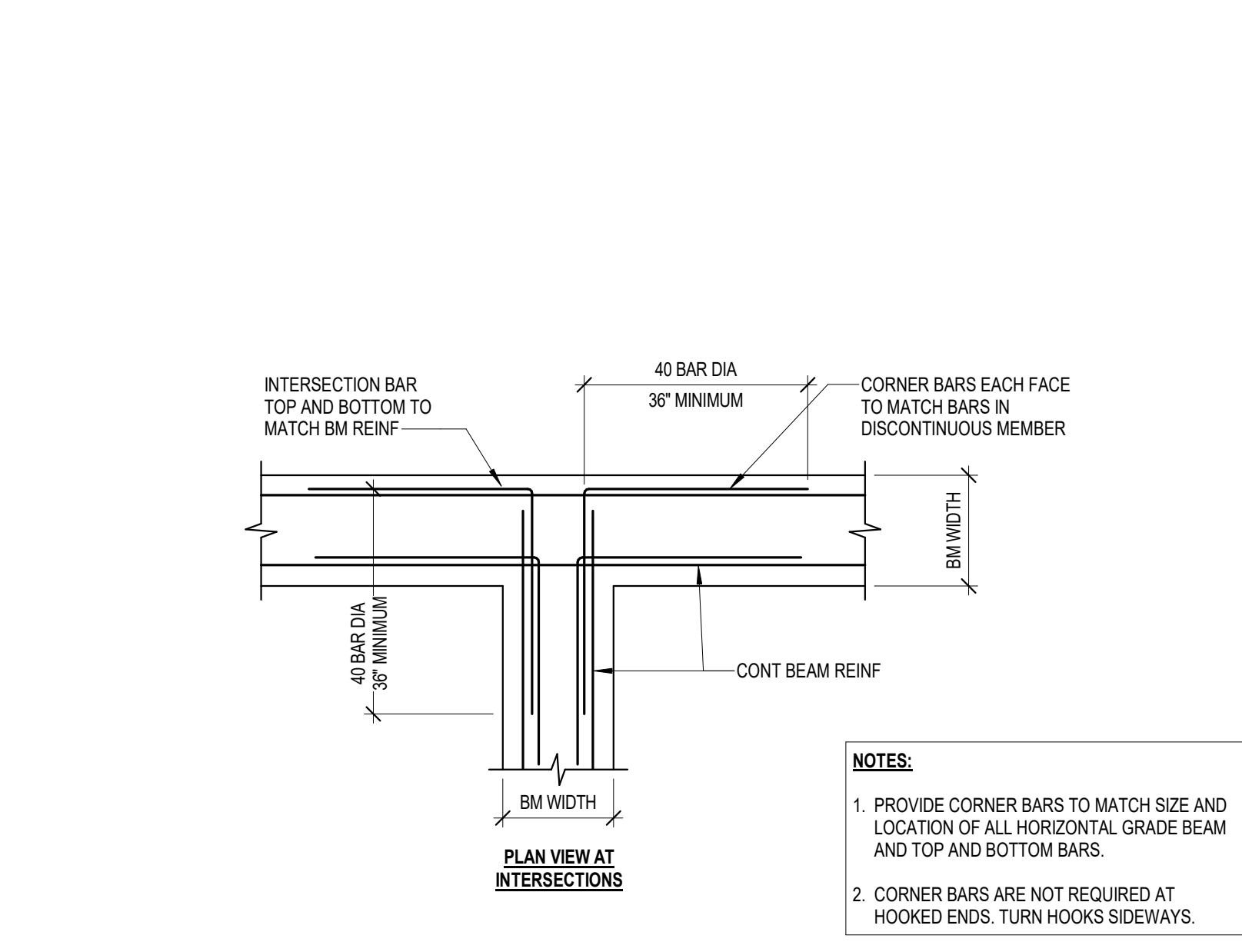
GRADE BEAM SCHEDULE

MARK	SIZE		REINFORCING			TIES		REMARKS
	WIDTH	DEPTH	TOP	BOTTOM	MIDDLE	SIZE	SPACING	
GB1	1'-6"	2'-6"	3#9	3#9	-	#4 S1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-
GB2	2'-0"	2'-6"	4#9	4#9	-	#4 S1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-
GB3	2'-8"	2'-6"	5#9	5#9	-	#4 S1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-
GB4	2'-10"	2'-6"	6#9	6#9	-	#4 D1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-
GB5	3'-0"	2'-6"	6#9	6#9	-	#4 D1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-
GB6	1'-2"	2'-6"	3#7	3#7	-	#4 D1	1#2", R @ 10"; 1#2", R @ 6" @ CANTI	-

INTERMEDIATE BAR SCHEDULE

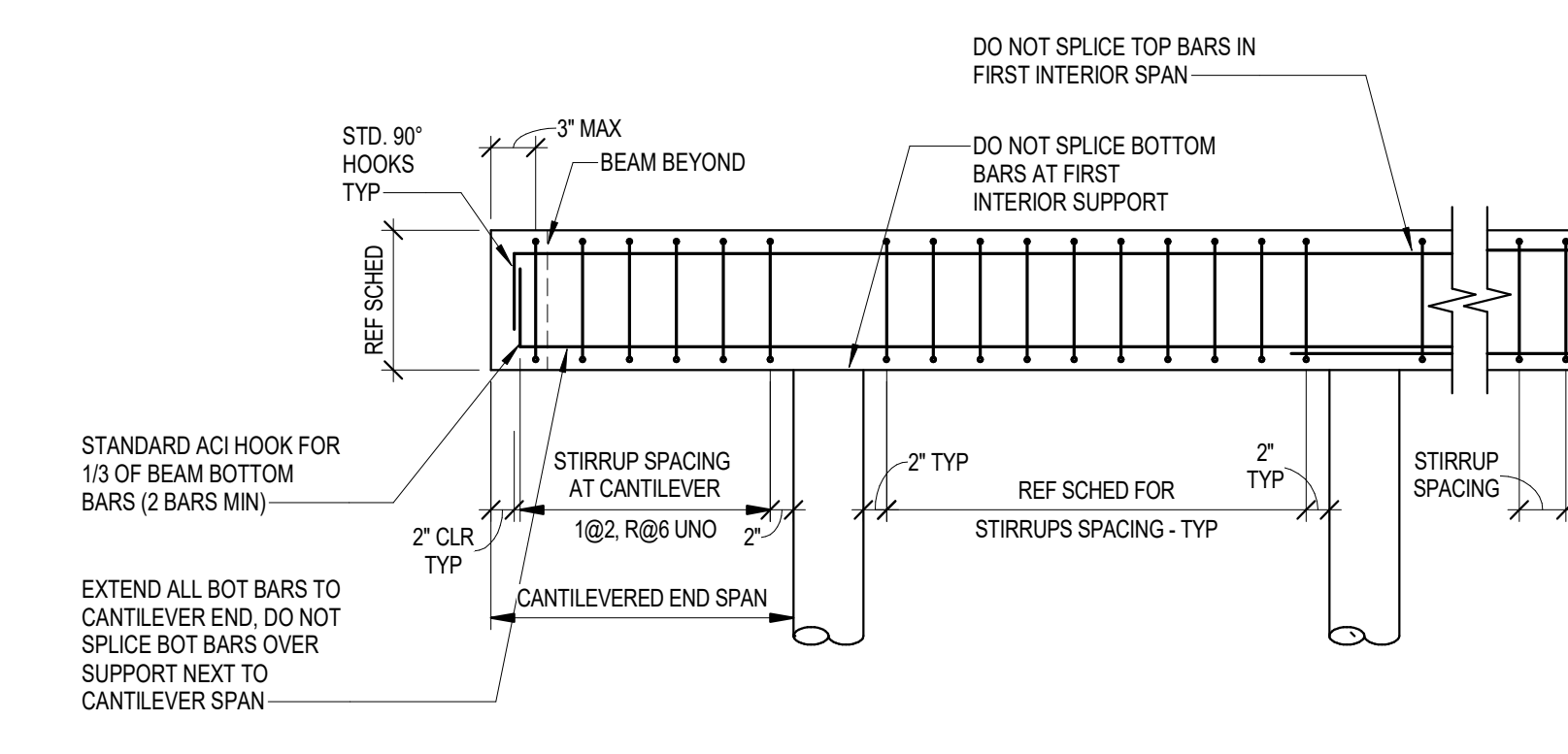
BEAM DEPTH	NUMBER OF BARS EA FACE	SIZE OF BAR
> 36" - 48"	5	#3
49" - 54"	5	#4
55" - 60"	5	#5
61" - 66"	6	#5
67" - 78"	7	#5
79" AND ABOVE	AT 9" OC	#5

1 GRADE BEAM SCHEDULE NO SCALE

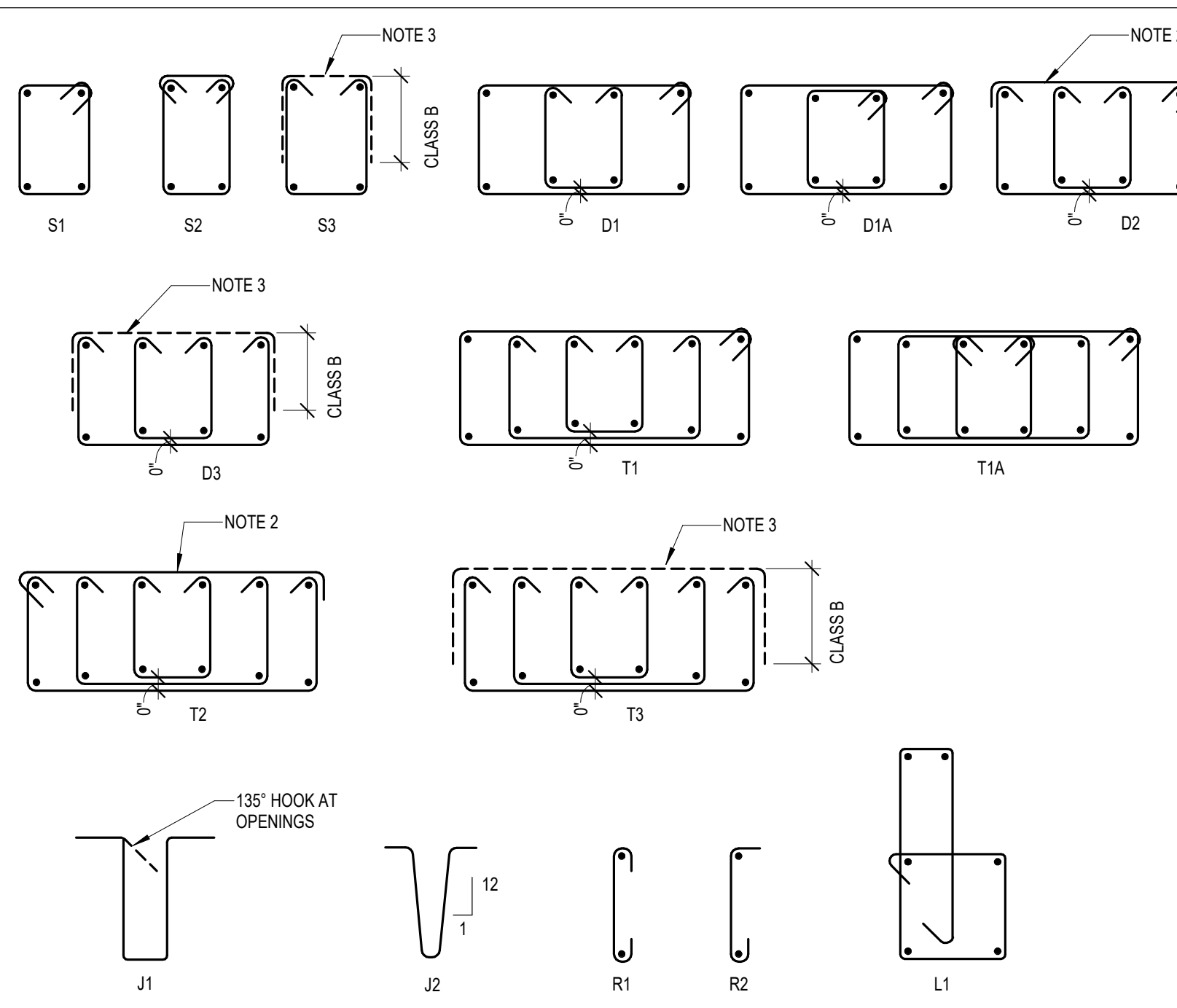


5 TYPICAL CORNER BAR DETAILS NO SCALE

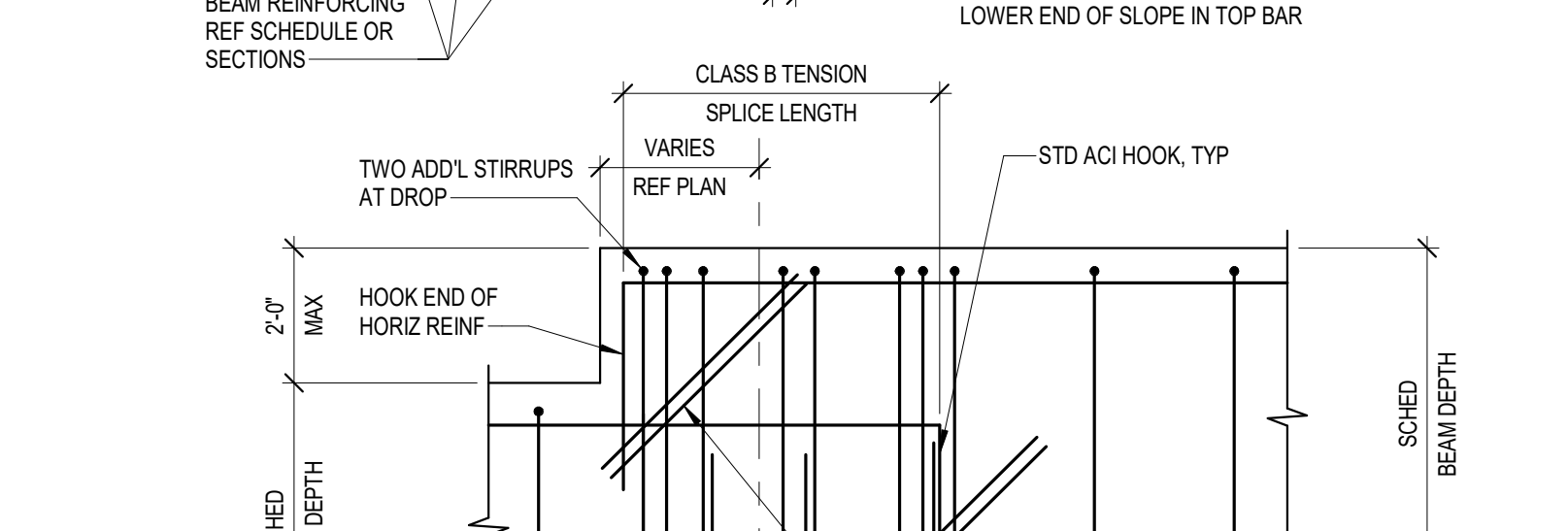
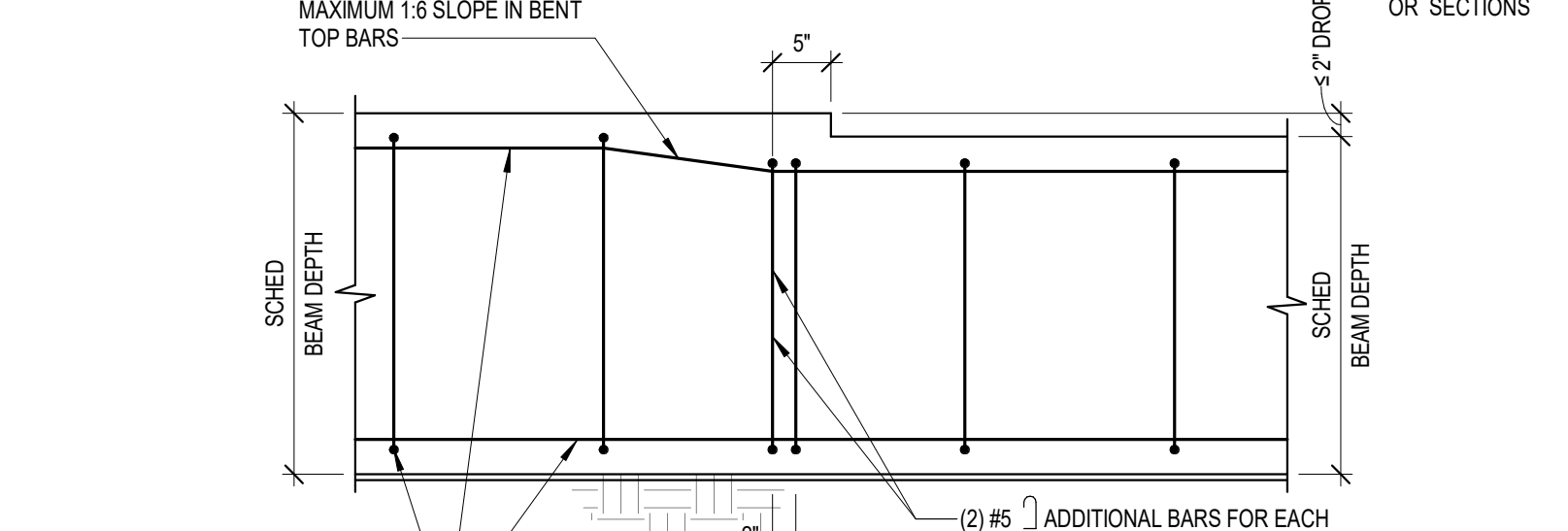
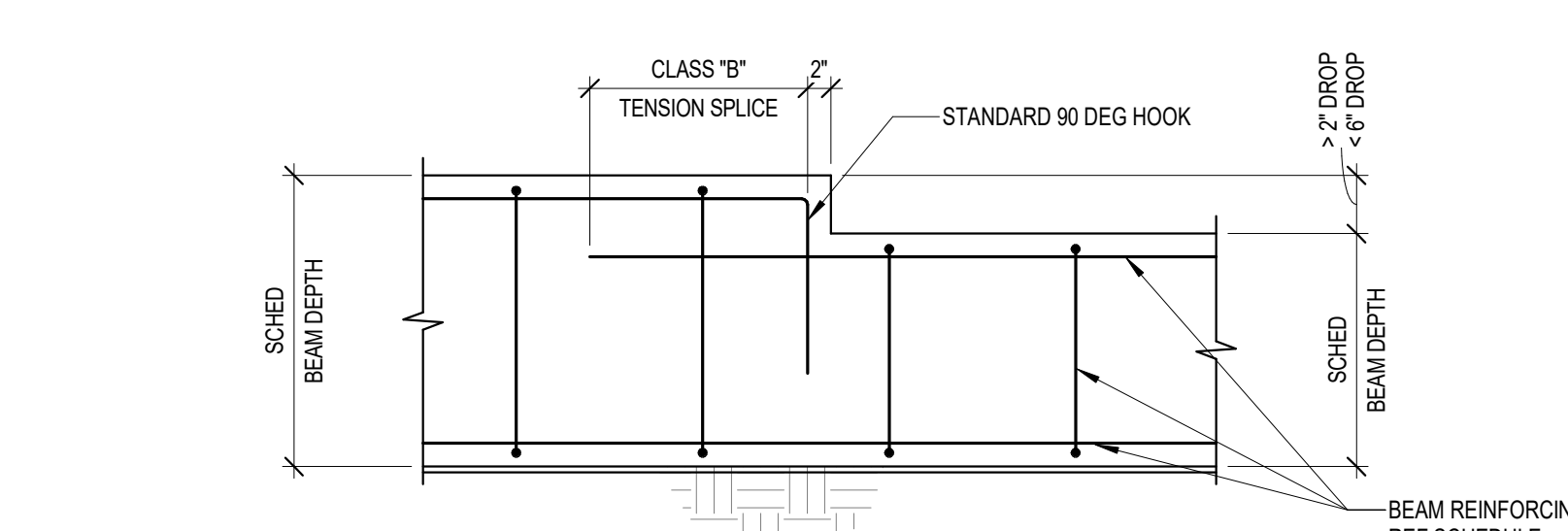
- NOTES:**
- TOP CONT BARS MAY BE SPLICED AT MID-SPAN OF BEAM ONLY. EXCEPT AT CANTILEVER SPANS, SPANS ADJACENT TO CANTILEVER SPANS AND SPANS WITH LENGTH NOT GREATER THAN ONE HALF OF THE ADJACENT SPAN LENGTH.
 - ALL HOOKS SHOWN ARE STANDARD HOOKS (90° OR 180°).
 - SPLICES IN SCHEDULED BARS LESS THAN 60" LENGTH ARE ONLY PERMITTED AT THE SPECIFIED LOCATIONS SHOWN. SPLICES IN SCHEDULED BARS MORE THAN 60" LENGTH SHALL BE REFERRED TO ENGINEER FOR APPROVAL.
 - BEAMS WITH DEPTH OF 30" OR LESS, DO NOT NEED INTERMEDIATE BARS UNLESS REQUIRED BY BEAM SCHEDULE, REF 1#S-302
 - REF TO DETAIL 6#S-302 FOR DROP IN TOP OR BOTTOM OF GRADE BEAM.



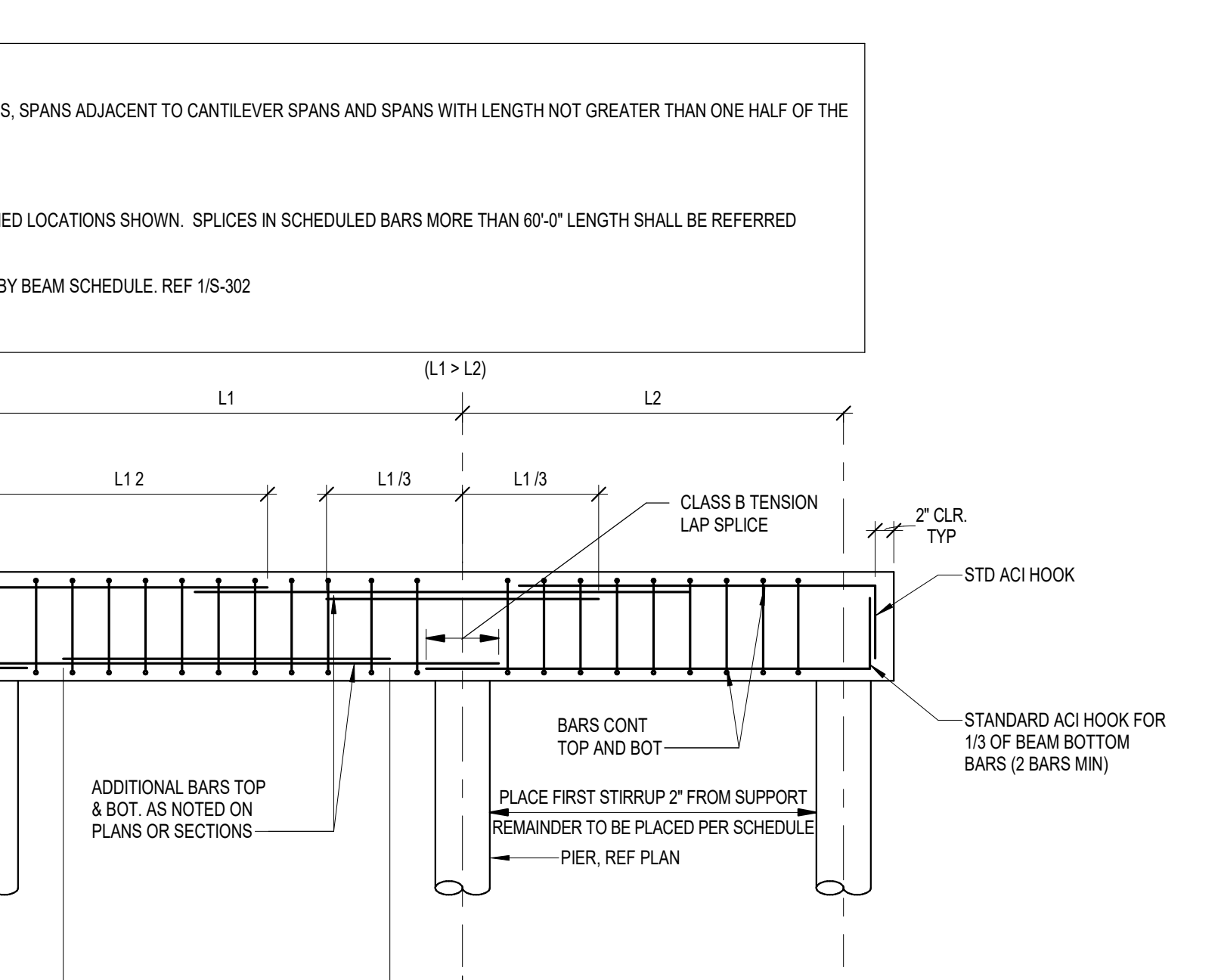
8 TYPICAL GRADE BEAM ELEVATION NO SCALE



2 TYPICAL GRADE BEAM STIRRUP TYPES 3/4" = 1'-0"

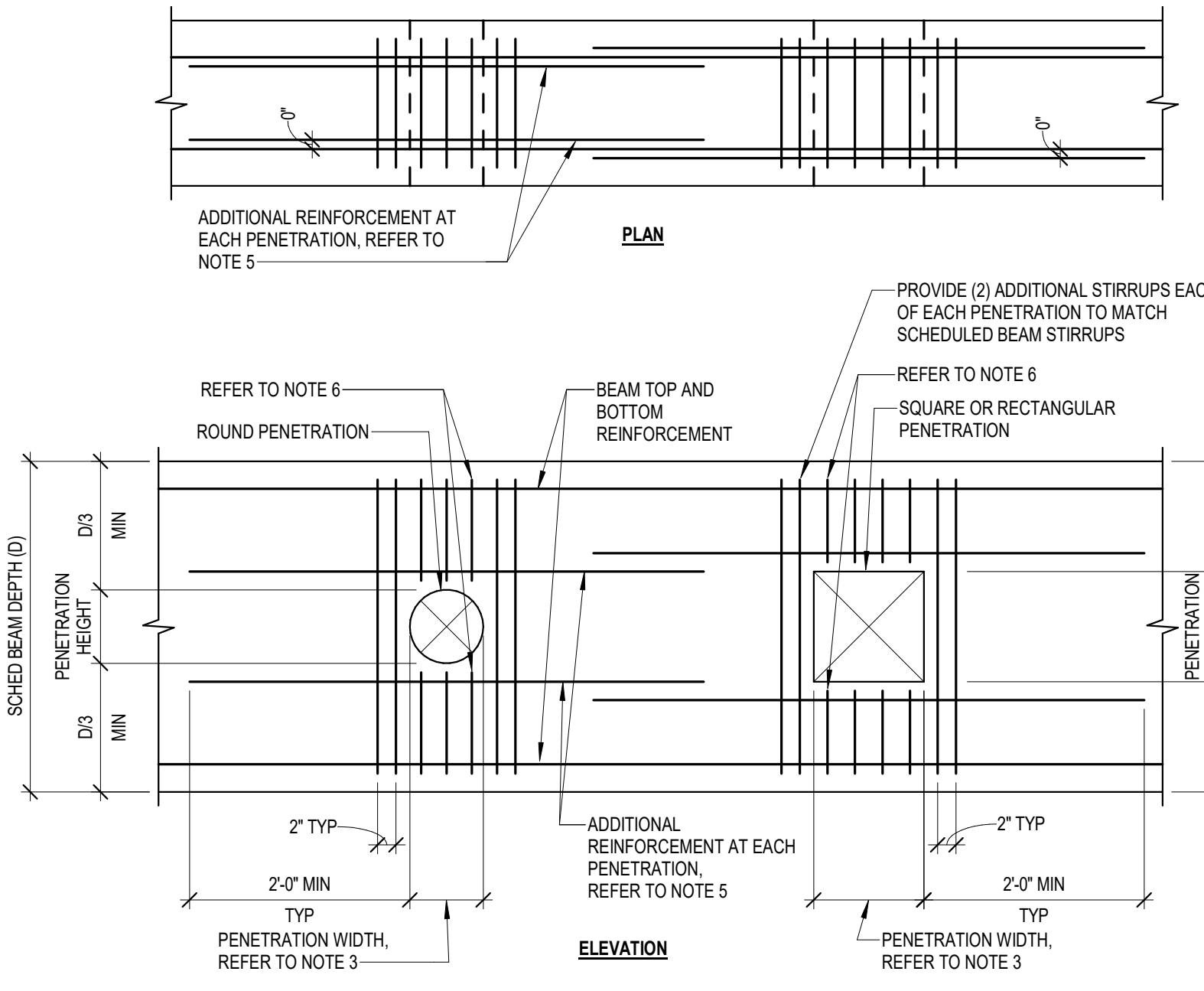


6 TYPICAL GRADE BEAM STEPDOWN DETAIL NO SCALE



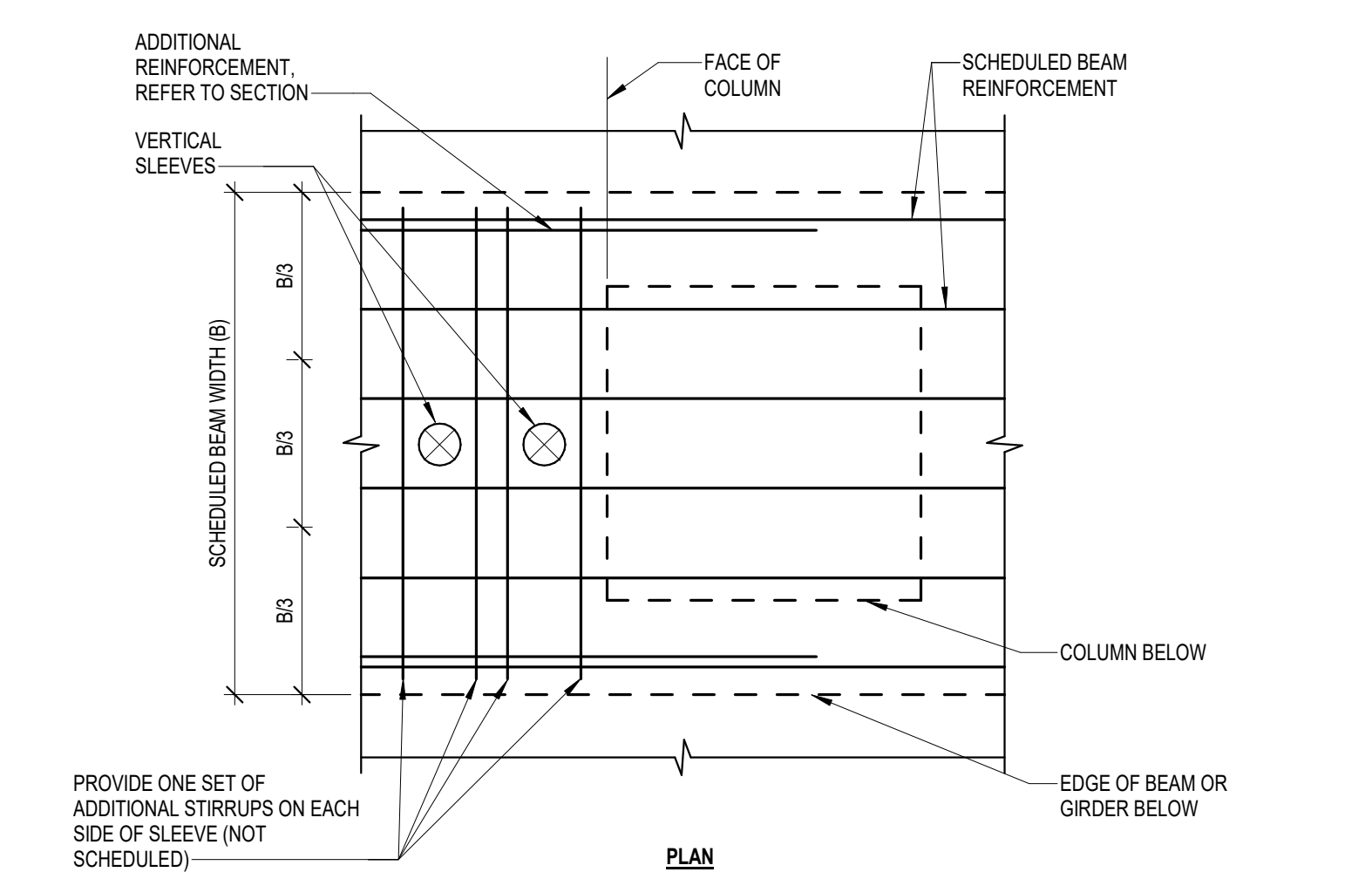
8 TYPICAL GRADE BEAM ELEVATION NO SCALE

- NOTES:**
- CLEAR SPACING BETWEEN PENETRATIONS SHALL BE 24" MINIMUM UNLESS NOTED OTHERWISE BY THE STRUCTURAL ENGINEER.
 - PENETRATIONS SHALL BE LOCATED ACCORDING TO THE FOLLOWING CRITERIA:
 - FOR BEAMS NOT SUPPORTING INTERSECTING BEAMS LOCATE PENETRATIONS WITHIN TWO FEET EITHER SIDE OF BEAM MIDSPAN.
 - FOR BEAMS SUPPORTING INTERSECTING BEAMS CHECK WITH STRUCTURAL ENGINEER.
 - PENETRATION WIDTH MUST NOT EXCEED PENETRATION HEIGHT, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
 - FOR LOCATIONS AND/OR SIZES OF PENETRATIONS NOT CONFORMING TO THE ABOVE CRITERIA AND NOT OTHERWISE DETAILED ON THE STRUCTURAL DRAWINGS, CONTRACTOR SHALL COORDINATE REQUIRED ADDITIONAL REINFORCEMENT WITH THE STRUCTURAL ENGINEER.
 - PROVIDE THE FOLLOWING REINFORCEMENT AT EACH SLEEVE, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS:
 - 1-#5 TOP AND BOTTOM AT BEAMS WITH WIDTHS LESS THAN 9"
 - 2-#5 TOP AND BOTTOM AT BEAMS WITH 4-LEG STIRRUPS
 - 4-#5 TOP AND BOTTOM AT BEAMS WITH 4-LEG STIRRUPS
 - "-#5 TOP AND BOTTOM AT BEAMS WITH "Y-LEG STIRRUPS.
 - PROVIDE ADDITIONAL STIRRUPS ABOVE AND BELOW PENETRATIONS AT SPACING NOT TO EXCEED ONE THIRD OF THE SCHEDULED STIRRUP SPACING, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
 - SCHEDULED BEAM STIRRUPS NOT SHOWN FOR CLARITY.

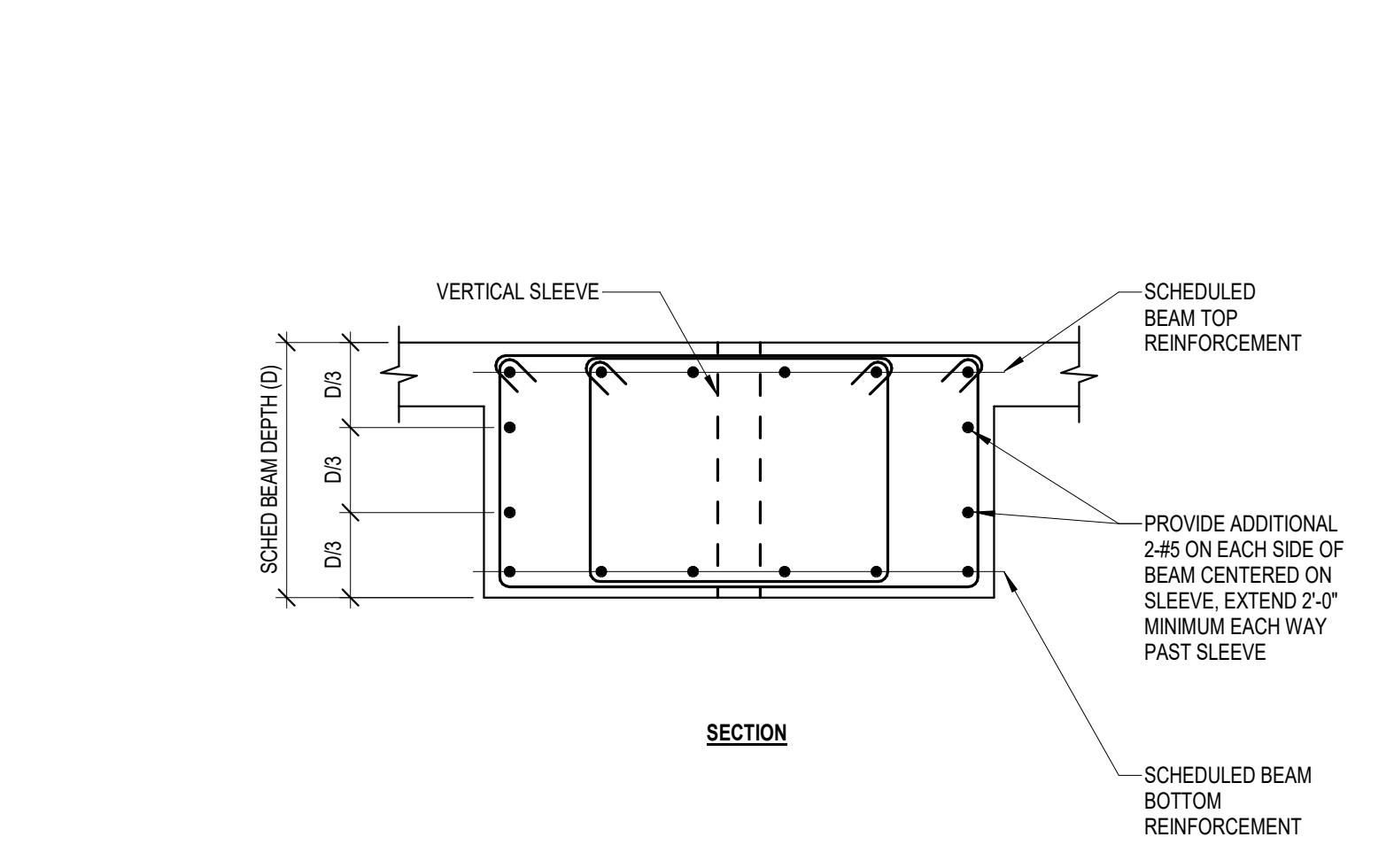


3 TYPICAL HORIZONTAL PENETRATION IN CONCRETE BEAM NO SCALE

- NOTES:**
- GENERAL CONTRACTOR SHALL COORDINATE REQUIRED BEAM SLEEVES WITH MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS. REQUIRED SLEEVES MAY OR MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS. GENERAL CONTRACTOR SHALL SUBMIT PLAN SHOWING LAYOUT OF ALL SLEEVES WITH FORMWORK SHOP DRAWING SUBMITTAL.
 - SLEEVES SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE SCHEDULED BEAM WIDTH.
 - CONTINUOUS BEAM REINFORCEMENT MAY BE SLIGHTLY DISPLACED (3" MAXIMUM) OR ADJACENT BARS BUNDLED (2 BAR BUNDLES MAXIMUM) TO FACILITATE SLEEVE INSTALLATION. DO NOT CUT, OFFSET, OR BEND REINFORCEMENT.
 - SLEEVES OCCURRING ON OPPOSITE SIDES OF A COLUMN MUST BE IN LINE.
 - THE OUTSIDE DIAMETER OF A SLEEVE MAY NOT EXCEED 15% OF THE SCHEDULED WIDTH OF THE BEAM THROUGH WHICH IT MUST PASS.
 - THE CONTRACTOR SHALL CONTACT THE ENGINEER OF RECORD WHEN A SLEEVE SIZE OR LOCATION DOES NOT MEET THE ABOVE CONDITIONS.
 - SCHEDULED BEAM STIRRUPS NOT SHOWN FOR CLARITY.

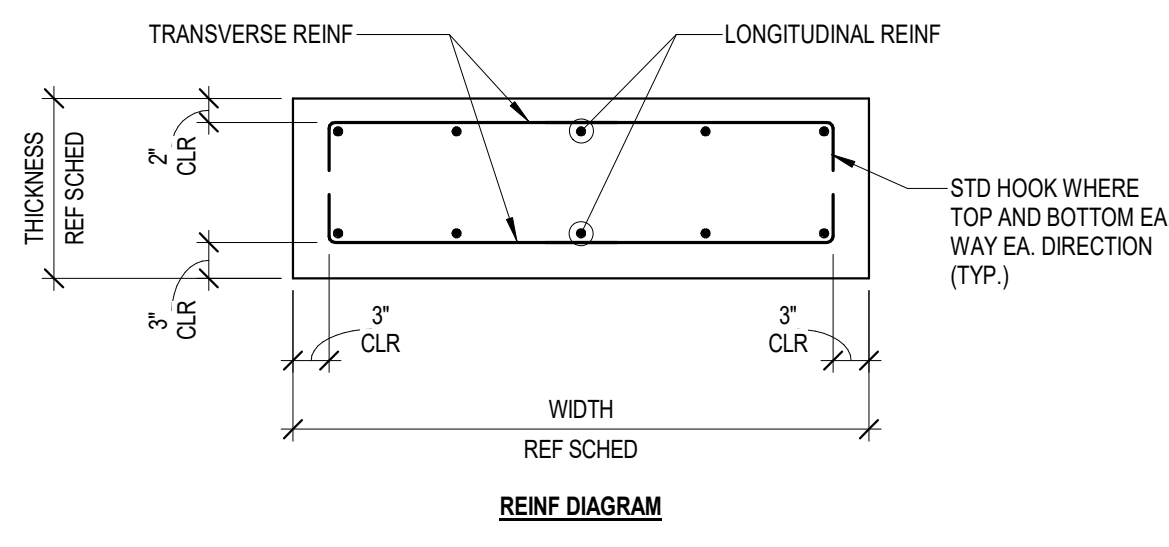


9 TYPICAL VERTICAL PENETRATION IN CONCRETE BEAM NO SCALE



FOOTINGS:

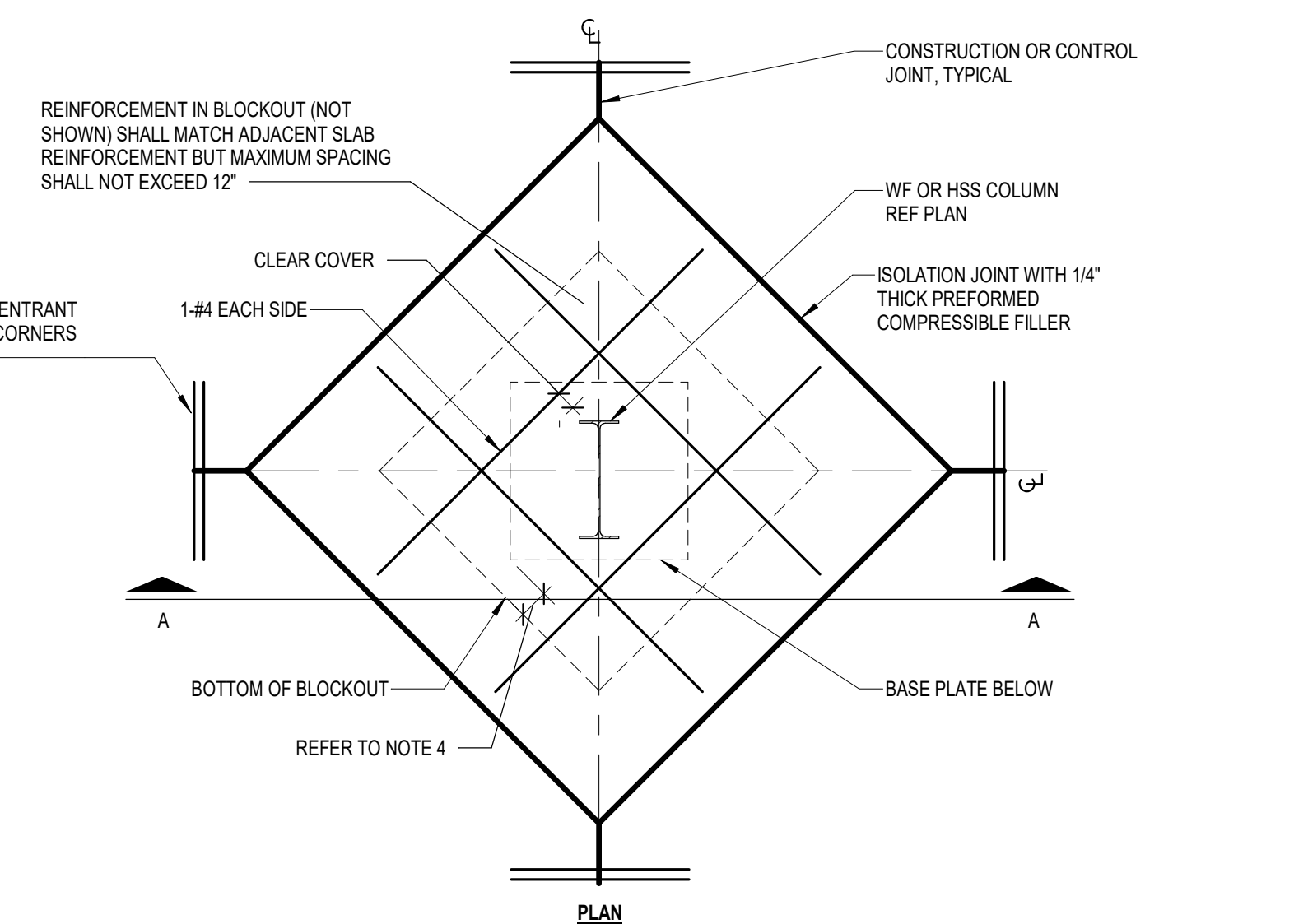
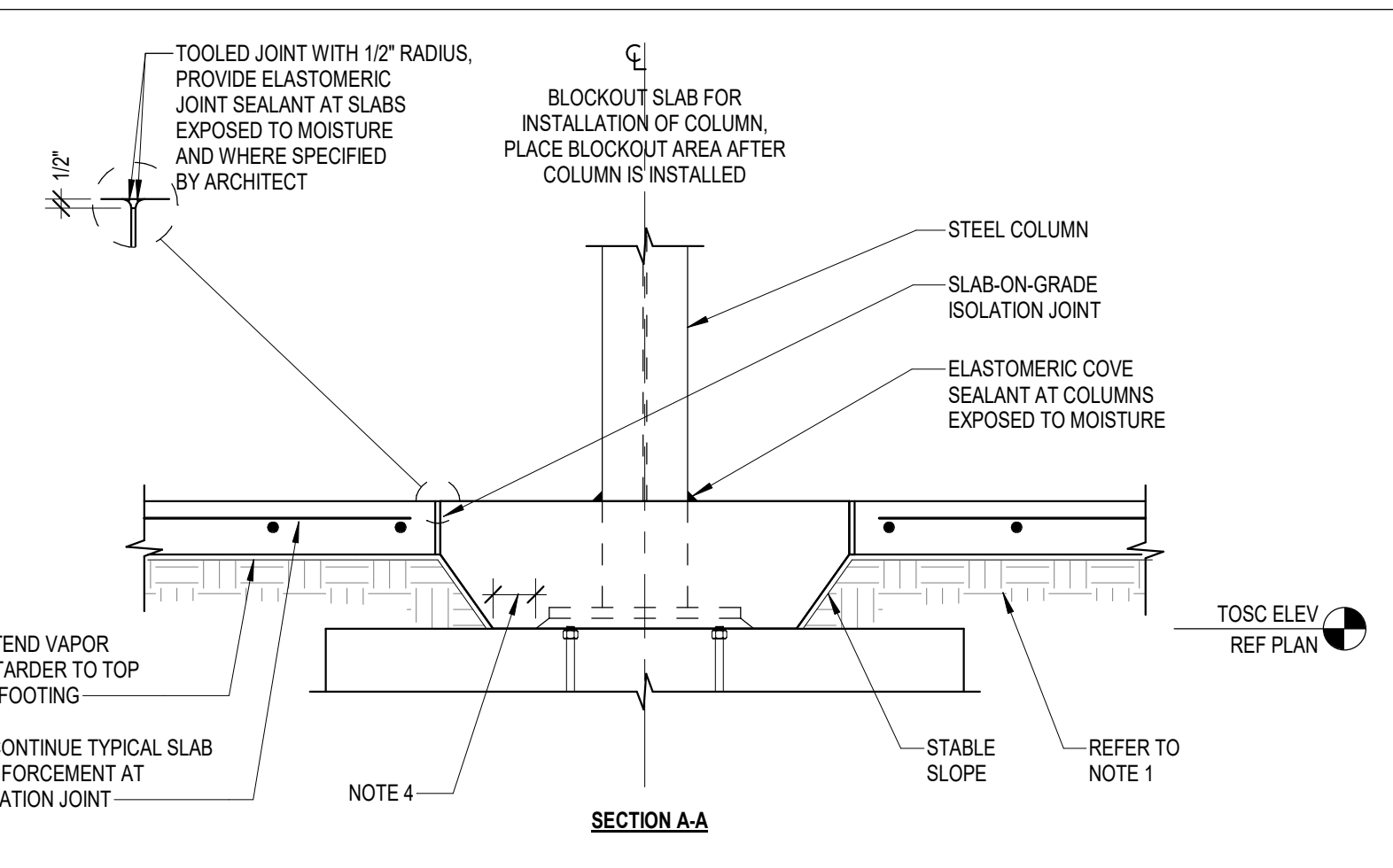
- FOOTING DESIGN IS BASED UPON THE FOLLOWING CRITERIA. REFER TO TYPICAL FOOTING DETAIL FOR FOOTING SCHEDULE AND REINFORCING DISTRIBUTION OF FOOTING TYPES AS INDICATED ON PLANS.
 - NET ALLOWABLE SOIL BEARING PRESSURE..... 2,000 PSF IN NATIVE SOILS / SANDS
2,500 PSF IN SELECT FILL
 - MINIMUM FOOTING BEARING DEPTH..... 2'-0" FT BELOW FINAL EXTERIOR GRADE
 - MINIMUM WALL FOOTING WIDTH..... 24 INCHES
 - MINIMUM COLUMN FOOTING WIDTH..... 24 INCHES
 - COEFFICIENT OF BASE FRICTION AT FOOTING BASE..... 3
 - ALLOWABLE PASSIVE EARTH PRESSURE FOR FOOTINGS BEARING AGAINST NATIVE SANDS OR SAND BACKFILL..... 250 PSF/FT
 - ALLOWABLE PASSIVE EARTH PRESSURE FOR FOOTINGS BEARING AGAINST NATIVE CLAYS OR PROPERLY PLACED AND COMPACTED CLAY BACKFILL..... 250 PSF
 - DEPTH OF SOIL WHERE PASSIVE EARTH PRESSURE SHALL BE NEGLECTED..... 1'-0" FT FROM ADJACENT GRADE
 - BEARING STRATUM..... NATIVE SANDY SOILS OR PROPERLY COMPACTED SELECT FILL
- FOOTINGS NOT SPECIFICALLY LOCATED ON THE PLAN SHALL BE LOCATED ON CENTERLINE OF THE COLUMN ABOVE, WHERE NO COLUMN OCCURS, LOCATE FOOTING ON CENTERLINE OF WALL OR BEAM.
- PROVIDE DOWELS FROM FOOTINGS INTO CONCRETE ABOVE PER THE TYPICAL FOOTING DETAIL.
- ELEVATION OF TOP OF FOOTING IS NOTED ON DRAWINGS.
- REFERENCE PLANS AND FOOTING SCHEDULE FOR FOOTING SIZE, REINFORCING, AND DEPTH OF BEARING STRATUM.
- ALL FOOTINGS SHALL BE INSPECTED BY A REPRESENTATIVE OF THE PROJECT'S GER IN ORDER TO ENSURE THAT THE BEARING STRATUM IS PROPER AND IN ACCORDANCE WITH THE RECOMMENDATIONS FIVEN IN THE GEOTECHNICAL REPORT.
- USE TEMPLATES TO SET DOWELS AND ANCHOR BOLTS IN FOOTINGS. PROVIDE DETAILS OF THE TEMPLATES IN THE FOOTING SHOP DRAWINGS. REMOVE THE TEMPLATE COMPLETELY FROM THE TOP OF THE FOOTING PRIOR TO SUPERSTRUCTURE CONSTRUCTION.
- UNLESS REQUIRED BY THE PROJECT'S GER TO BE FORM-SIDED, THE FOOTING EXCAVATIONS SHALL BE MADE TO NEAT LINES AND SHALL BE FREE OF LOOSE OR WET MATERIALS. CONCRETE CAN BE PLACED DIRECTLY AGAINST THE SOIL WITHOUT FORMING.
- PLACE CONCRETE IN FOOTING EXCAVATION WITHIN 8 HOURS OF FINAL EXCAVATION OR AS SPECIFIED IN THE GEOTECHNICAL REPORT.



SPREAD FOOTING SCHEDULE

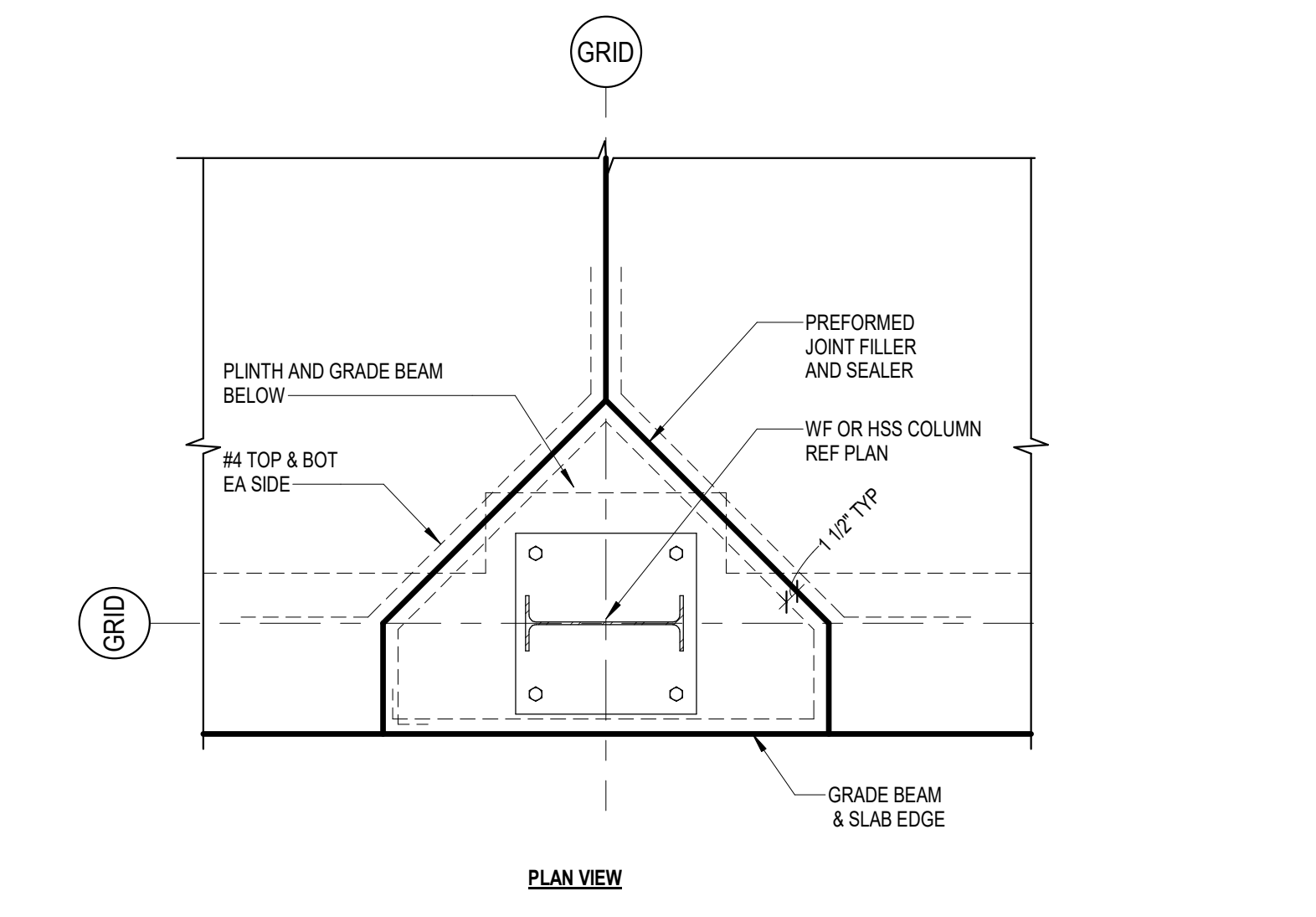
MARK	WIDTH (FT-IN)	LENGTH (FT-IN)	THICKNESS (FT-IN)	LONGITUDINAL TOP REINF	LONGITUDINAL BOT REINF	TRANSVERSE TOP REINF	TRANSVERSE BOT REINF	REMARKS
F1	15'-0"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F2	11'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F3	12'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F4	12'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F5	11'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F6	12'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	
F7	20'-6"	20'-0"	2'-6"	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	#7@10" OC EW	

1 TYPICAL FOOTING REINF DIAGRAM AND SCHEDULE
NO SCALE

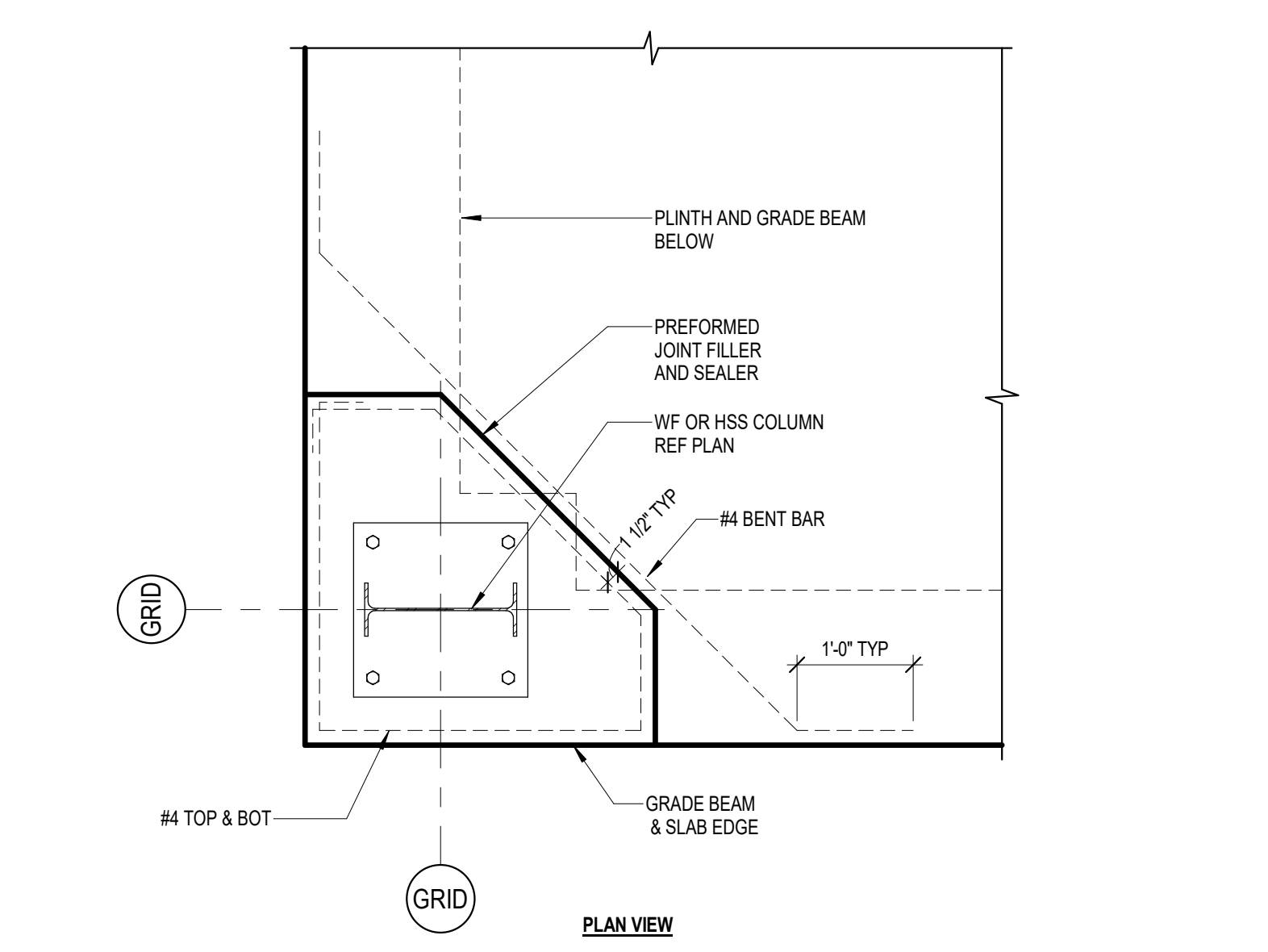


- NOTES:**
- REFER TO "TYPICAL SLAB-ON-GRADE SUBGRADE PREPARATION" FOR SUBGRADE PREPARATION REQUIREMENTS INCLUDING LOCATION OF VAPOR RETARDER RELATIVE TO SLAB AND GRANULAR BASE AND WHETHER FINE-GRADE GRANULAR MATERIAL IS REQUIRED.
 - REFER TO "TYPICAL CONSTRUCTION AND CONTROL JOINTS - SLAB-ON-GRADE" FOR CONSTRUCTION AND CONTROL JOINT REQUIREMENTS.
 - CONTRACTOR TO COORDINATE REQUIRED SIZE OF BLOCKOUT FOR STRUCTURAL STEEL COLUMNS WITH STEEL ERECTOR. SUBMIT THE DESIRED BLOCKOUT SIZE TO ARCHITECT FOR APPROVAL.
 - BLOCKOUT SHALL BE SIZED TO PROVIDE 3" MINIMUM CONCRETE COVER ALL AROUND COLUMN AND BASE PLATE.
 - THE BLOCKOUT SHALL BE KEYWAYED WITH RE-ENTRANT BARS.

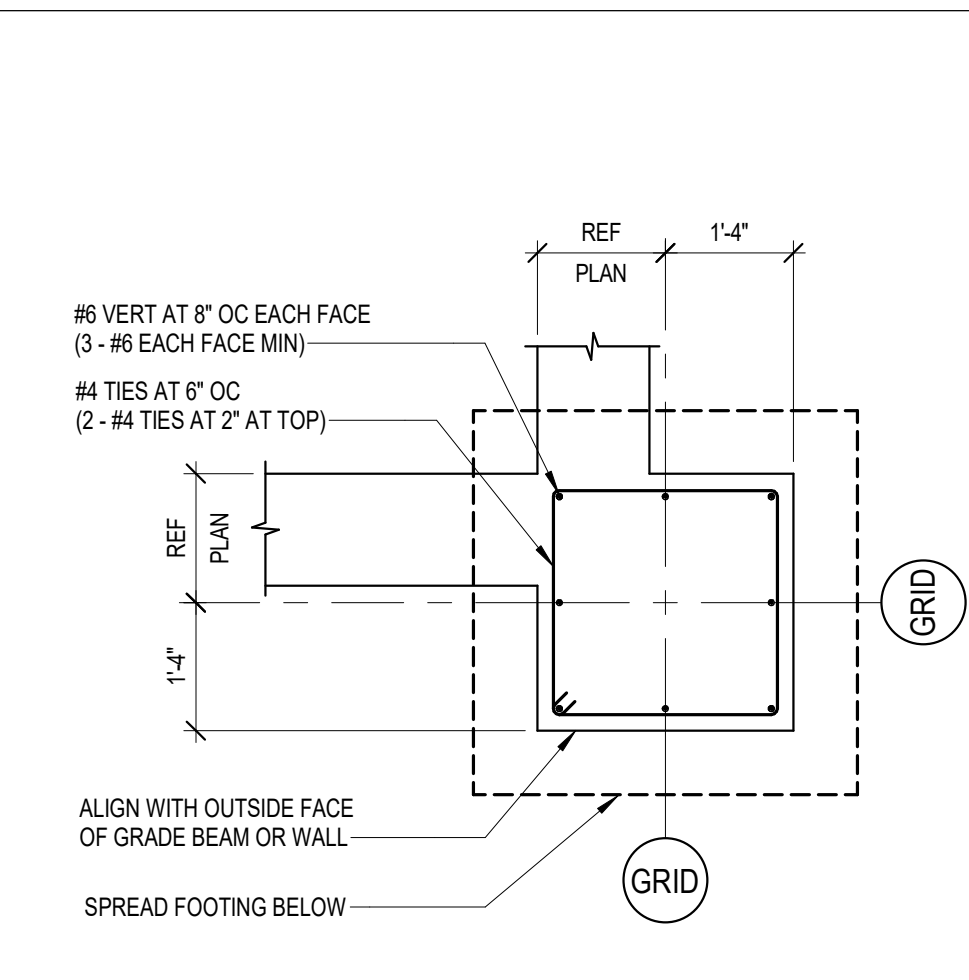
2 TYPICAL SLAB-ON-GRADE INTERIOR BLOCKOUT DETAIL
NO SCALE



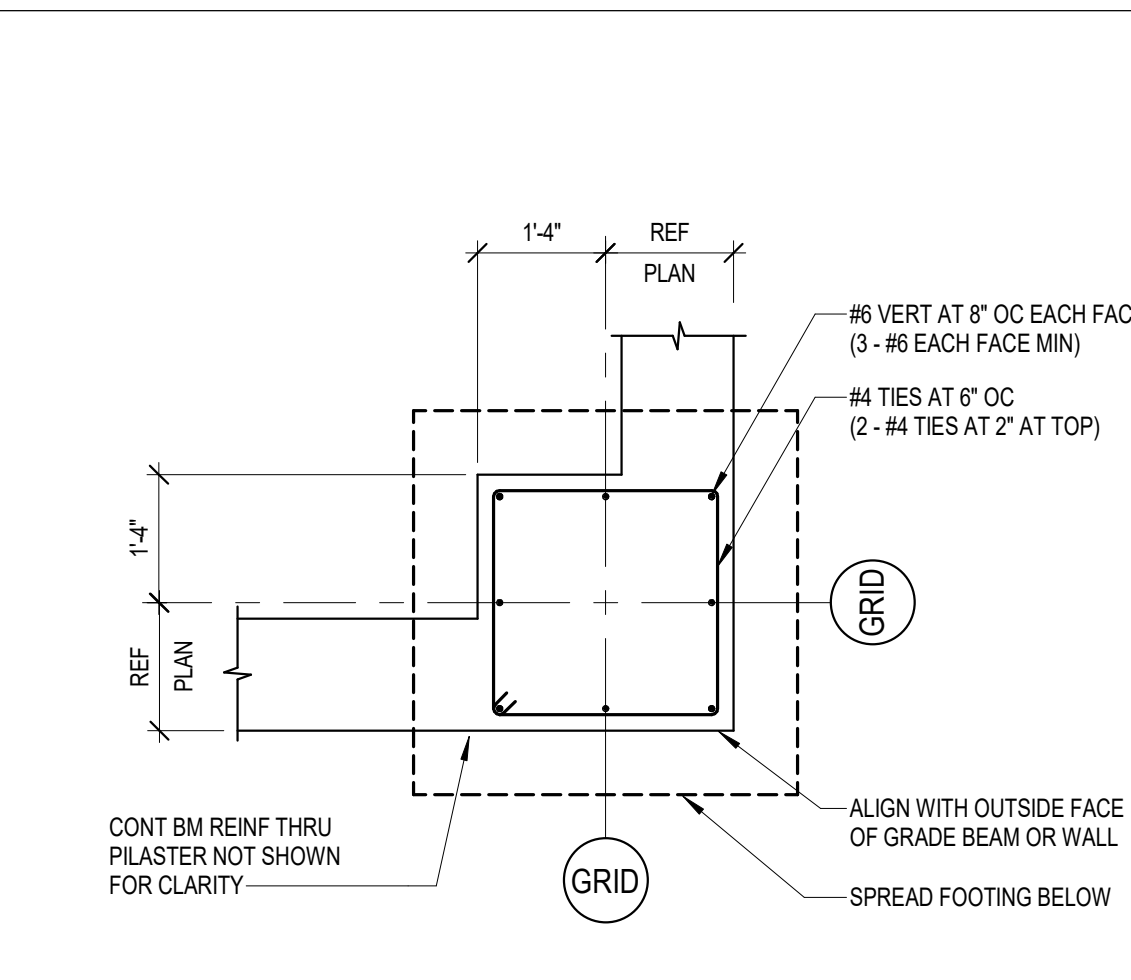
4 TYPICAL SLAB-ON-GRADE PERIMETER BLOCKOUT DETAIL
NO SCALE



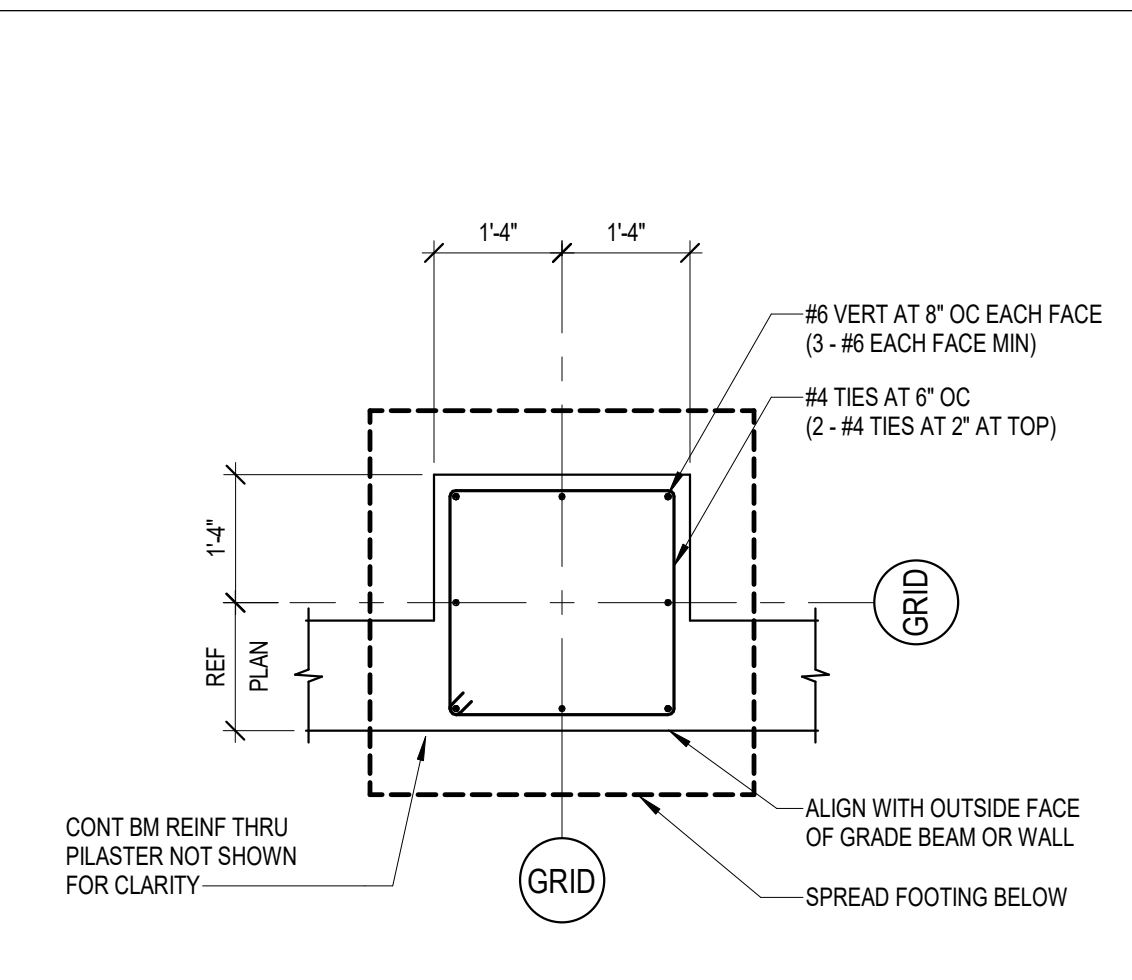
5 TYPICAL SLAB-ON-GRADE CORNER BLOCKOUT DETAIL
NO SCALE



INSIDE CORNER PILASTER

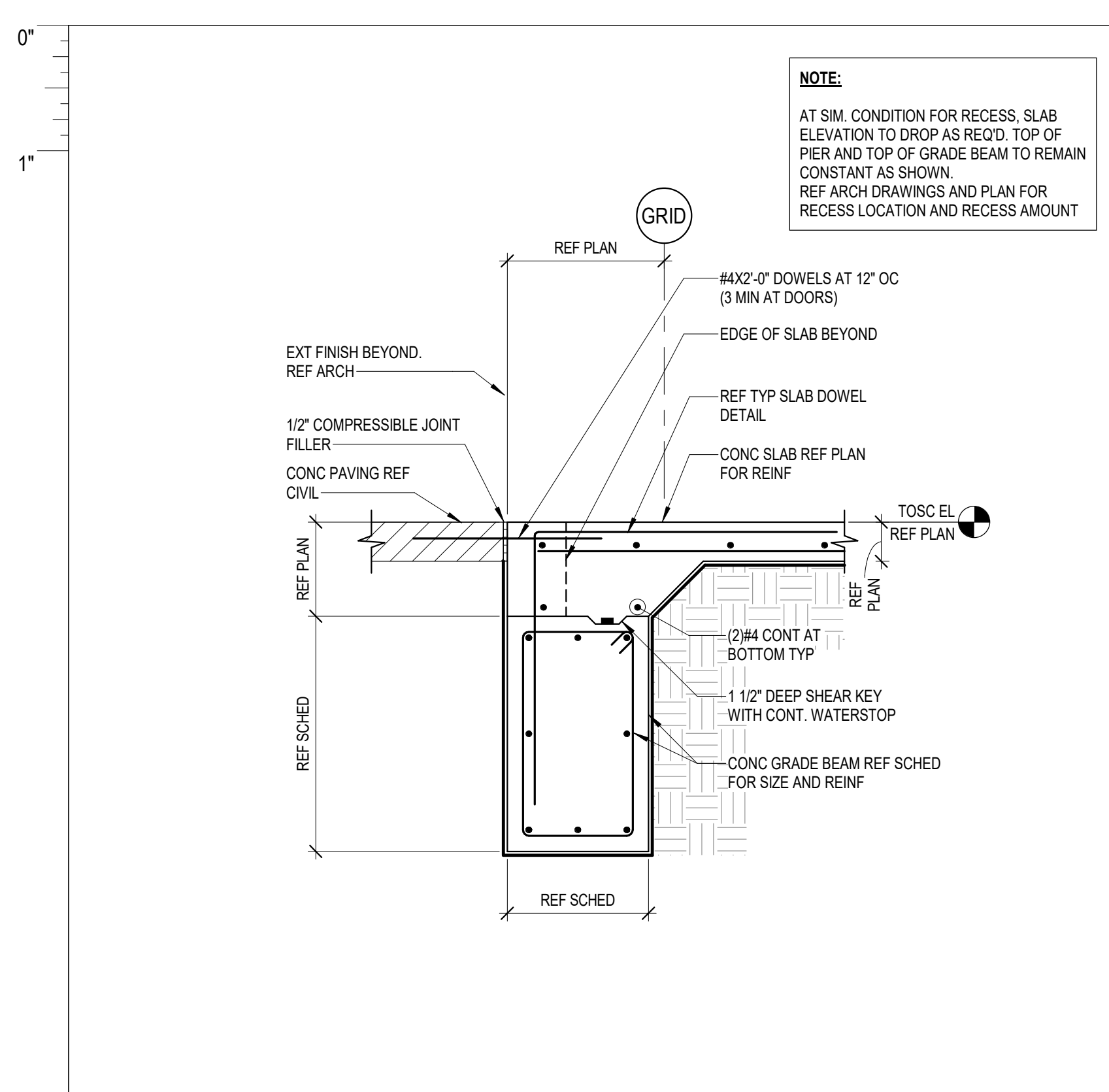


OUTSIDE CORNER PILASTER

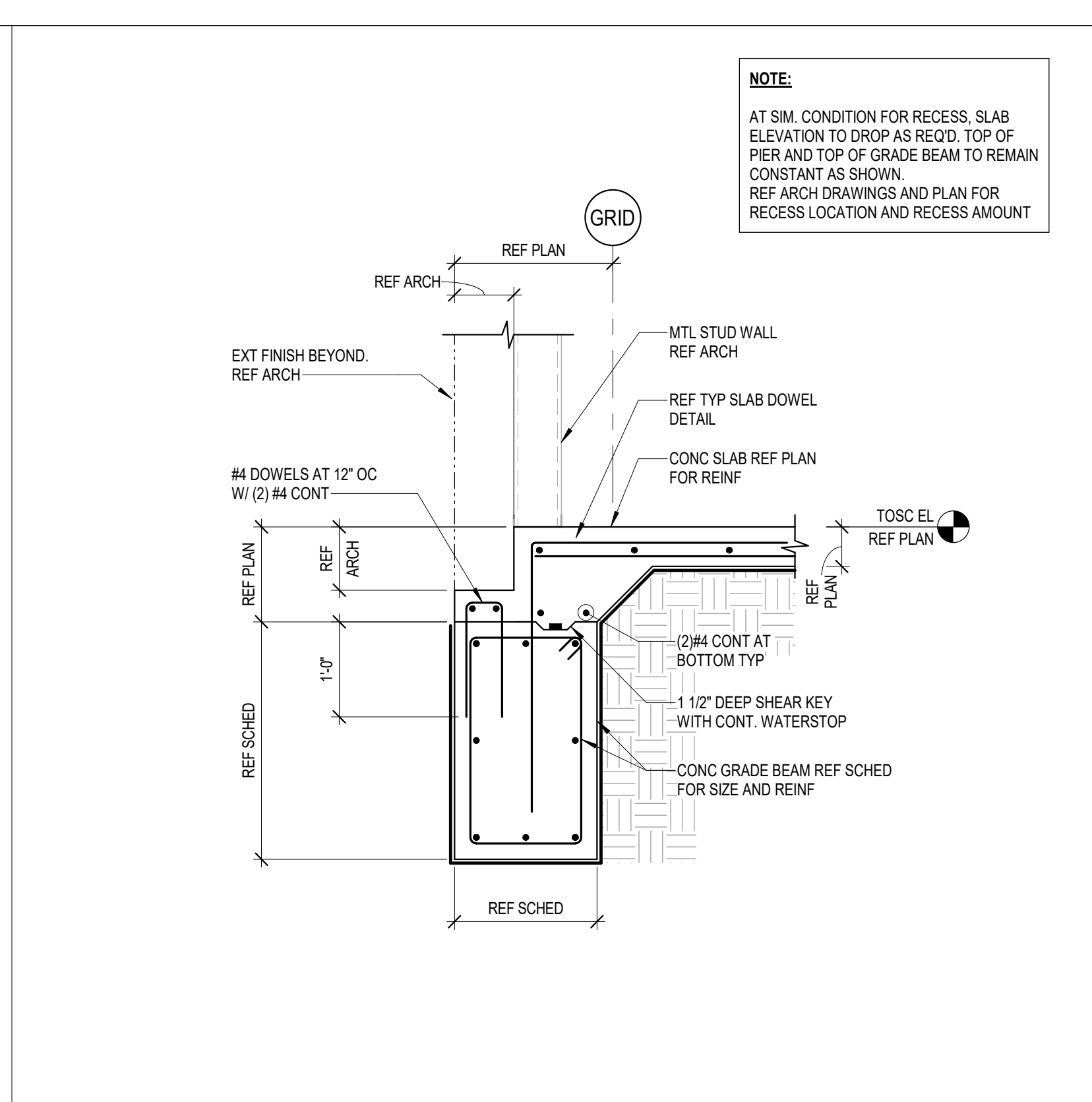


INTERIOR PILASTER

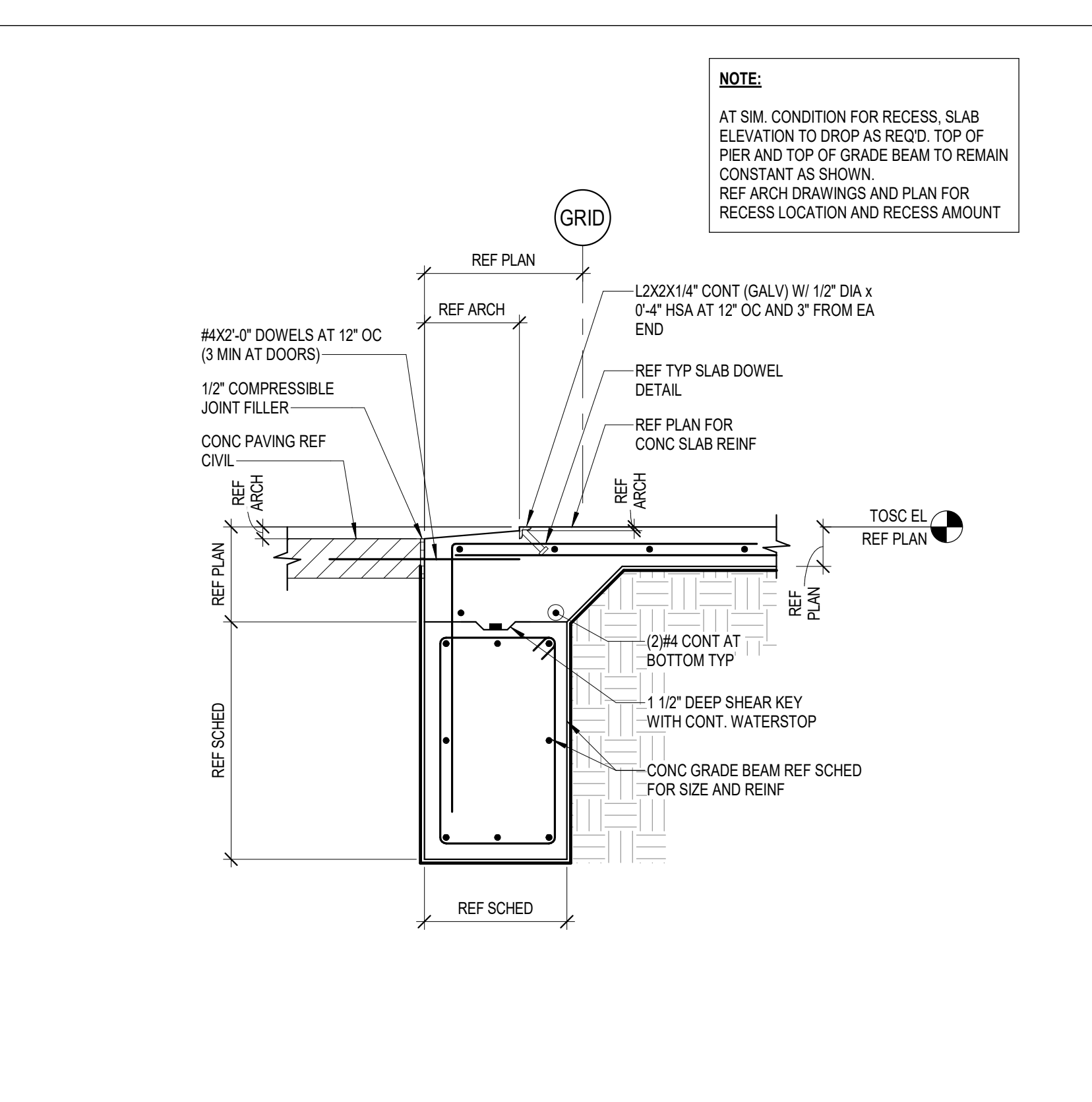
3 TYPICAL PILASTER DETAILS AT SPREAD FOOTING
NO SCALE



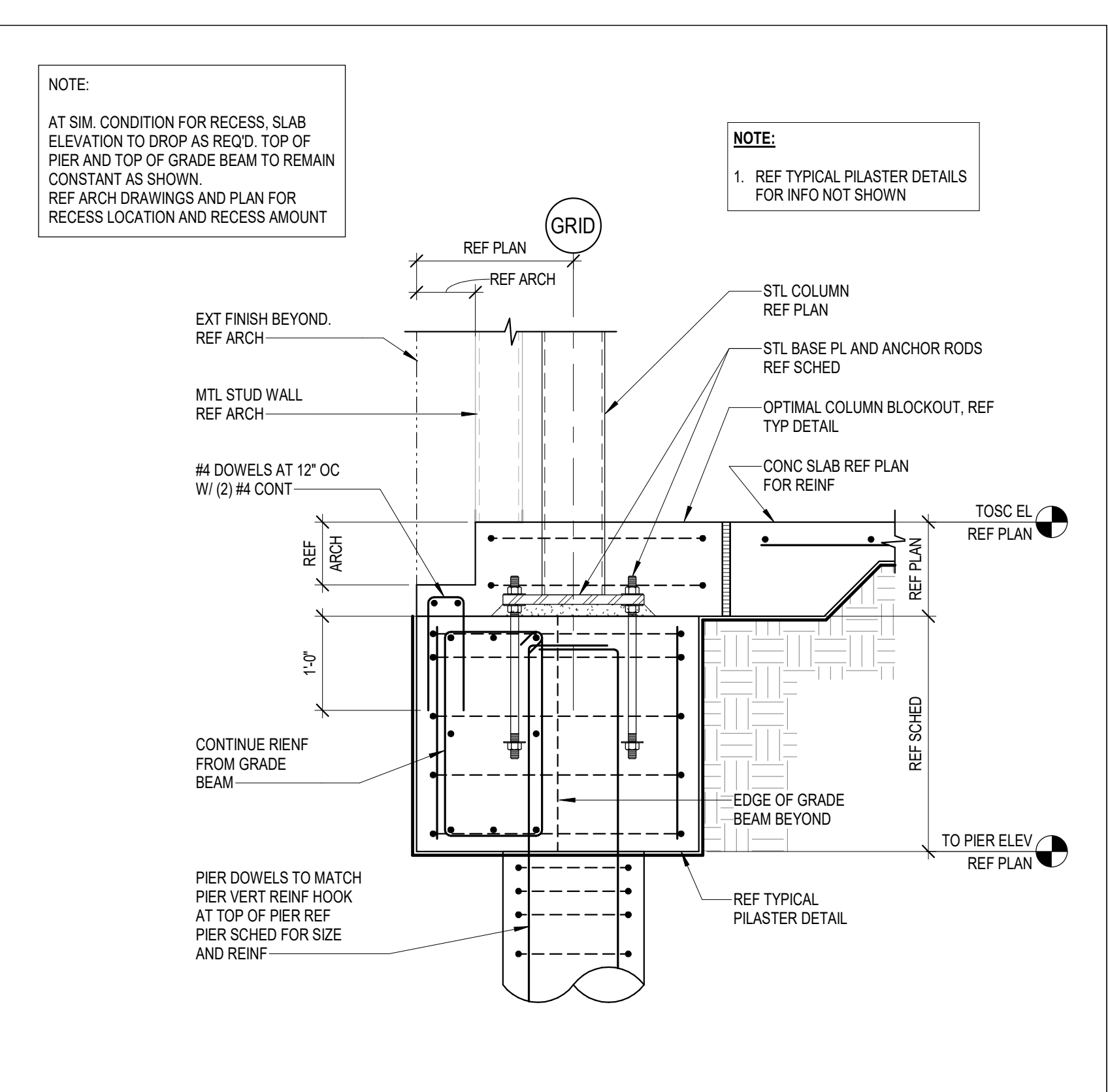
1 EXTERIOR GRADE BEAM AT DOOR
3/4" = 1'-0"



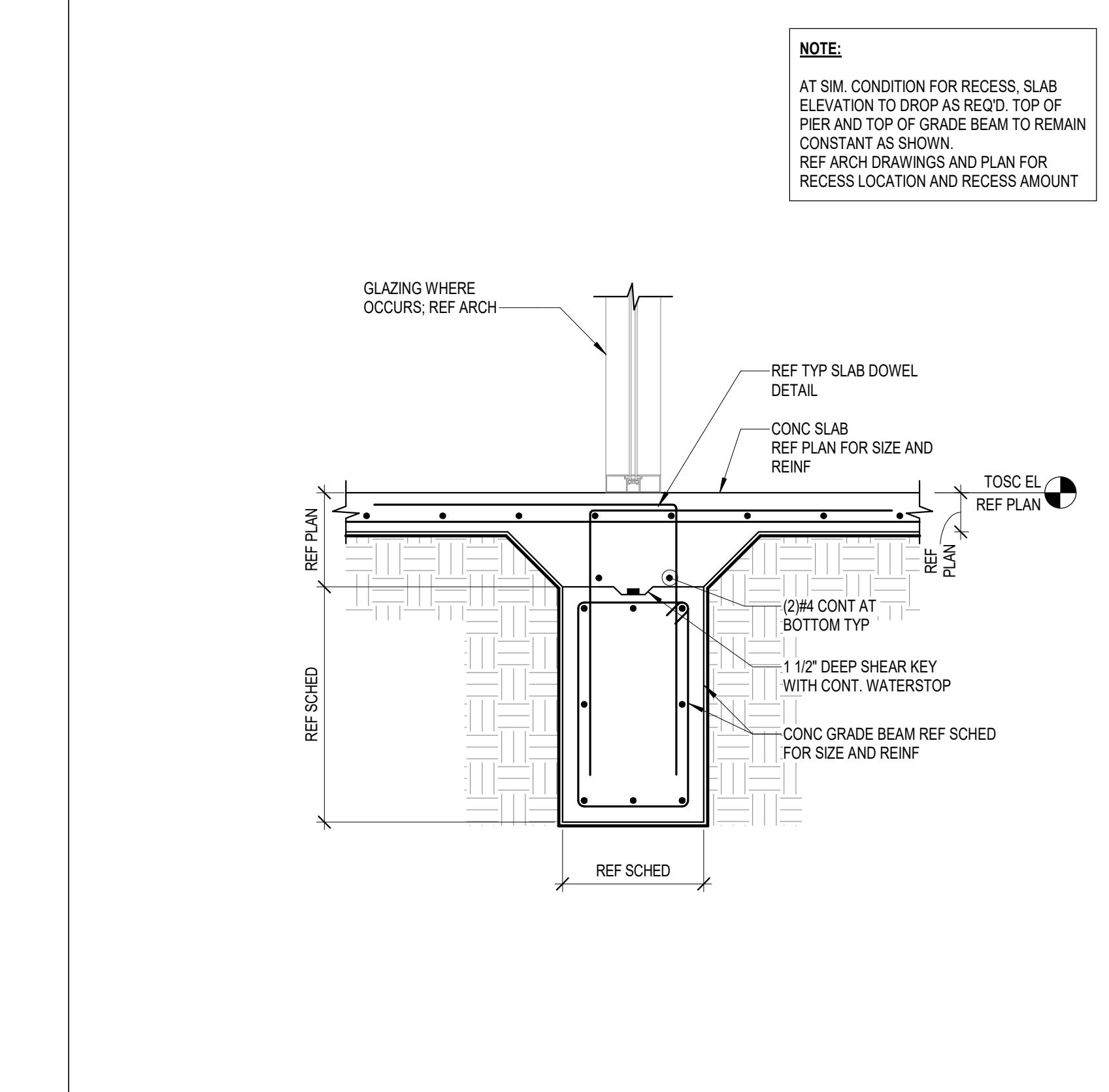
2 EXTERIOR GRADE BEAM AT LEDGE
3/4" = 1'-0"



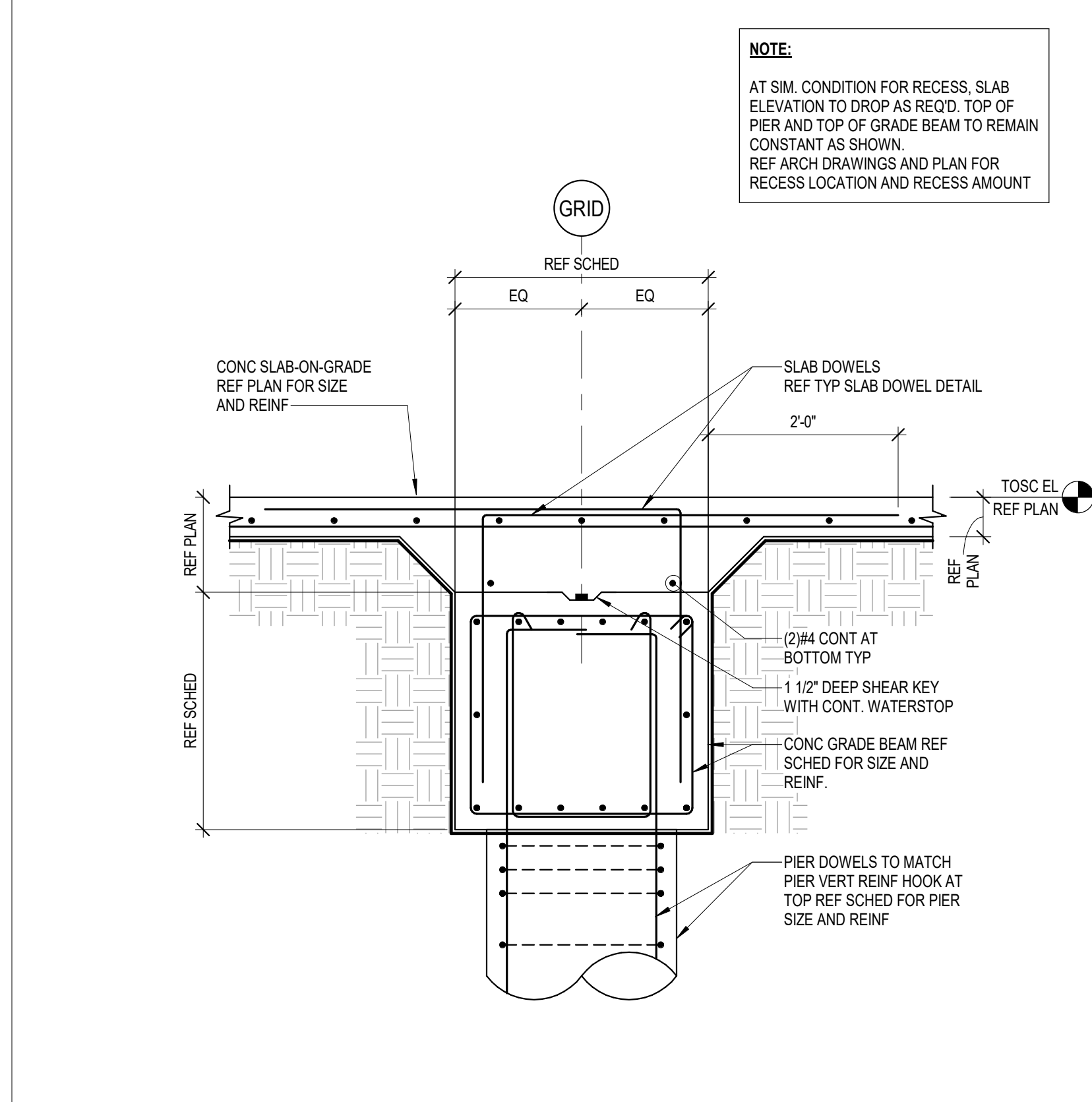
3 EXTERIOR GRADE BEAM AT OVERHEAD DOOR
3/4" = 1'-0"



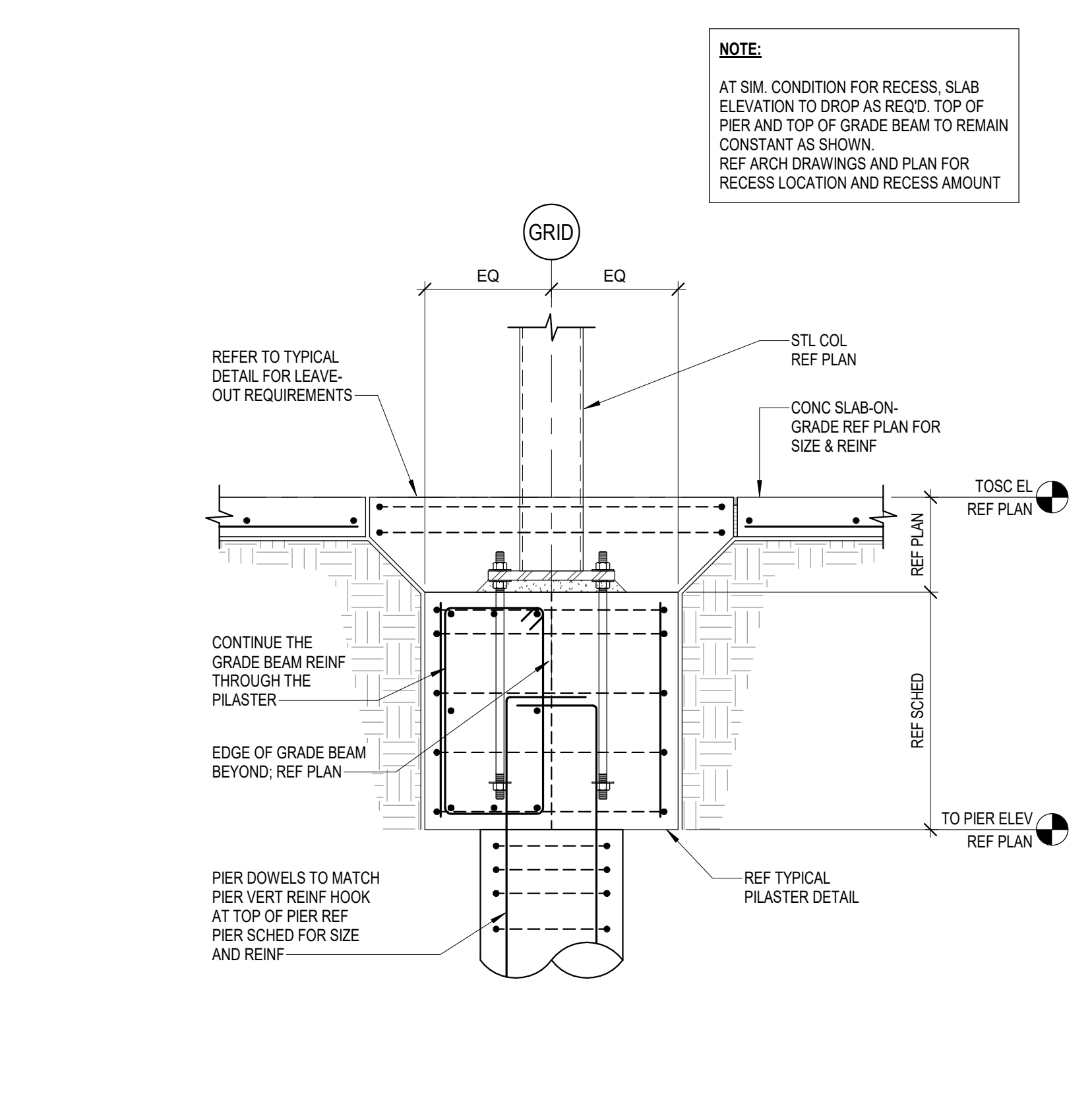
4 EXTERIOR PILASTER AT COLUMN
3/4" = 1'-0"



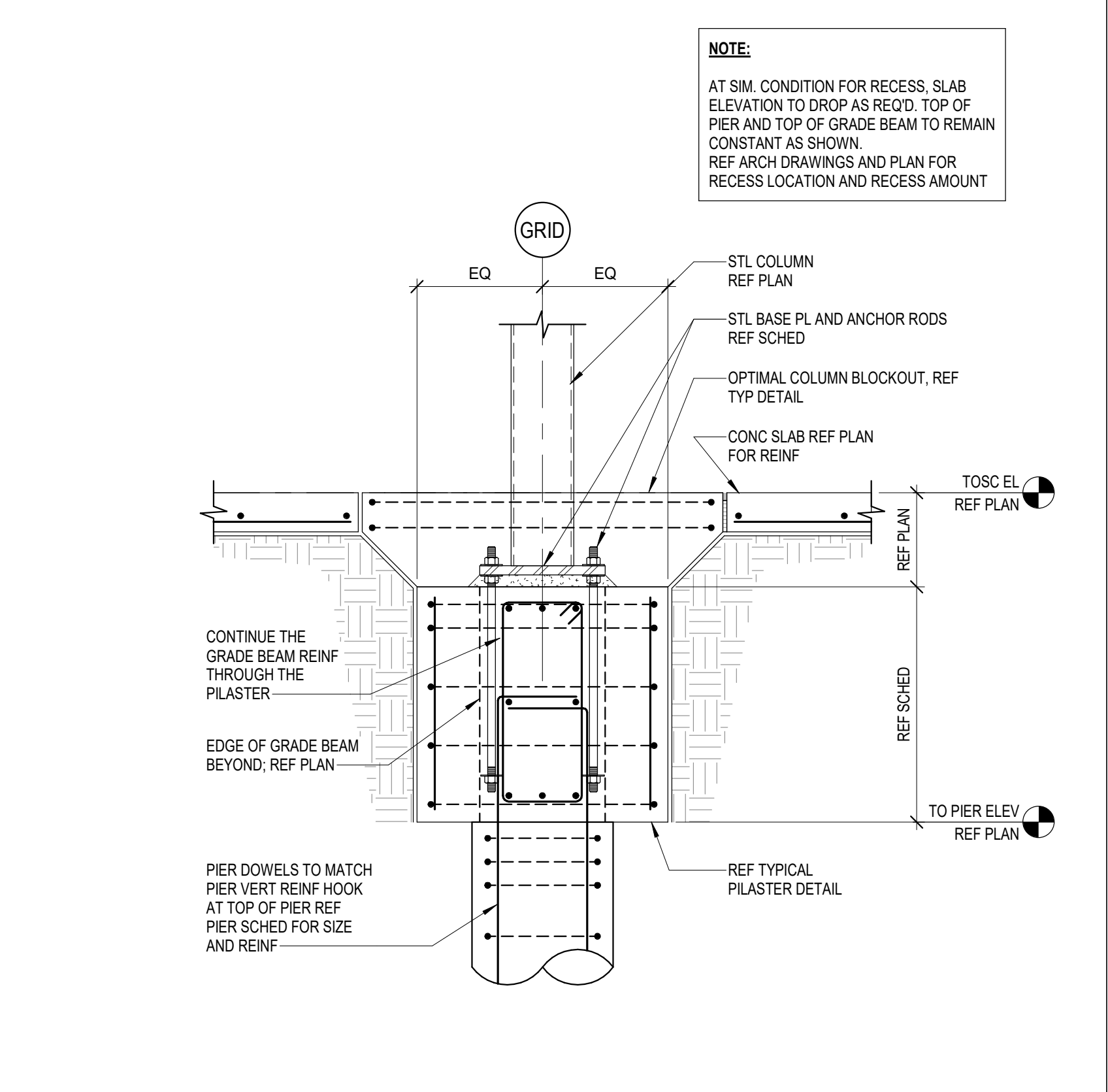
5 INTERIOR GRADE BEAM AT GLAZING
3/4" = 1'-0"



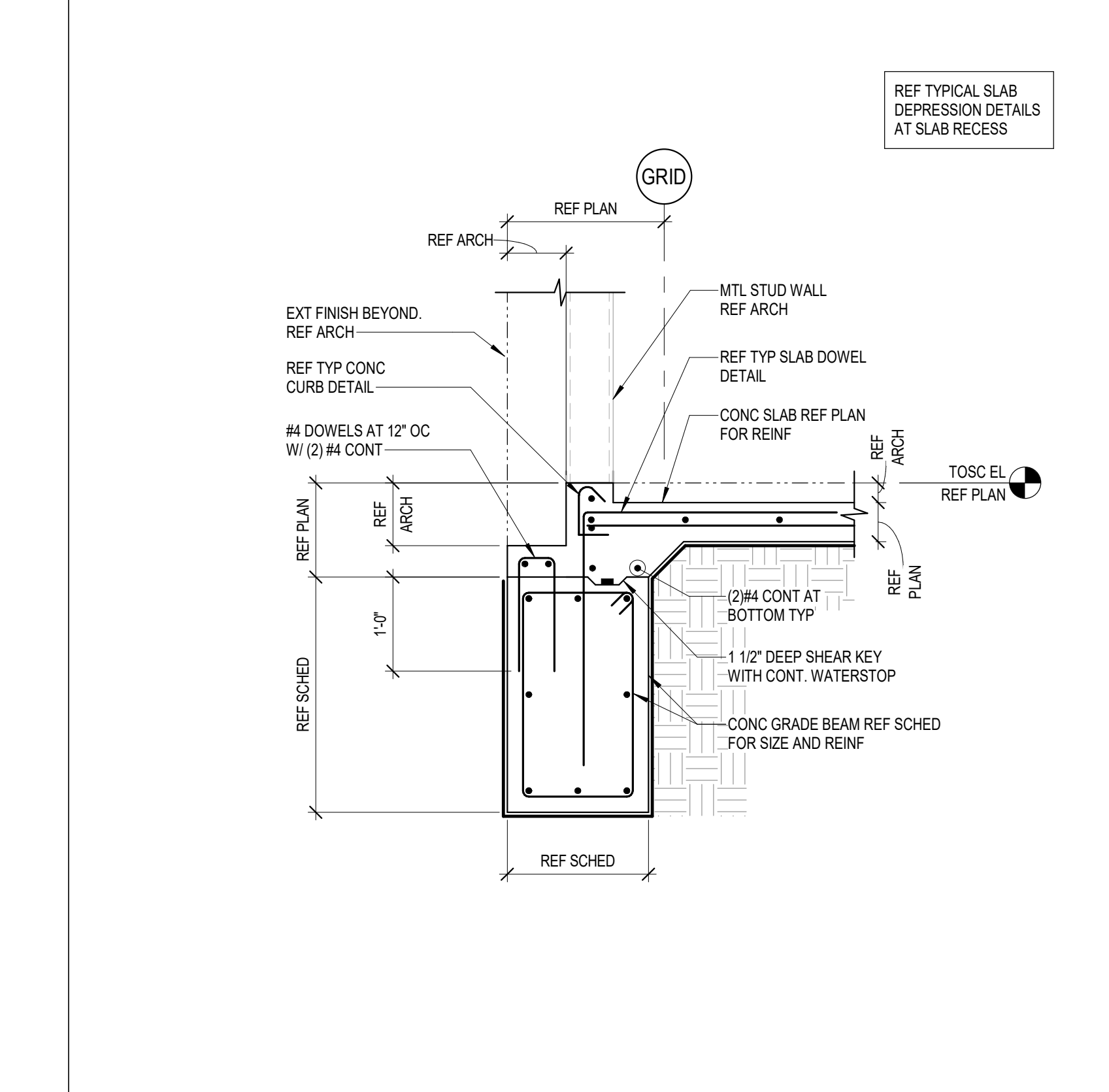
6 INTERIOR GRADE BEAM AT PIER
3/4" = 1'-0"



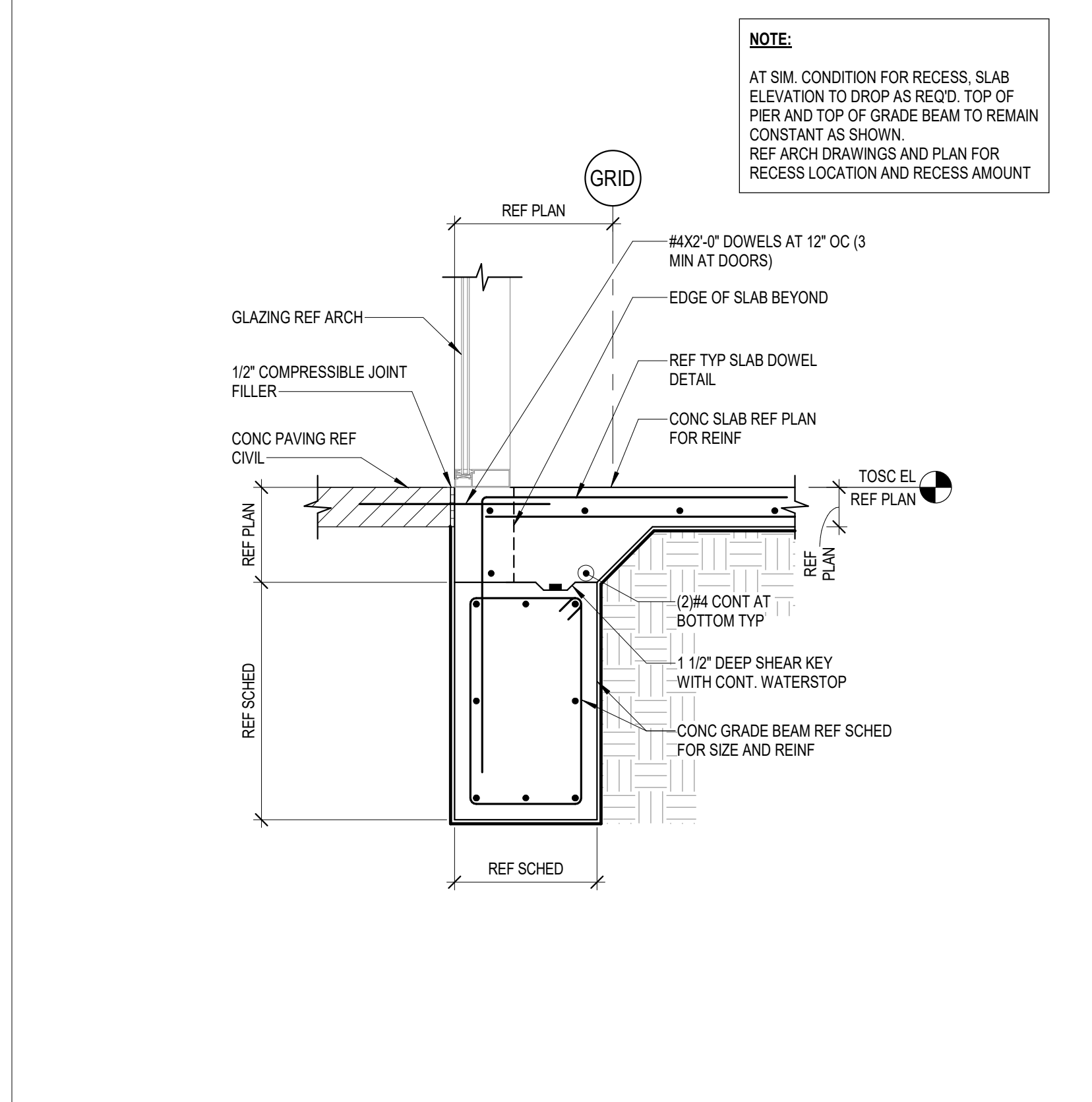
7 INTERIOR COLUMN AT PILASTER
3/4" = 1'-0"



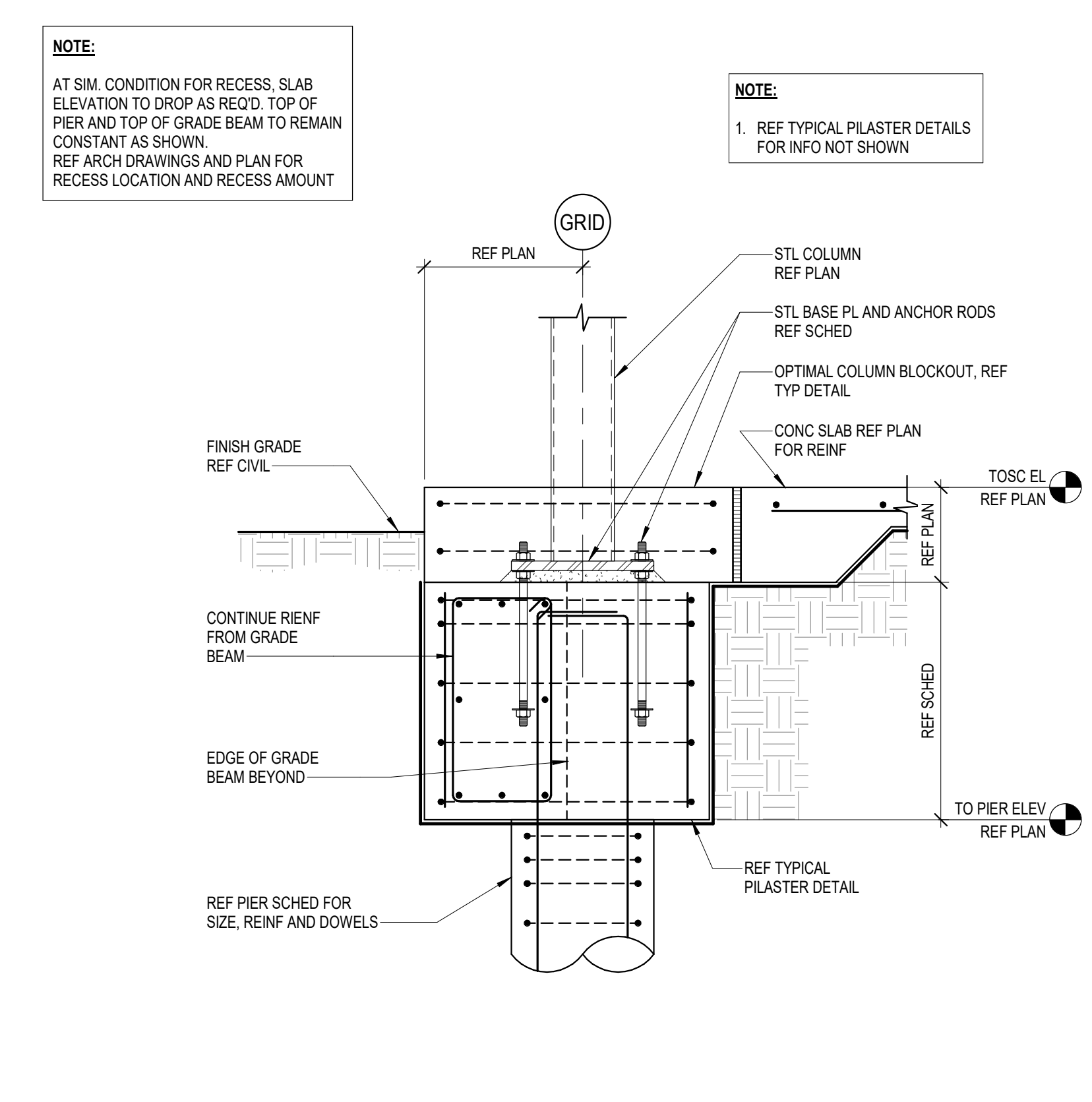
8 INTERIOR COLUMN AT PILASTER
3/4" = 1'-0"



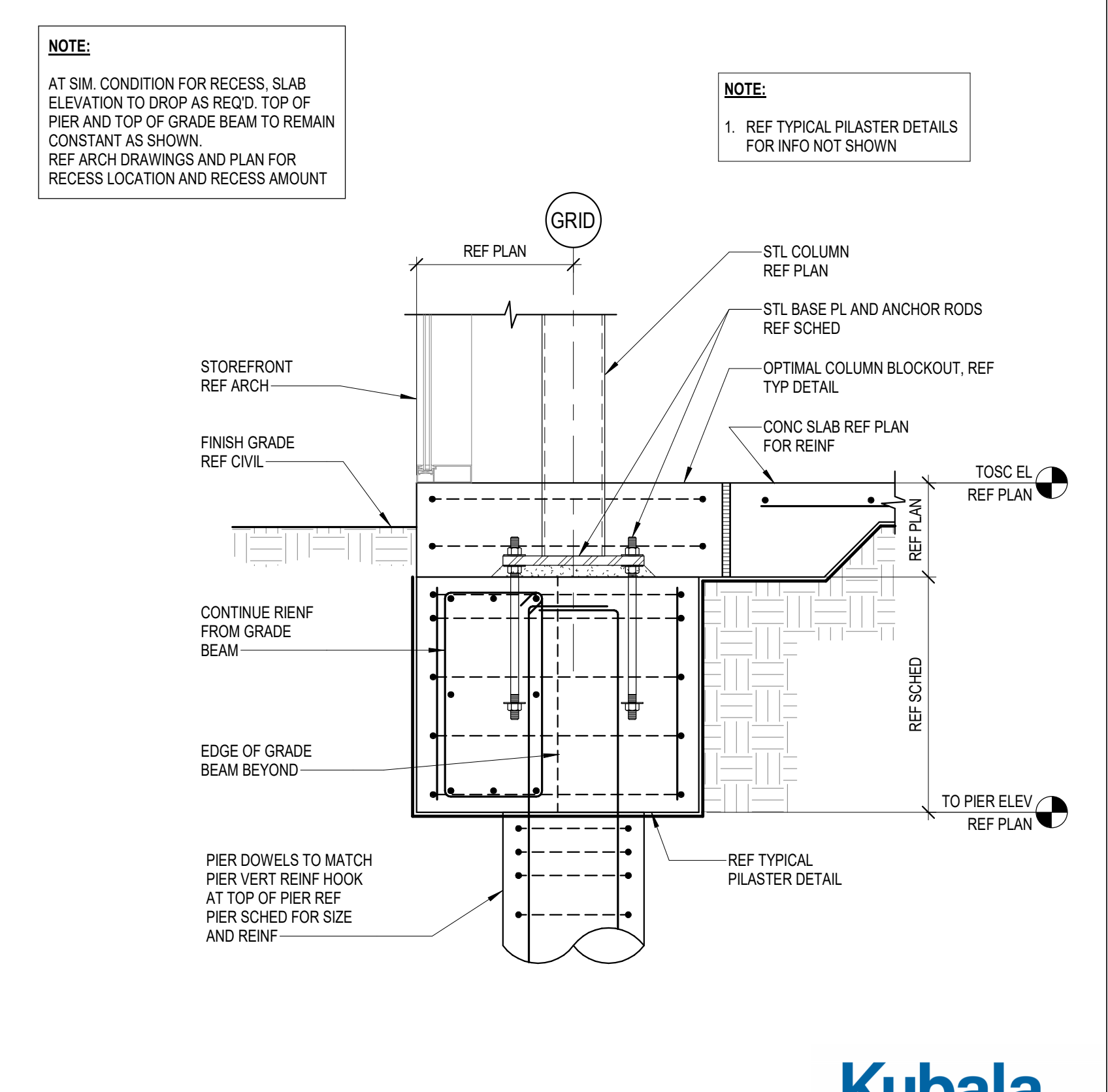
9 EXTERIOR GRADE BEAM
3/4" = 1'-0"



10 EXTERIOR GRADE BEAM AT GLAZING
3/4" = 1'-0"



11 EXTERIOR PILASTER AT COLUMN
3/4" = 1'-0"



12 EXTERIOR COLUMN AT PILASTER W/GLAZING
3/4" = 1'-0"

PKB Sports

ARCHITECT: PKB Architects, Inc.
HOUSTON, TX 77046
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0688 P
713-961-4571 F
TX Firm BR 1608

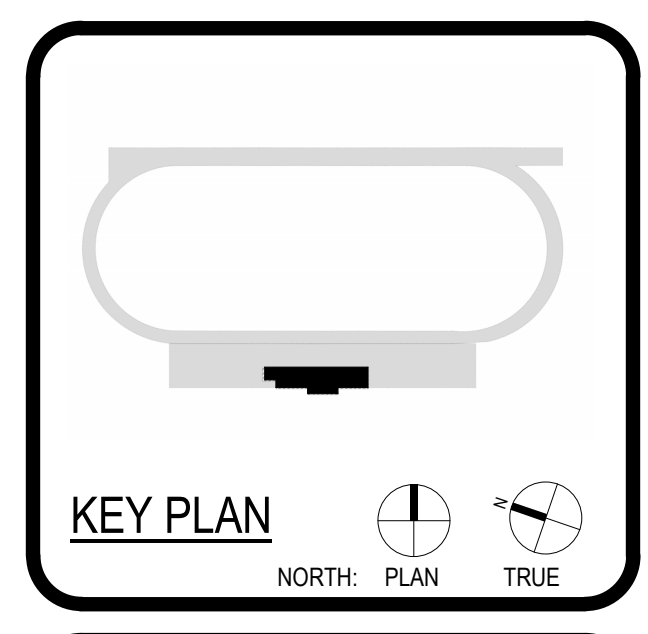
PROJECT NUMBER: 230063
DATE: 01/31/2024

1429 27TH ST.
GALVESTON, TX, 77550

GALVESTON INDEPENDENT SCHOOL DISTRICT

**GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX**

ISSUE FOR PROPOSAL



DISCIPLINE: ARCHITECTURE
STATE OF TEXAS
JOHN R. KUBALA
106120
Kubala Engineers
04/24/2024
F-23612

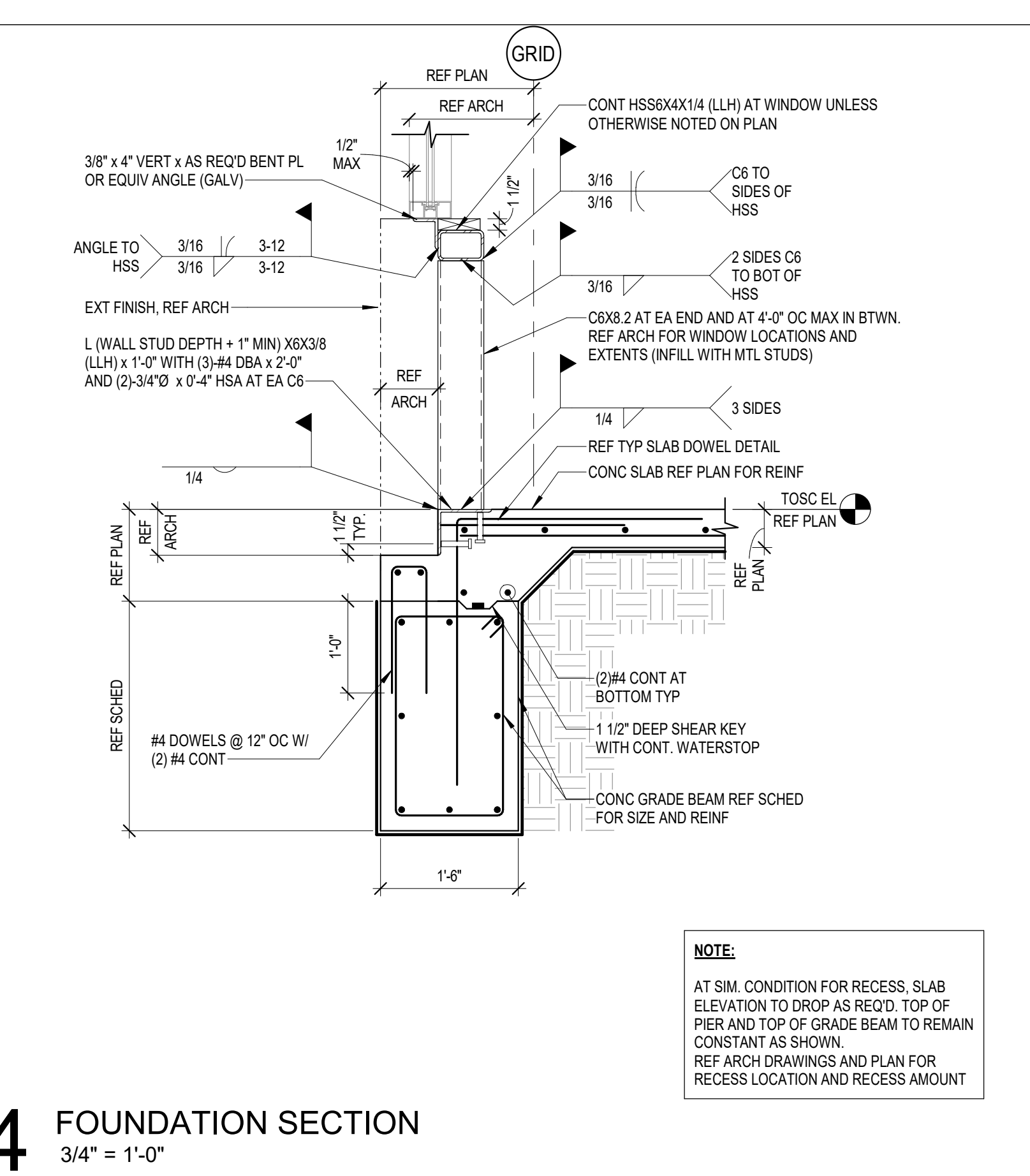
CLIENT: GALVESTON ISD

DATE: 01/31/2024 PROJECT NUMBER: 230063

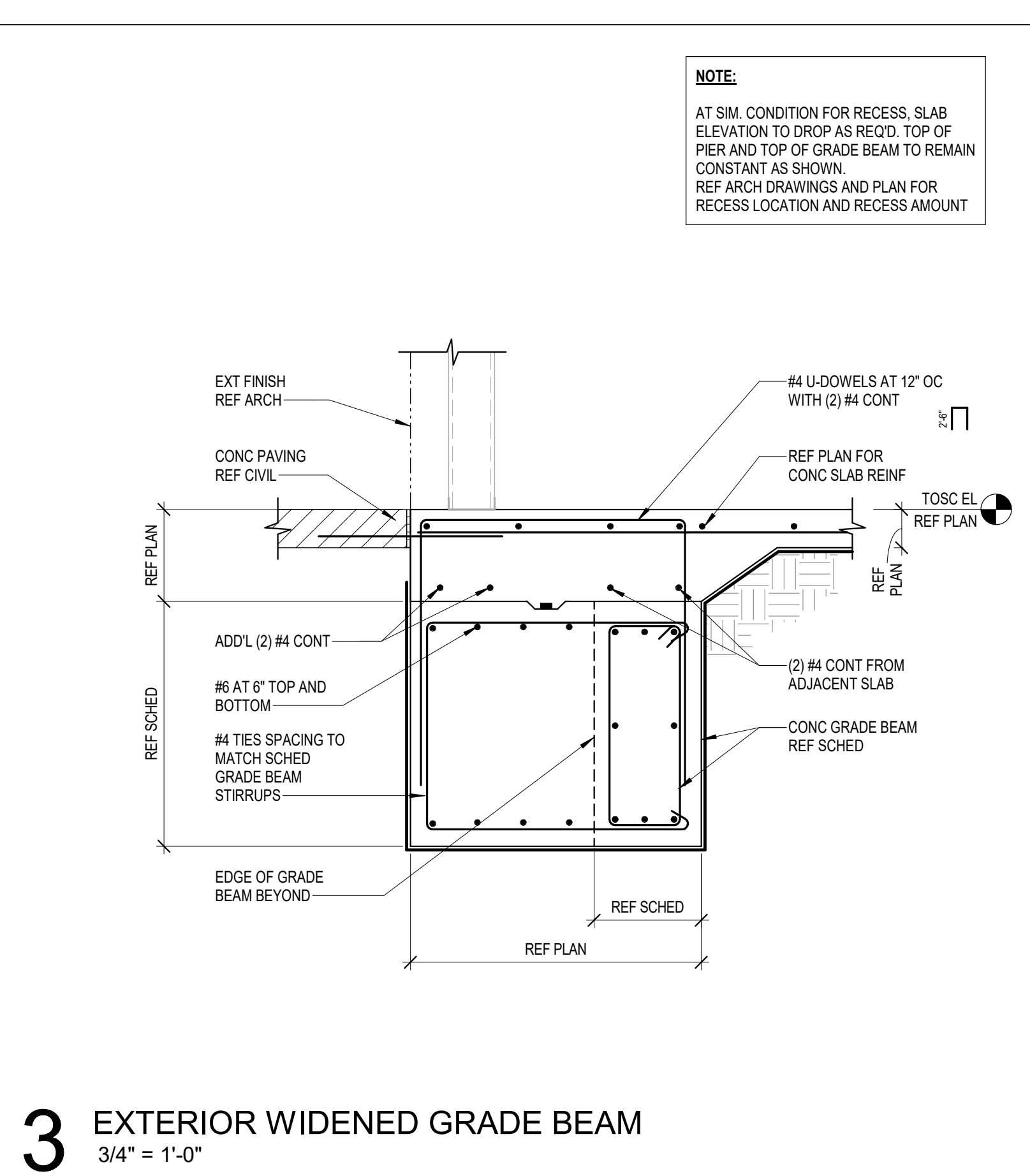
No.	Description	Date
1	AS1#1	04/24/2024

ISSUE FOR PROPOSAL
BUILDING NUMBER

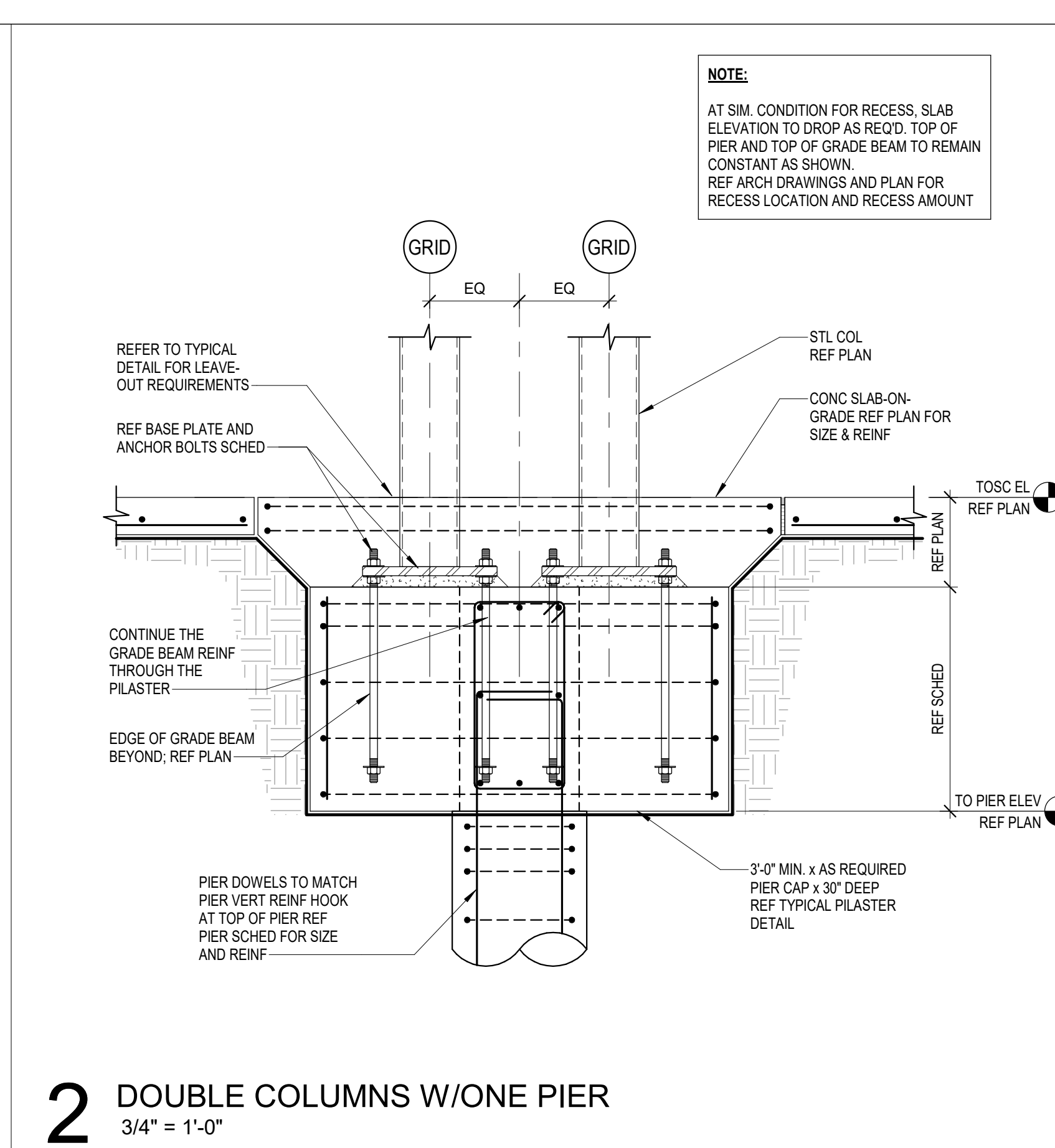
FOUNDATION DETAILS



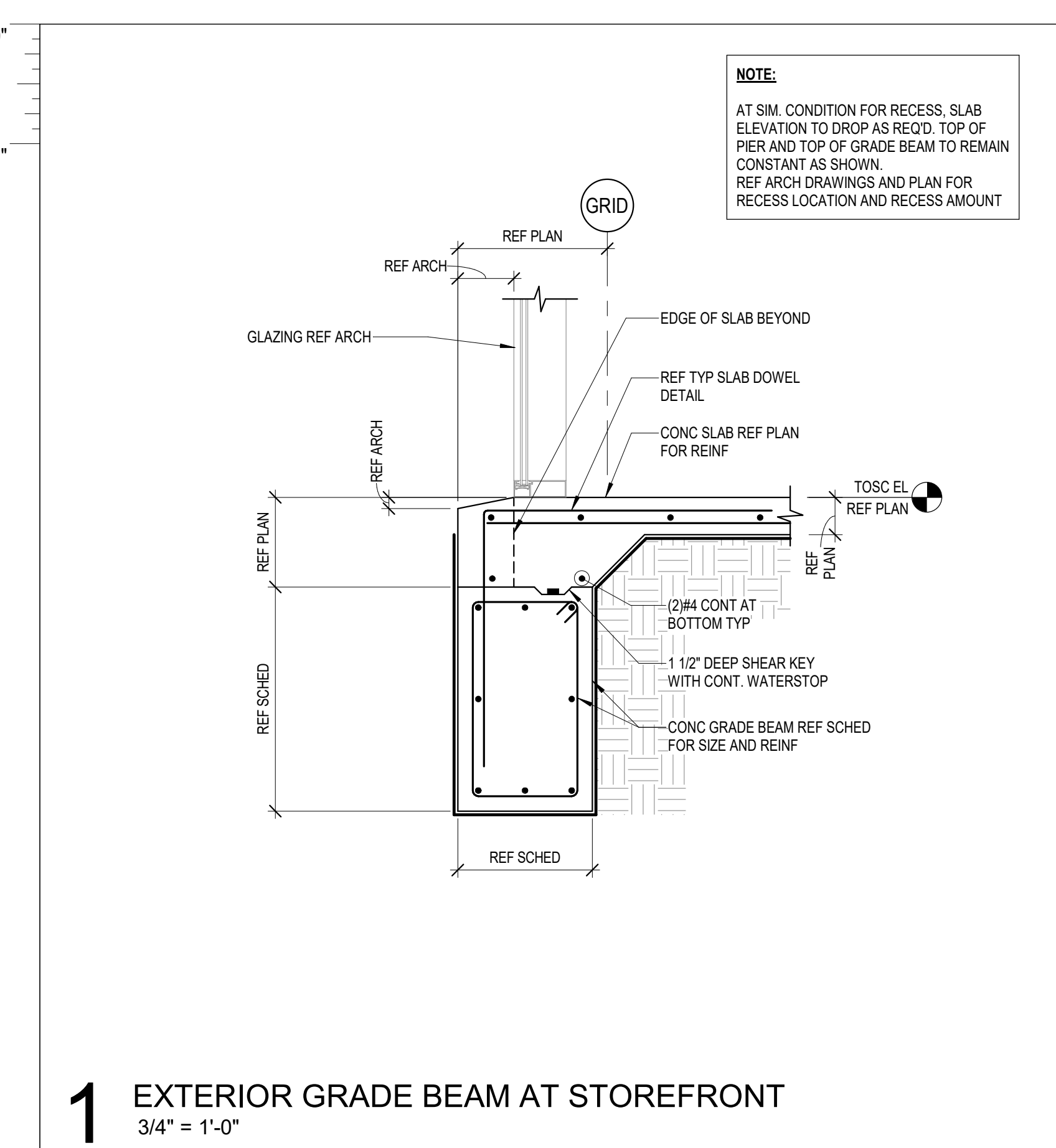
4 FOUNDATION SECTION
3/4" = 1'-0"



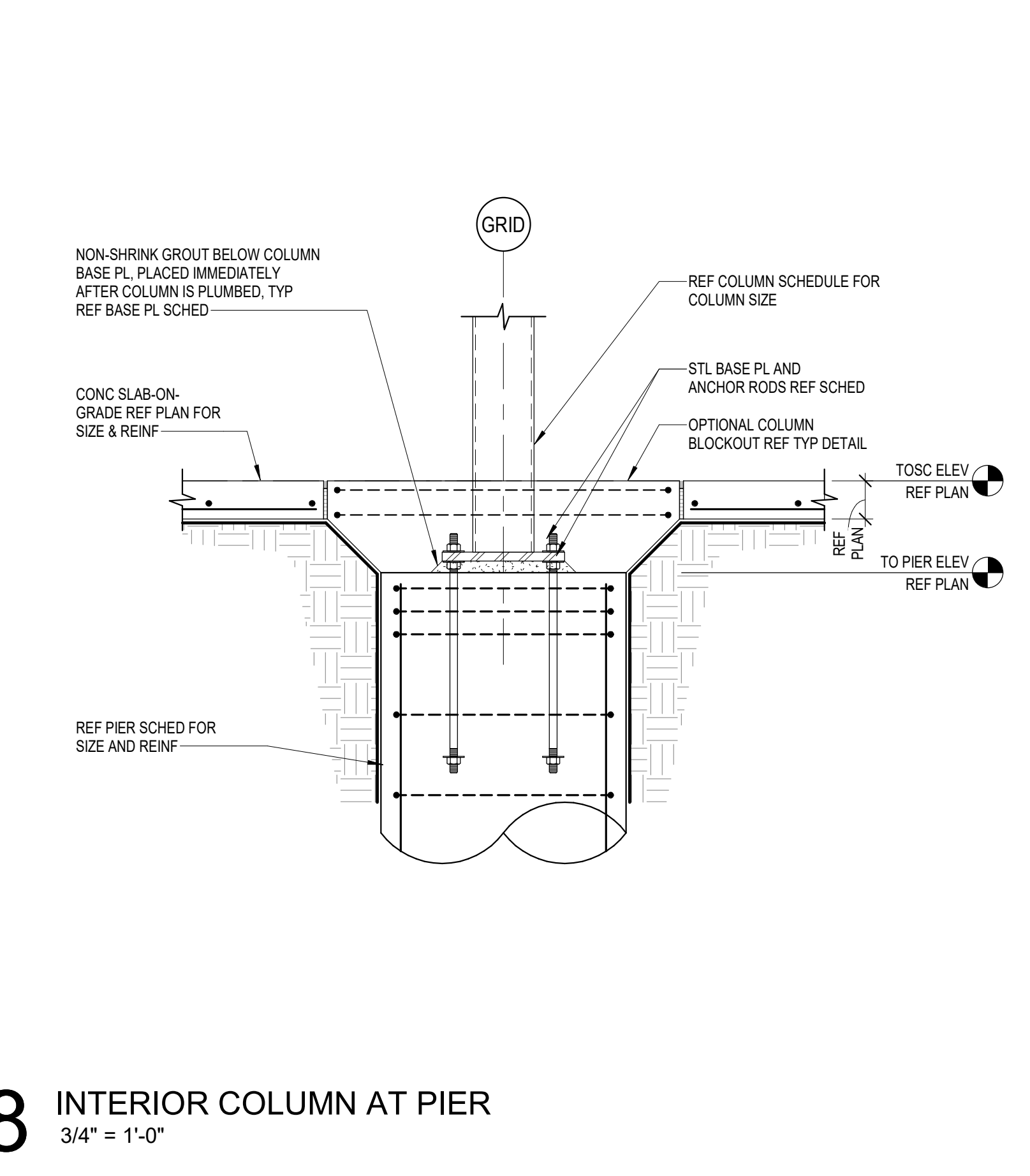
3 EXTERIOR WIDENED GRADE BEAM
3/4" = 1'-0"



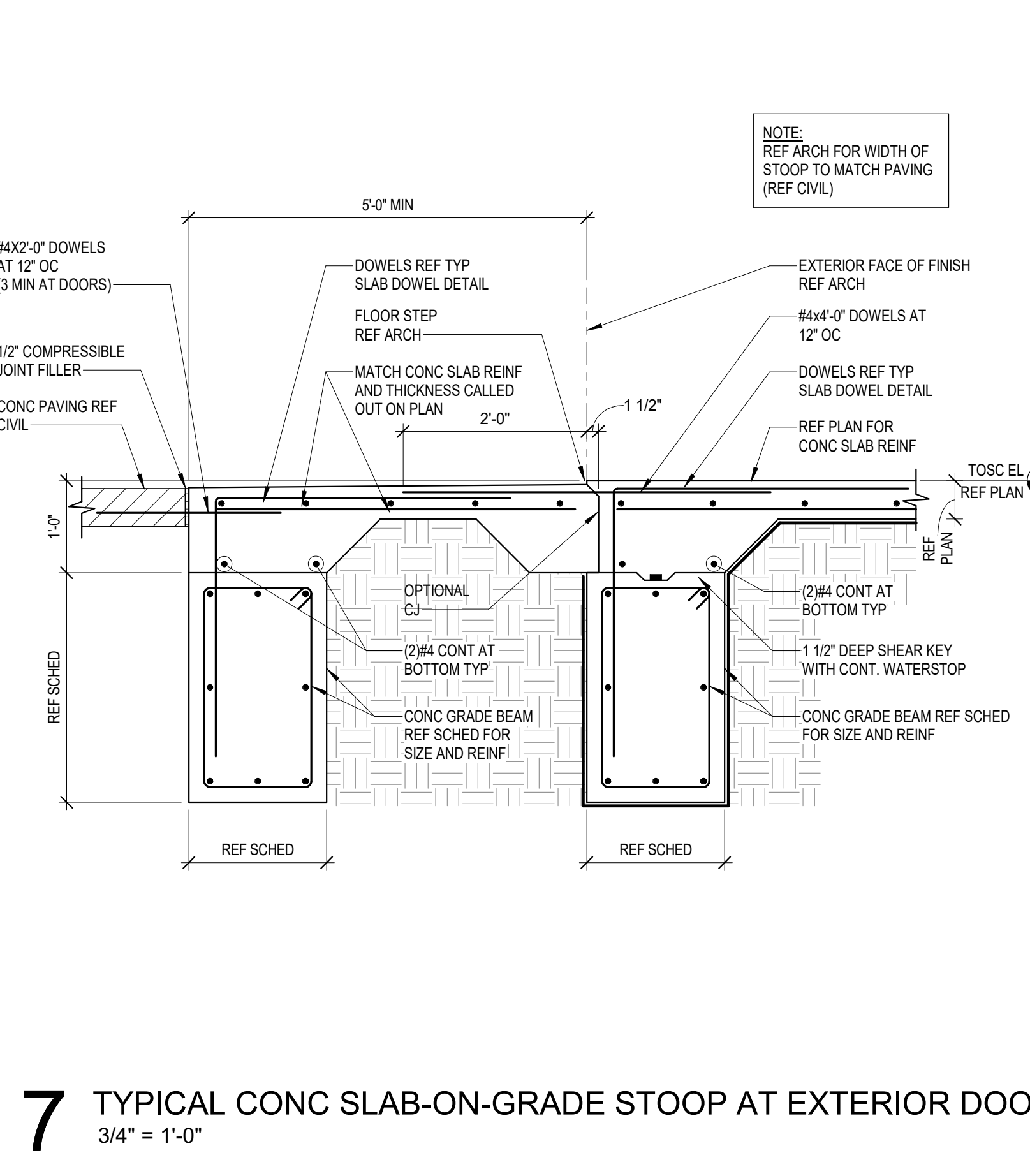
2 DOUBLE COLUMNS W/ONE PIER
3/4" = 1'-0"



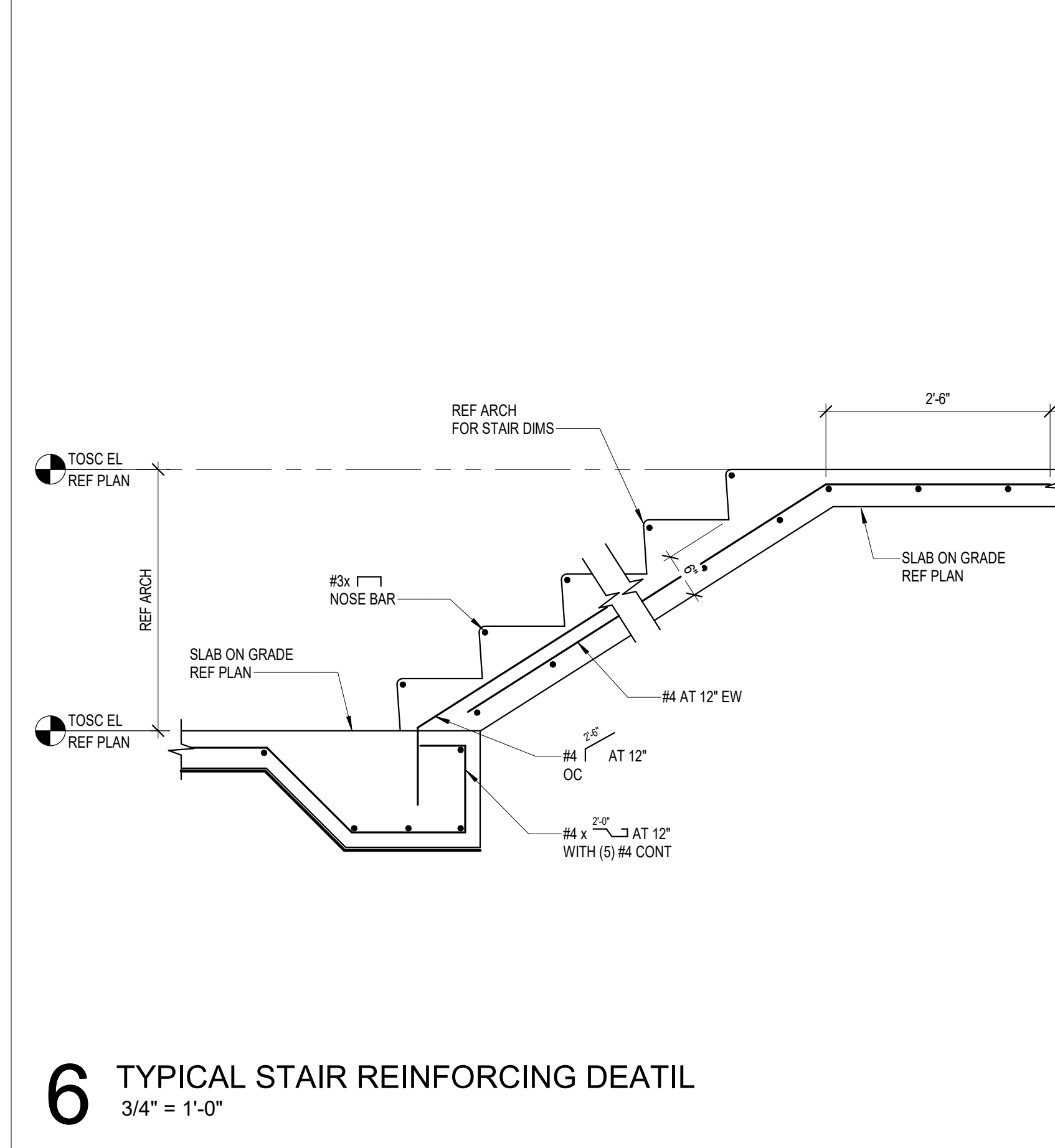
1 EXTERIOR GRADE BEAM AT STOREFRONT
3/4" = 1'-0"



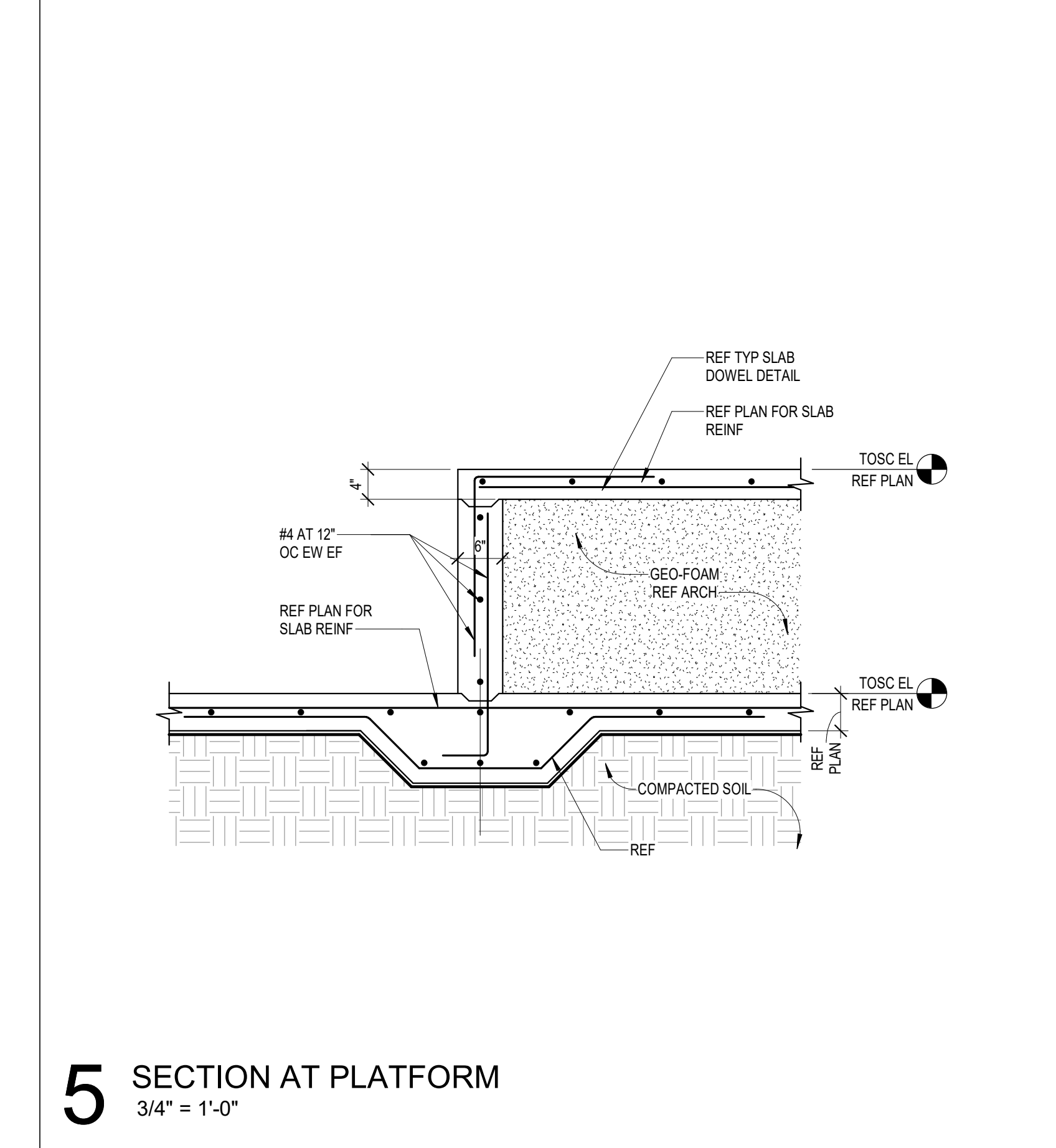
8 INTERIOR COLUMN AT PIER
3/4" = 1'-0"



7 TYPICAL CONC SLAB-ON-GRADE STOOP AT EXTERIOR DOORS
3/4" = 1'-0"



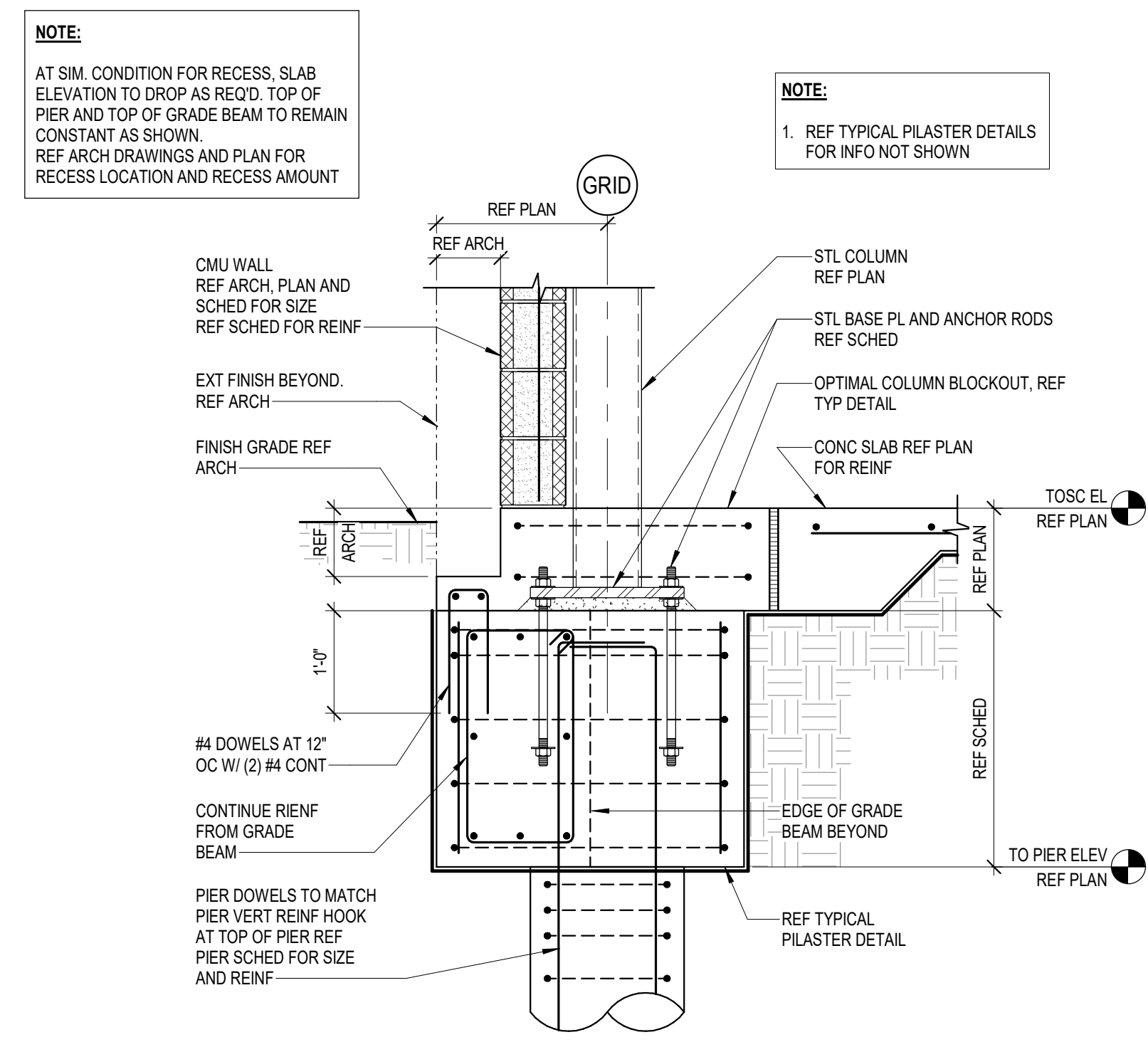
6 TYPICAL STAIR REINFORCING DETAIL
3/4" = 1'-0"



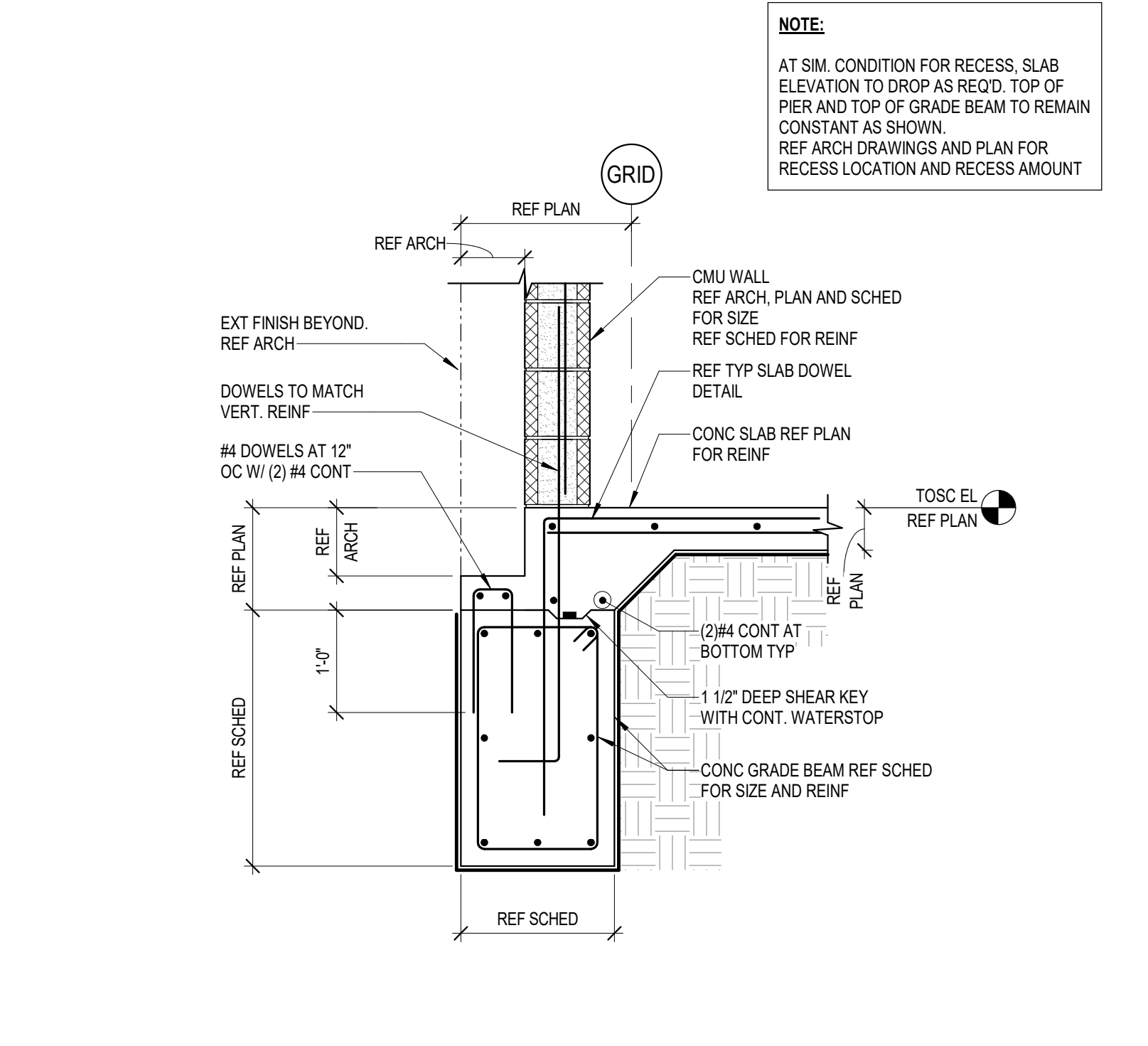
5 SECTION AT PLATFORM
3/4" = 1'-0"



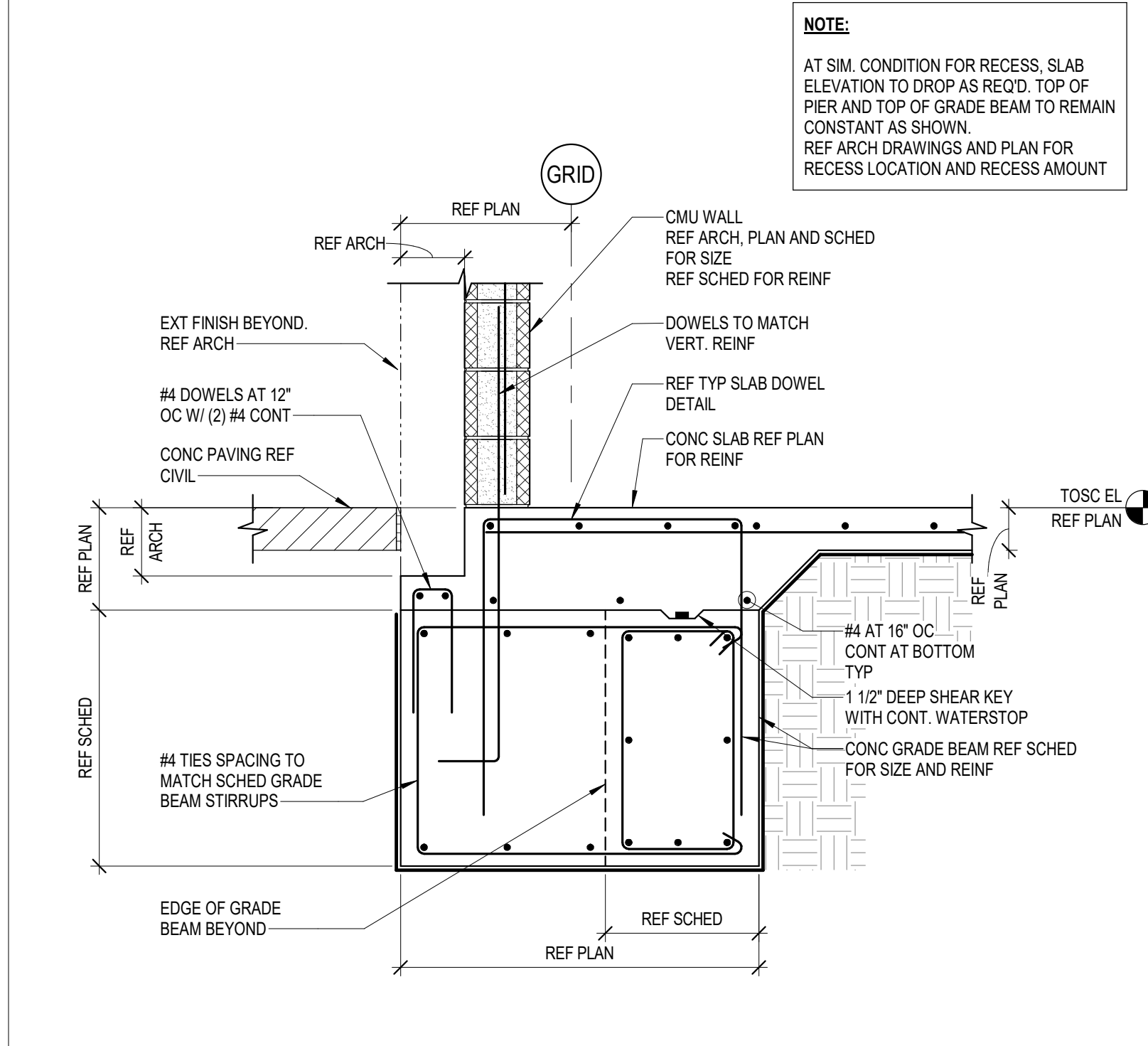
9 TYPICAL CONC SLAB-ON-GRADE STOOP AT EXTERIOR DOORS
3/4" = 1'-0"



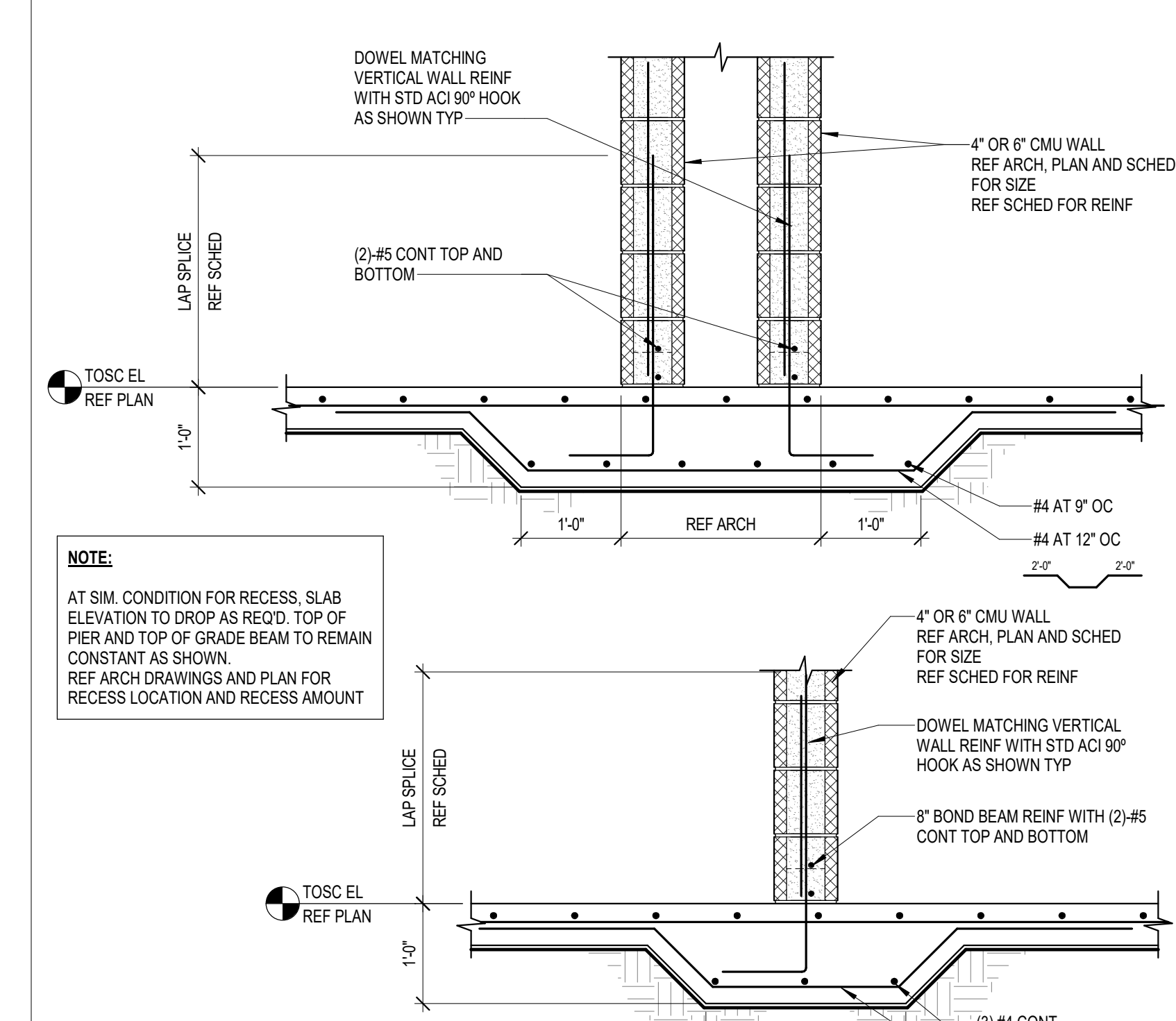
1 EXTERIOR PILASTER AT COLUMN
3/4" = 1'-0"



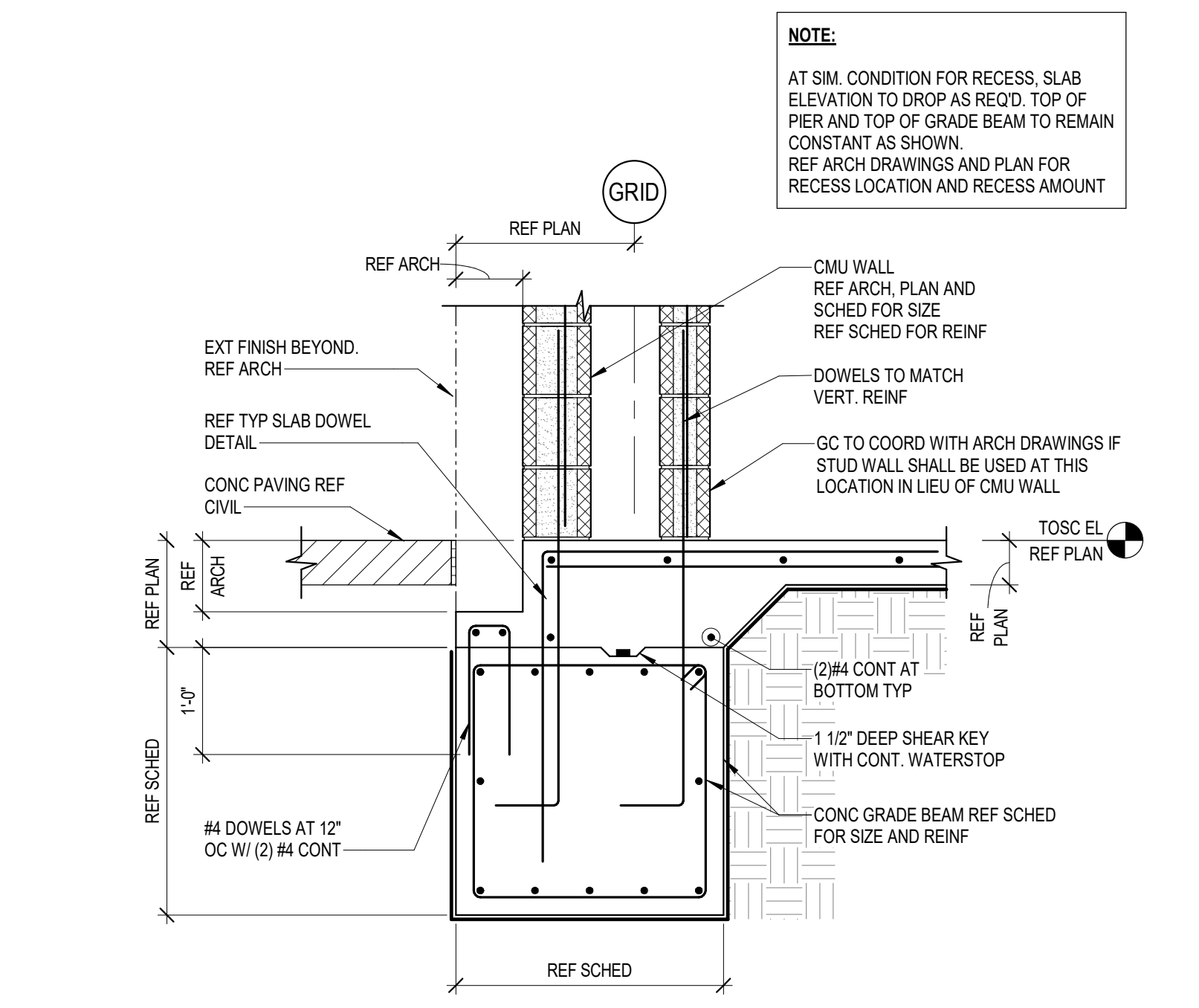
2 EXTERIOR GRADE BEAM AT CMU WALL
3/4" = 1'-0"



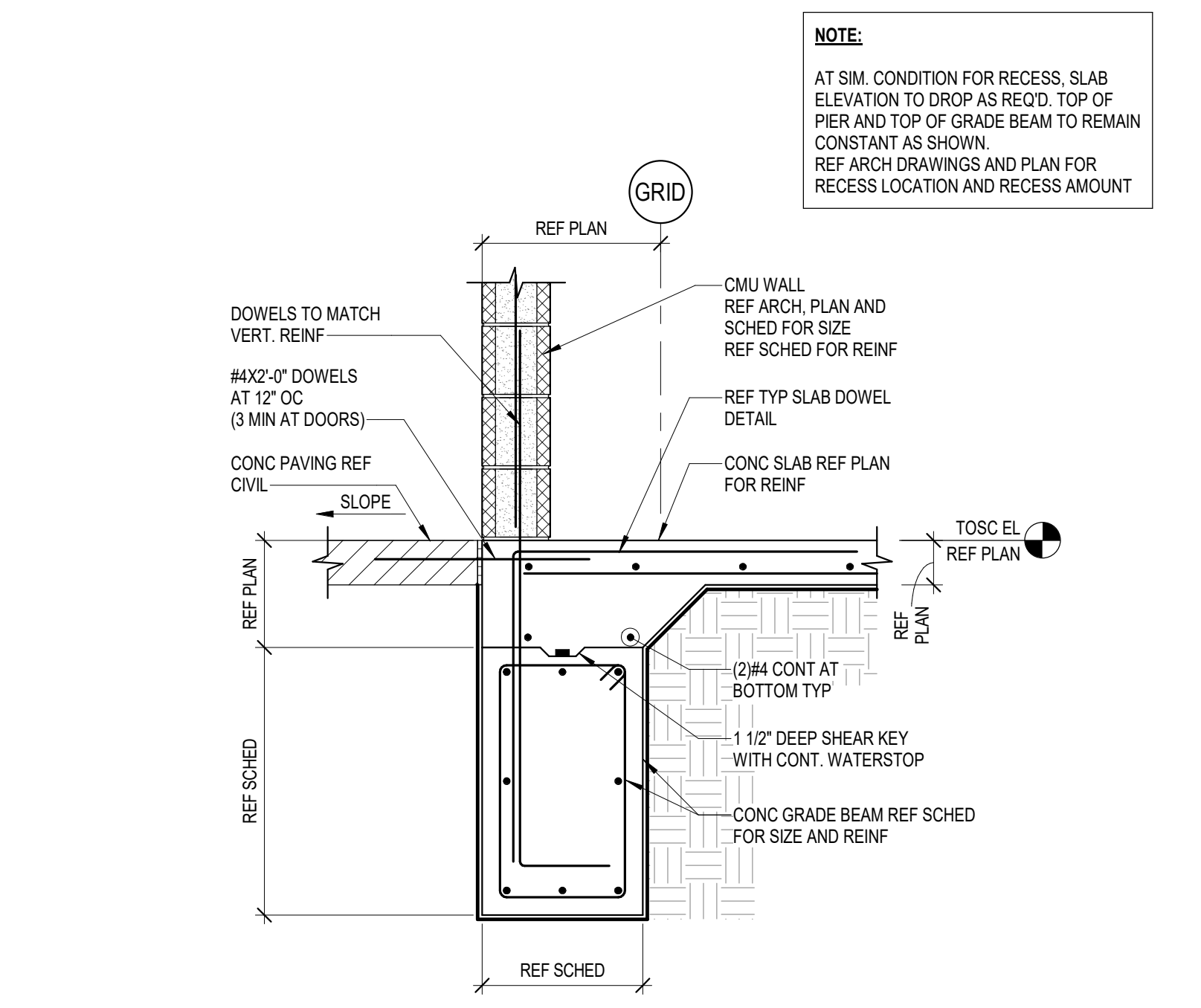
3 EXTERIOR WIDENED GRADE BEAM
3/4" = 1'-0"



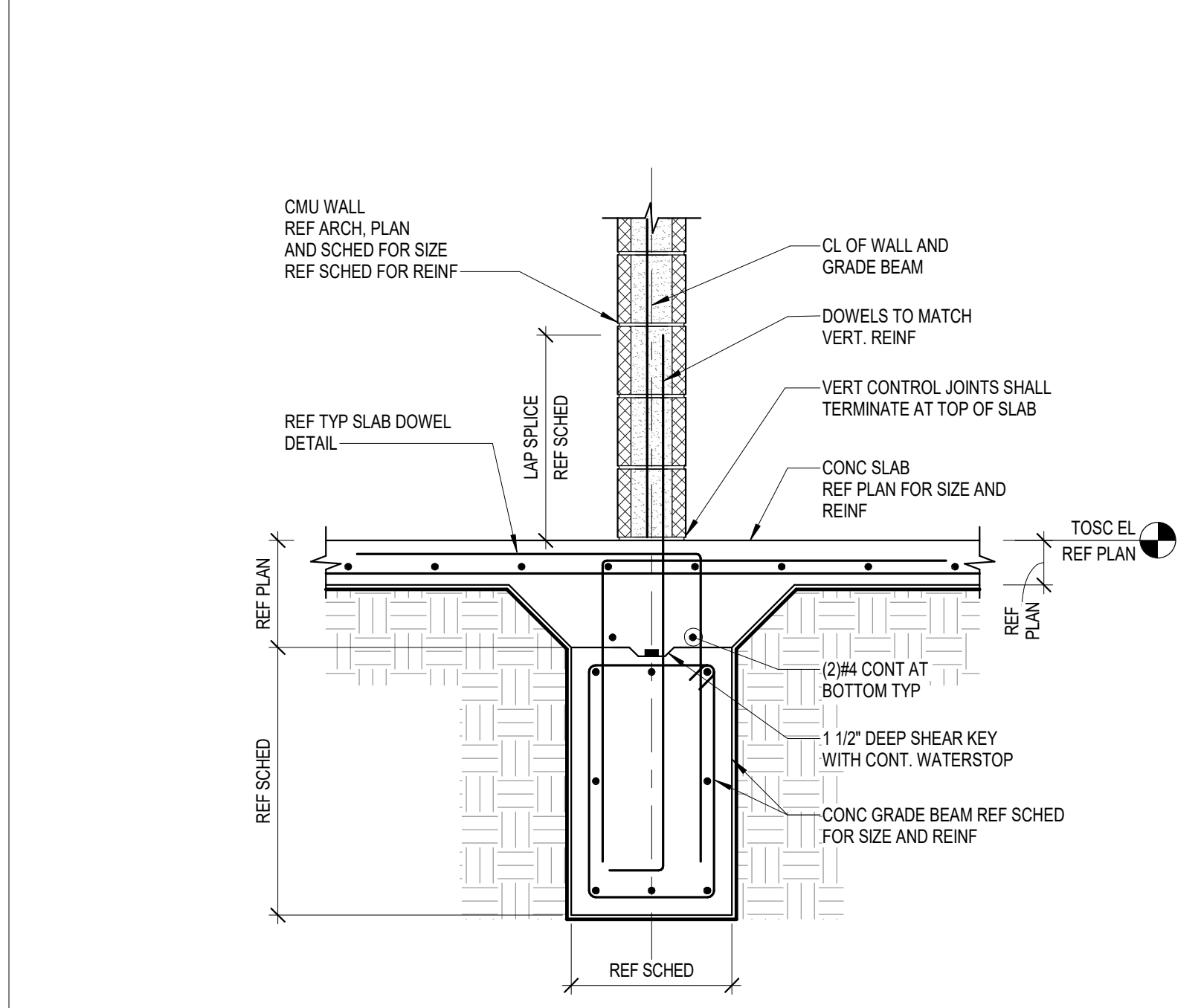
4 TYPICAL INTERIOR CMU ON SLAB-ON-GRADE
3/4" = 1'-0"



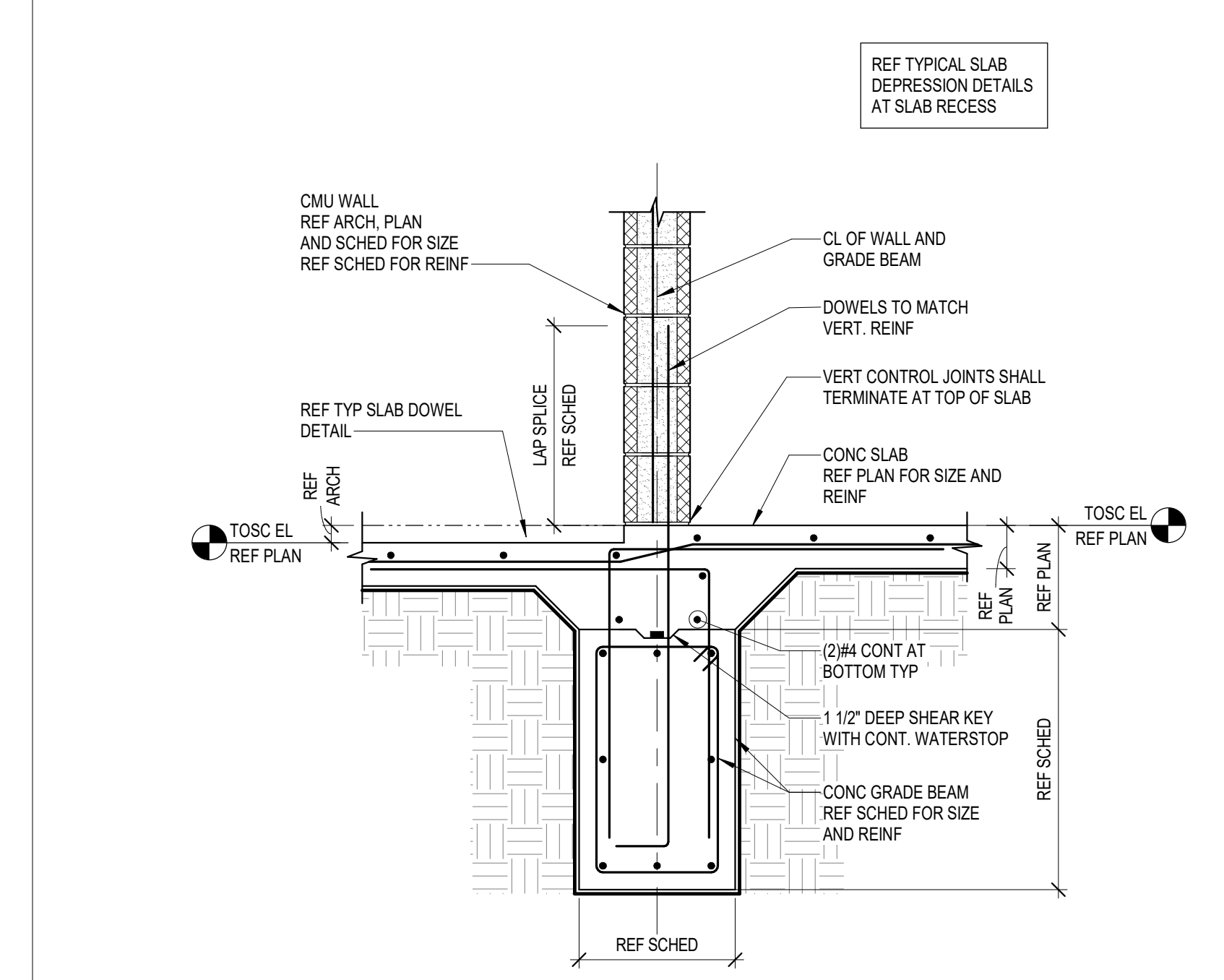
5 EXTERIOR GRADE BEAM AT DOUBLE CMU WALLS
3/4" = 1'-0"



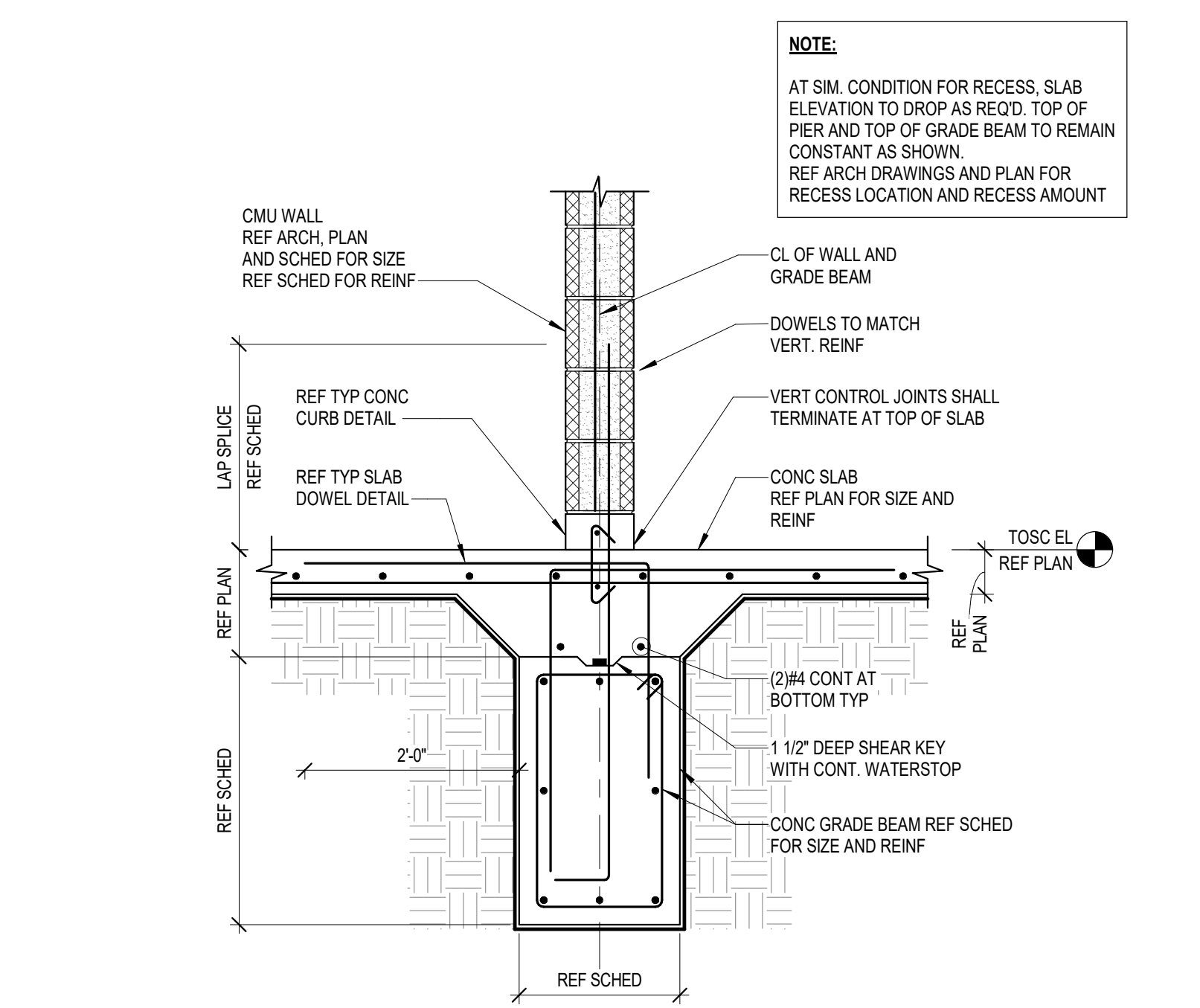
6 EXTERIOR GRADE BEAM
3/4" = 1'-0"



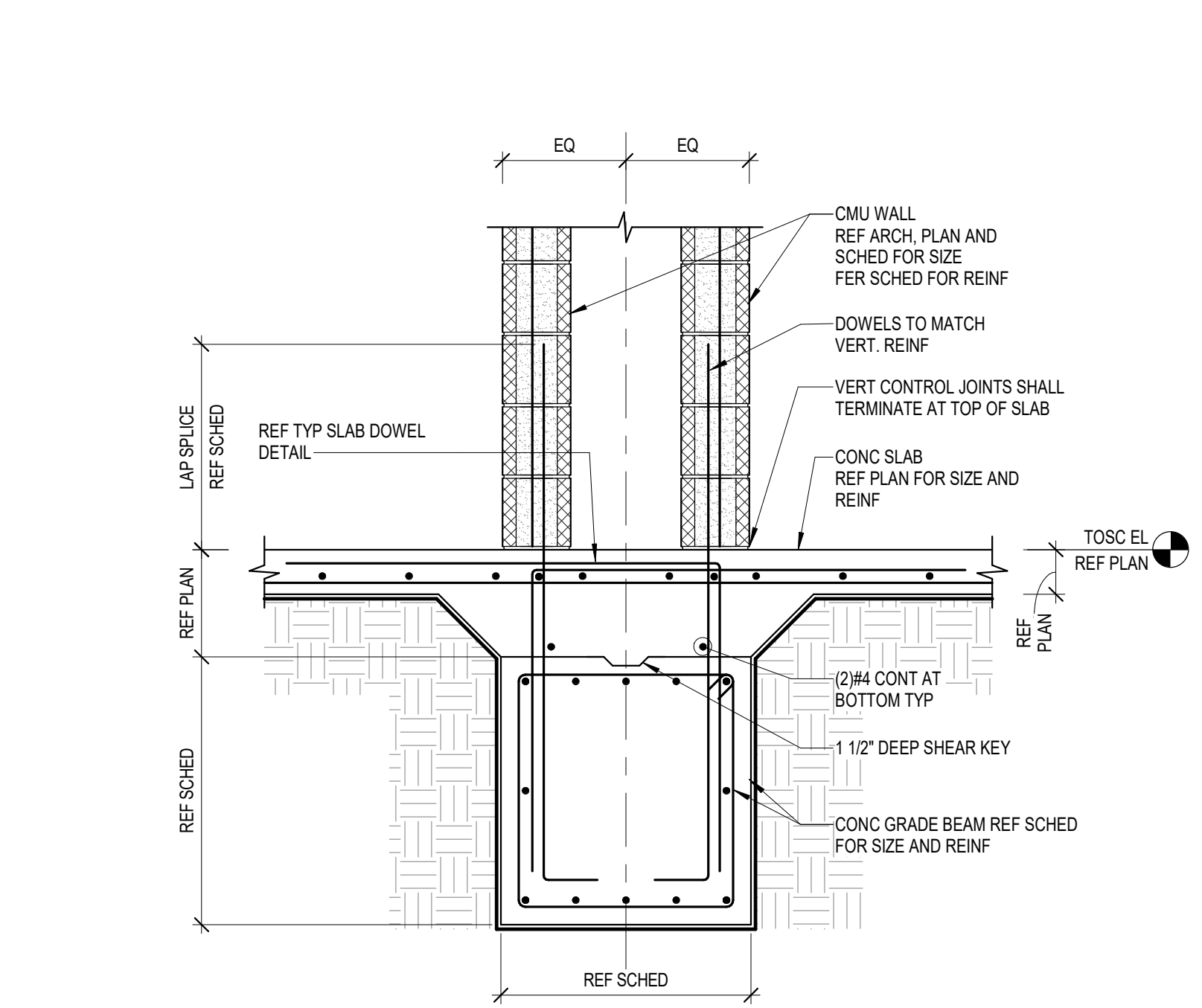
7 INTERIOR CMU WALL AT INTERIOR GRADE BEAM
3/4" = 1'-0"



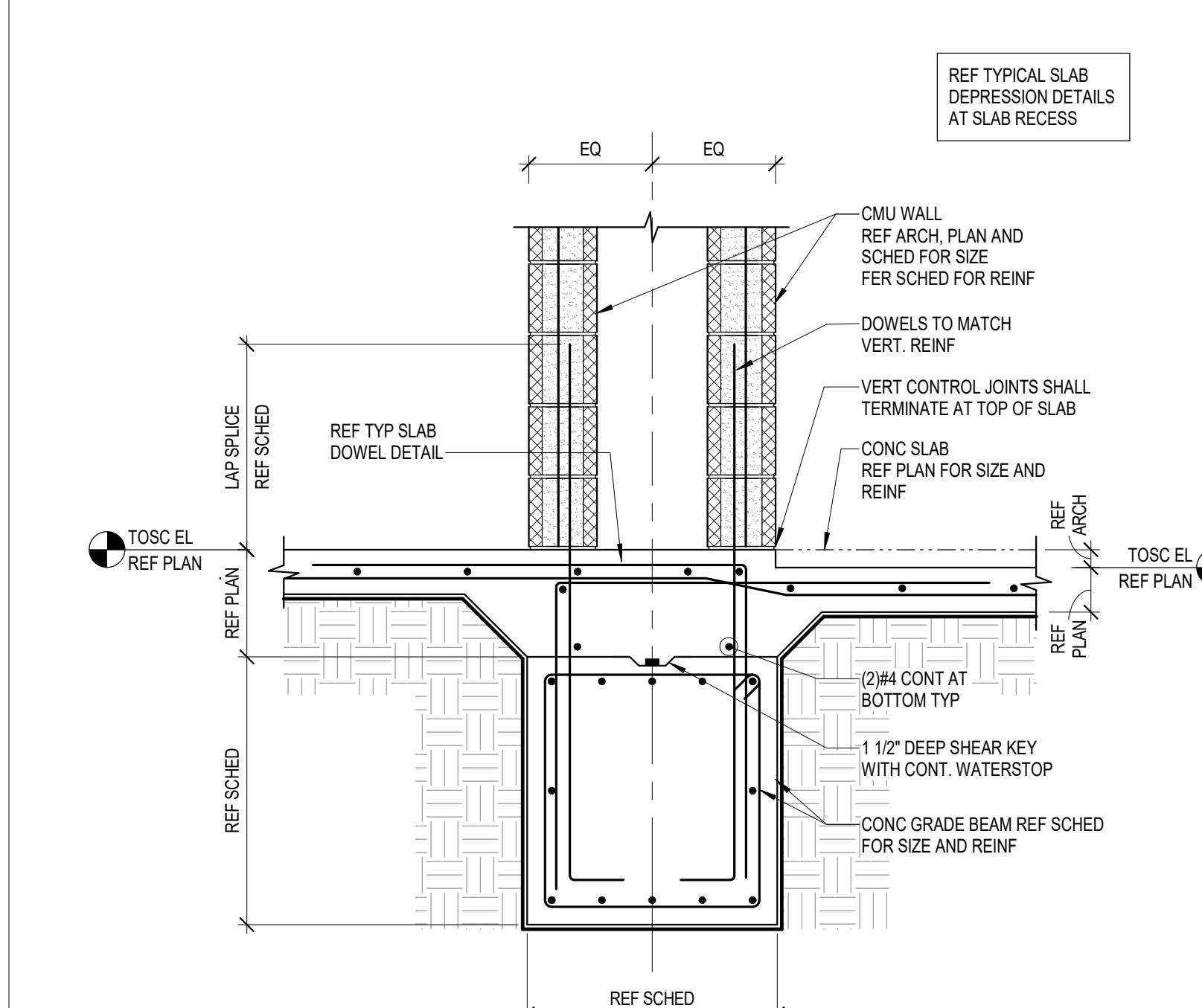
8 INTERIOR CMU WALL AT INTERIOR GRADE BEAM W/RECESS
3/4" = 1'-0"



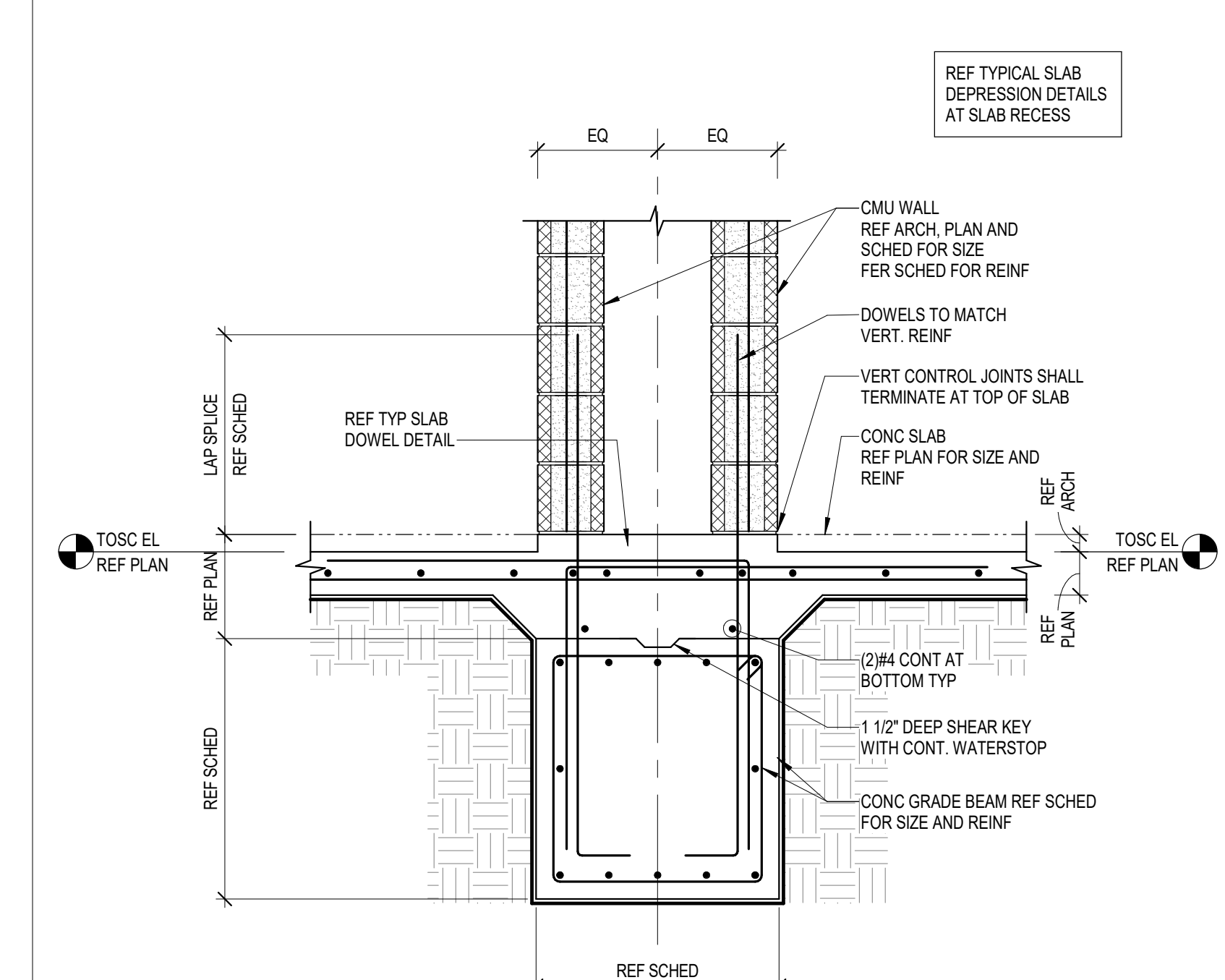
9 INTERIOR CMU WALL AT INTERIOR GRADE BEAM W/CONC CURB
3/4" = 1'-0"



10 INTERIOR GRADE BEAM AT DOUBLE CMU WALLS
3/4" = 1'-0"



11 INTERIOR GRADE BEAM AT DOUBLE CMU WALLS W/ RECESS ON ONE SIDE
3/4" = 1'-0"



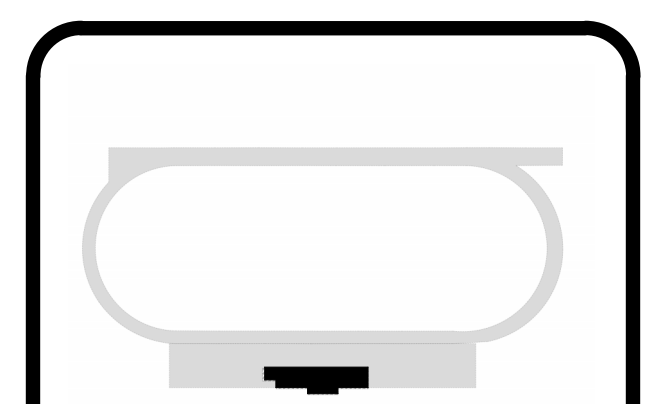
12 INTERIOR GRADE BEAM AT DOUBLE CMU WALLS W/RECESS ON BOTH SIDES
3/4" = 1'-0"



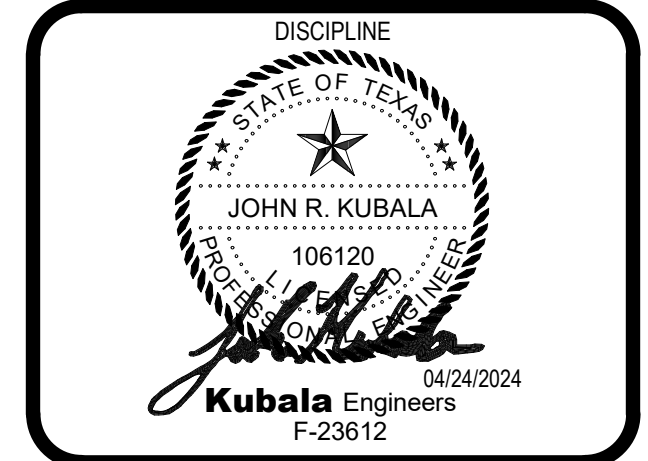
ARCHITECT
HOUSTON
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0688 P
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TX Firm BR 1608

ENGINEERS
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113.000.9933
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GALVESTON INDEPENDENT SCHOOL DISTRICT
GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX
1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL



KEY PLAN
NORTH: PLAN TRUE

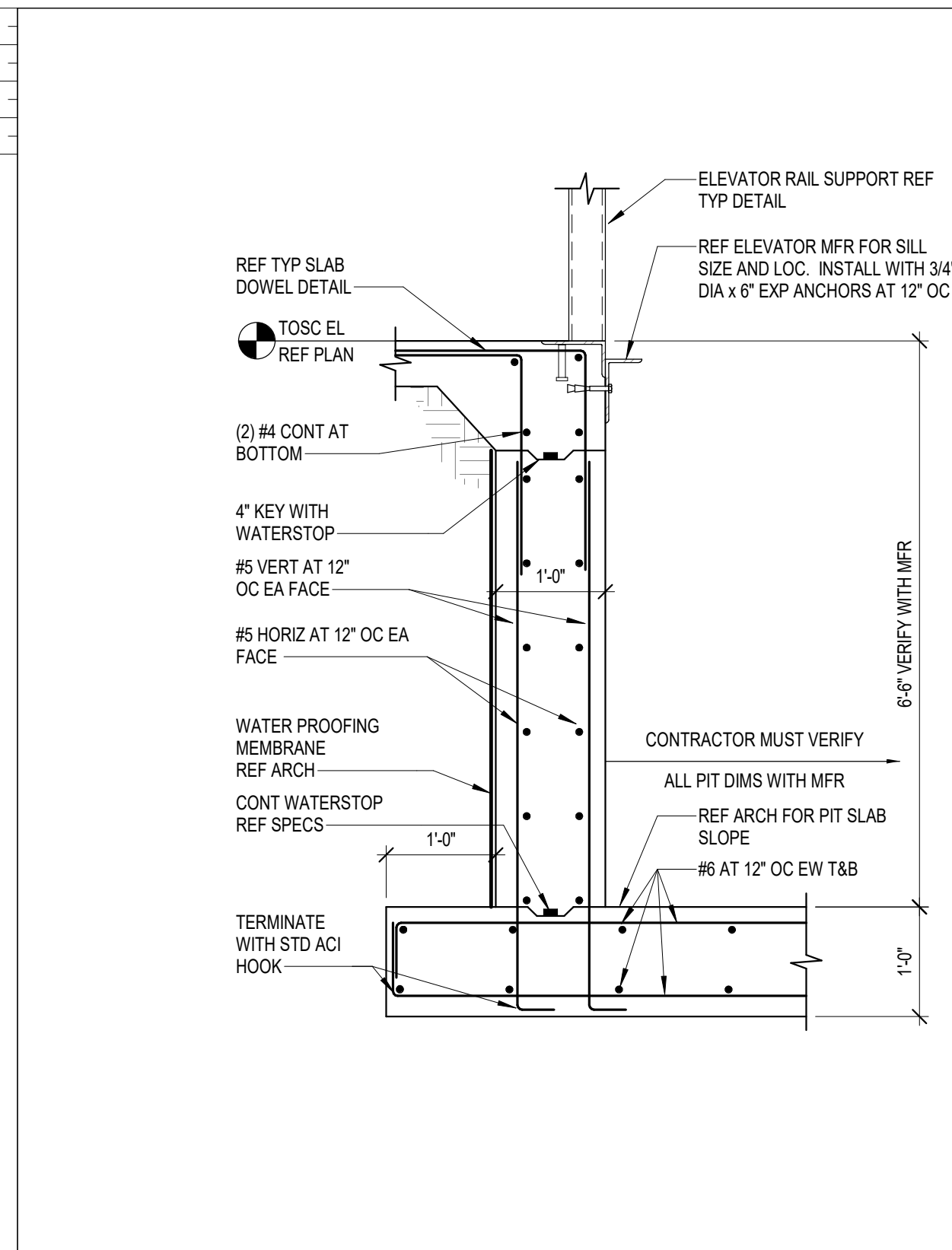


CLIENT GALVESTON ISD		
DATE 01/31/2024	PROJECT NUMBER 230063	
DRAWING HISTORY		
No.	Description	Date
1	AS#1	04/24/2024

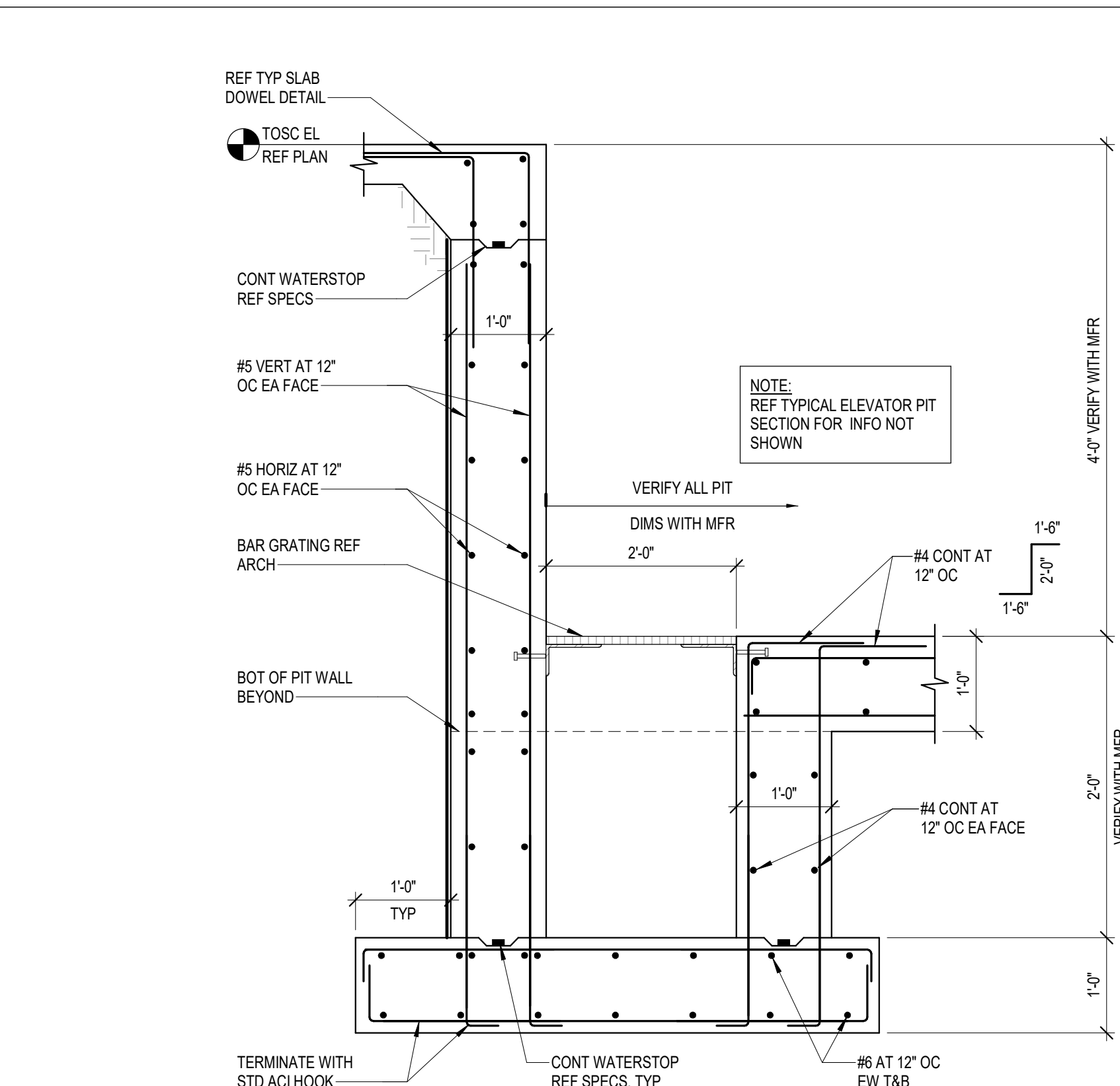
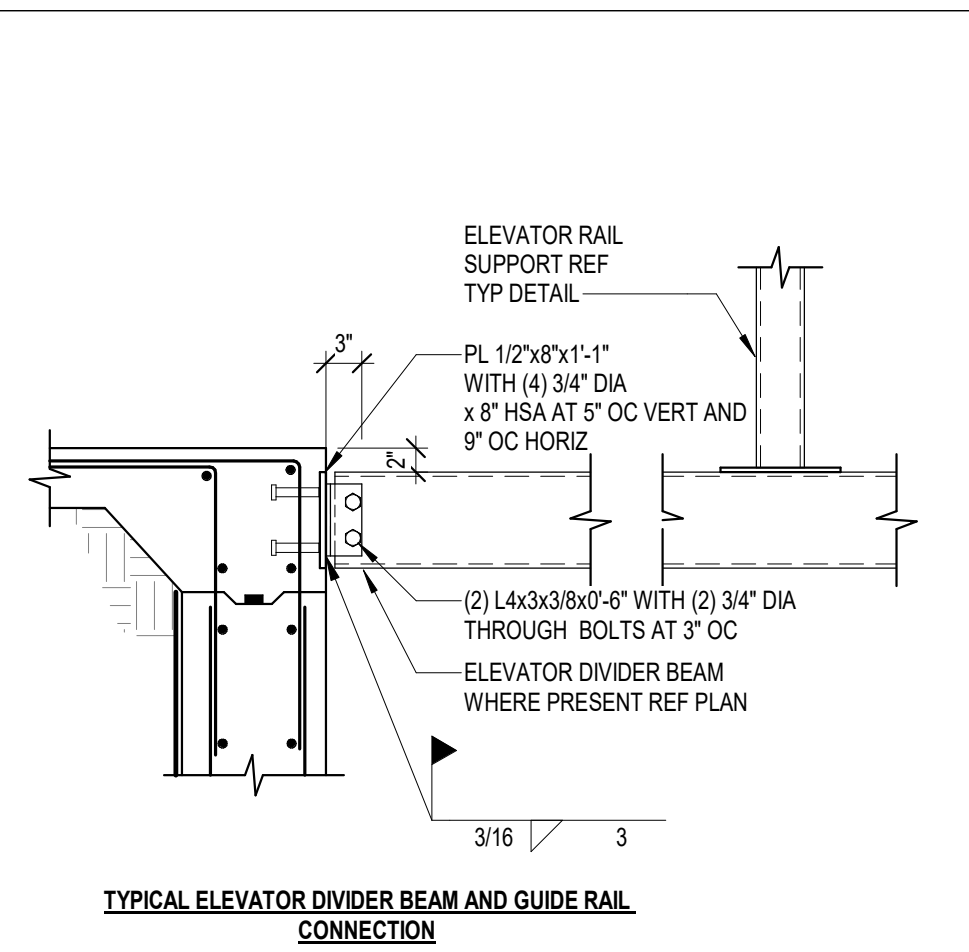
ISSUE FOR PROPOSAL
BUILDING NUMBER

FOUNDATION DETAILS (CMU)

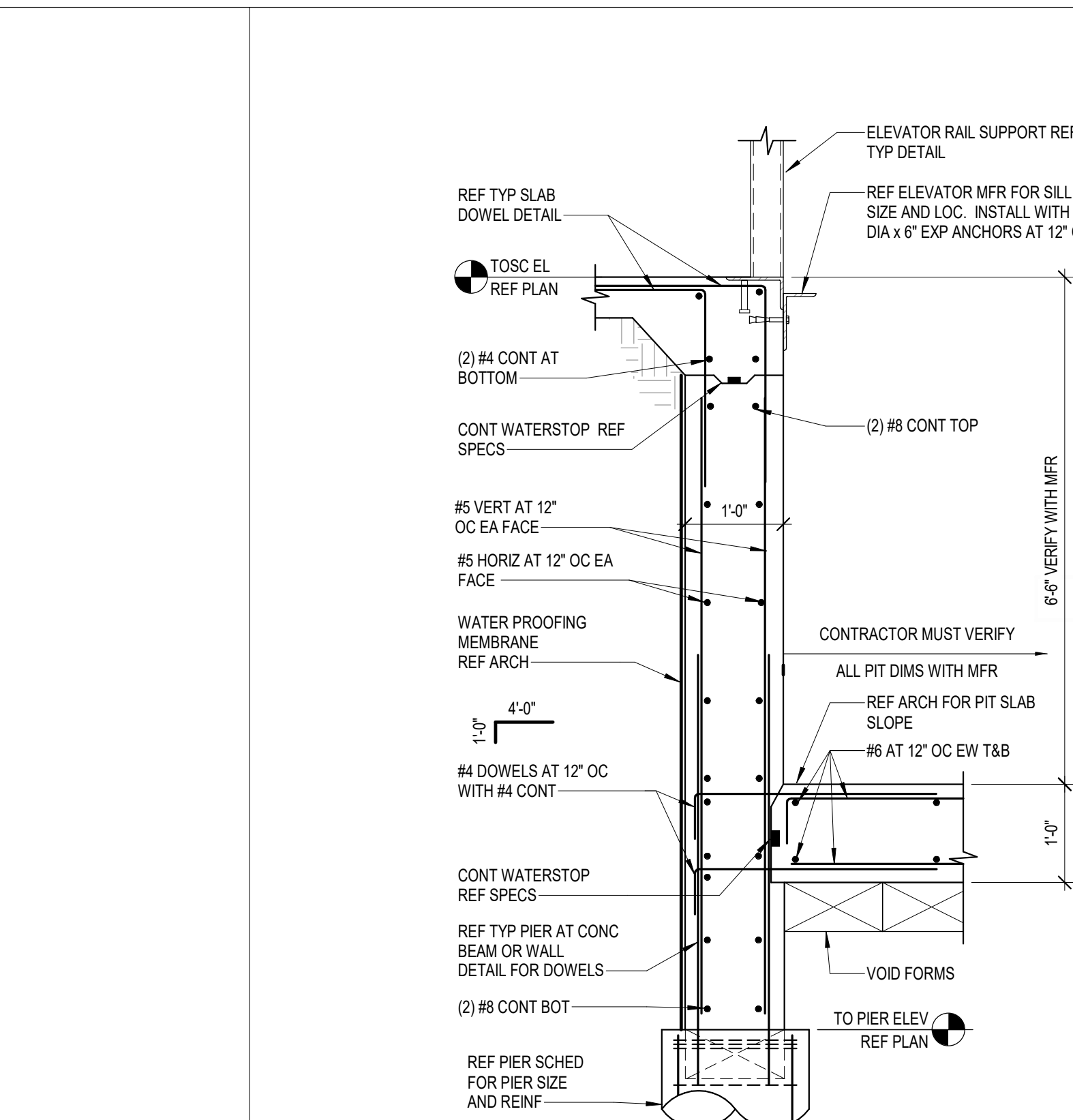
S-315
 ELEVATOR PIT DETAILS
 FOR BLUEBEAM LABELING/OCR



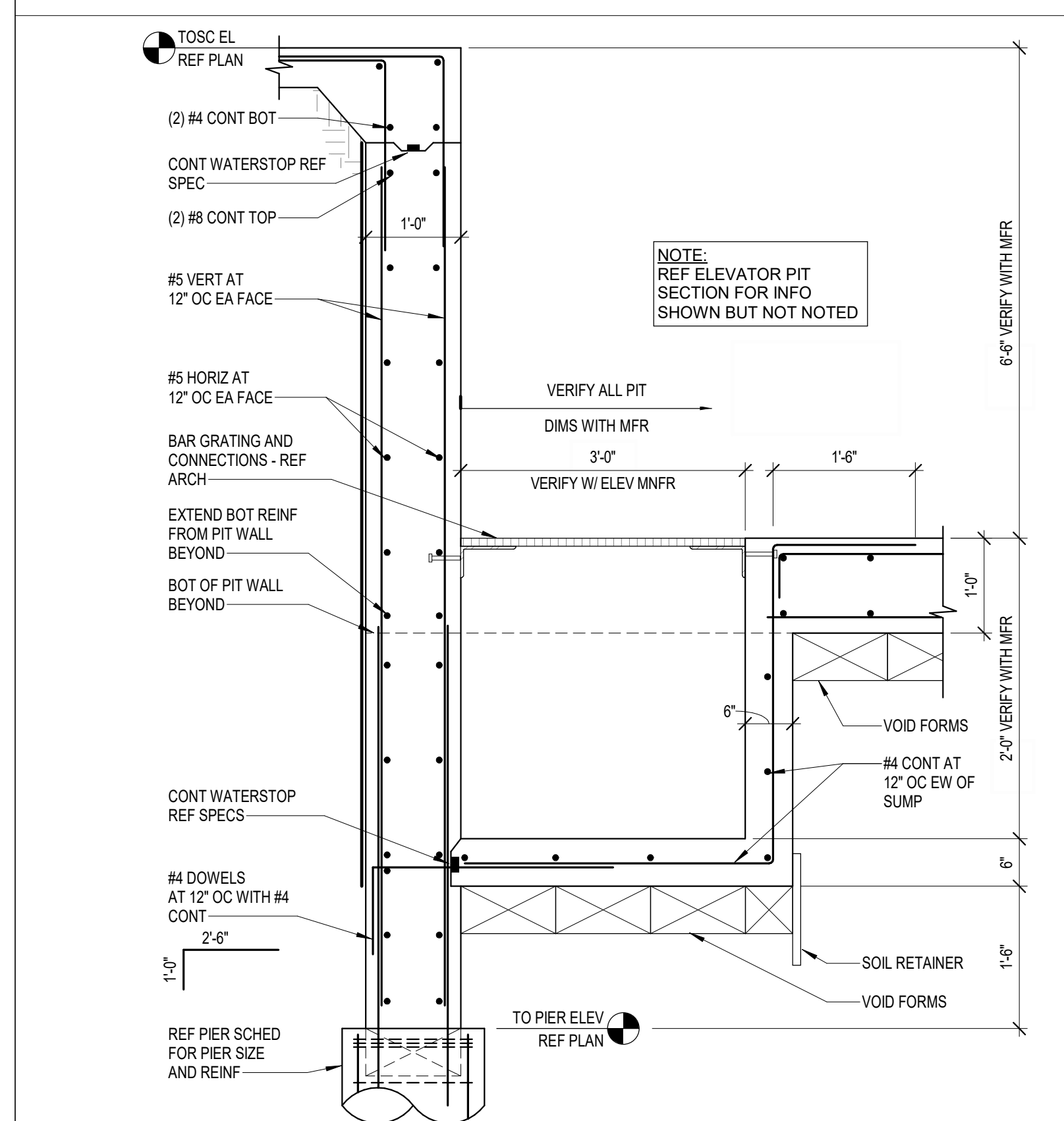
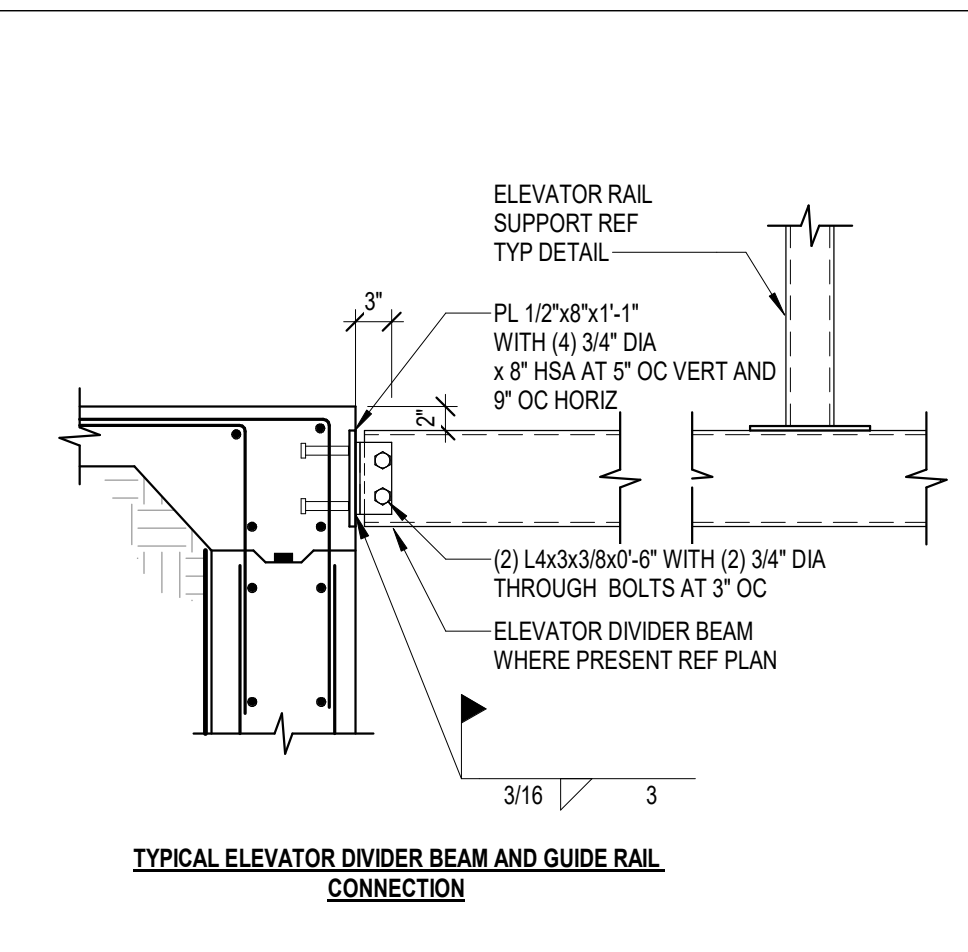
1 ELEVATOR PIT
 3/4" = 1'-0"



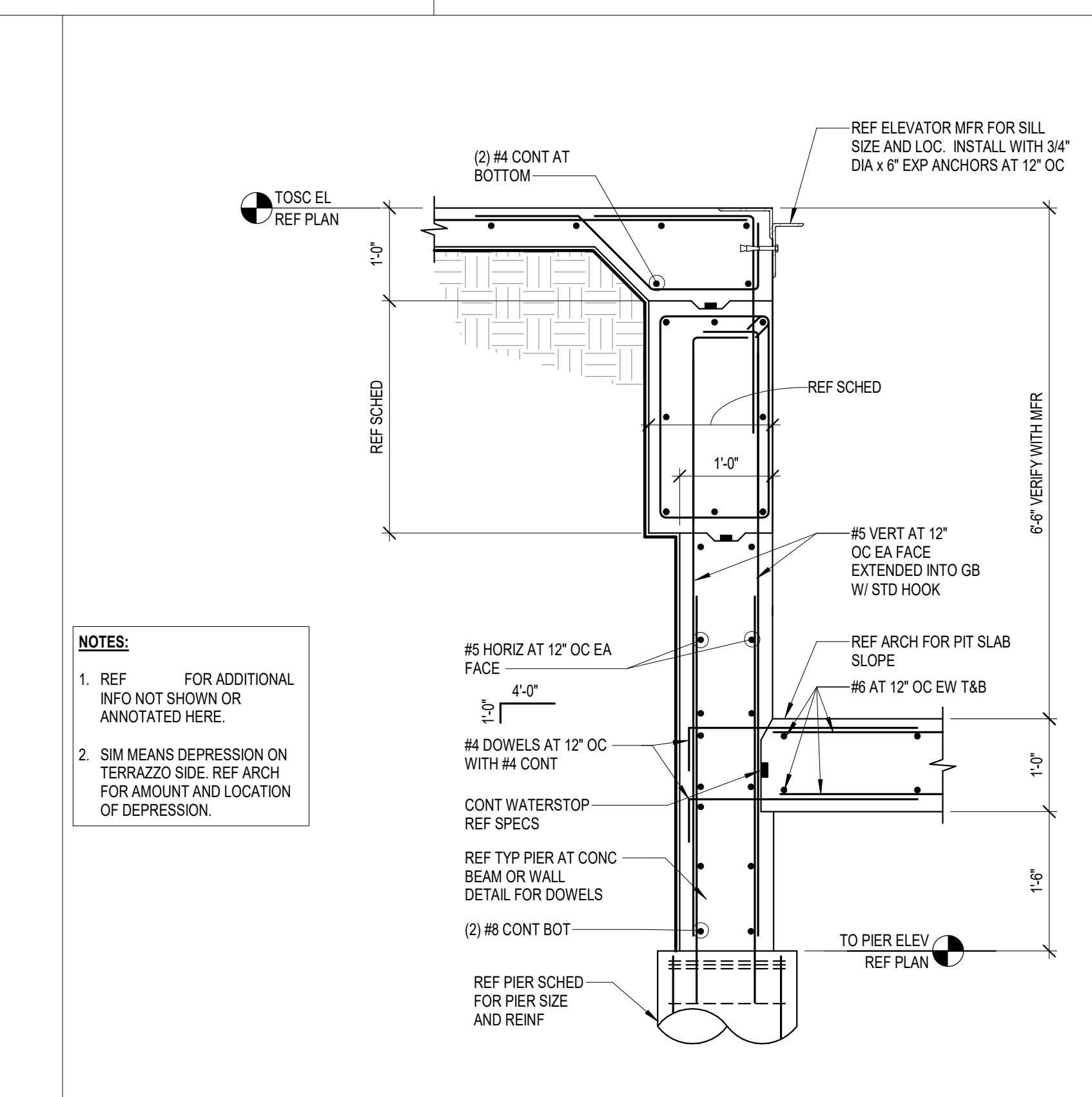
2 ELEVATOR SUMP PIT
 3/4" = 1'-0"



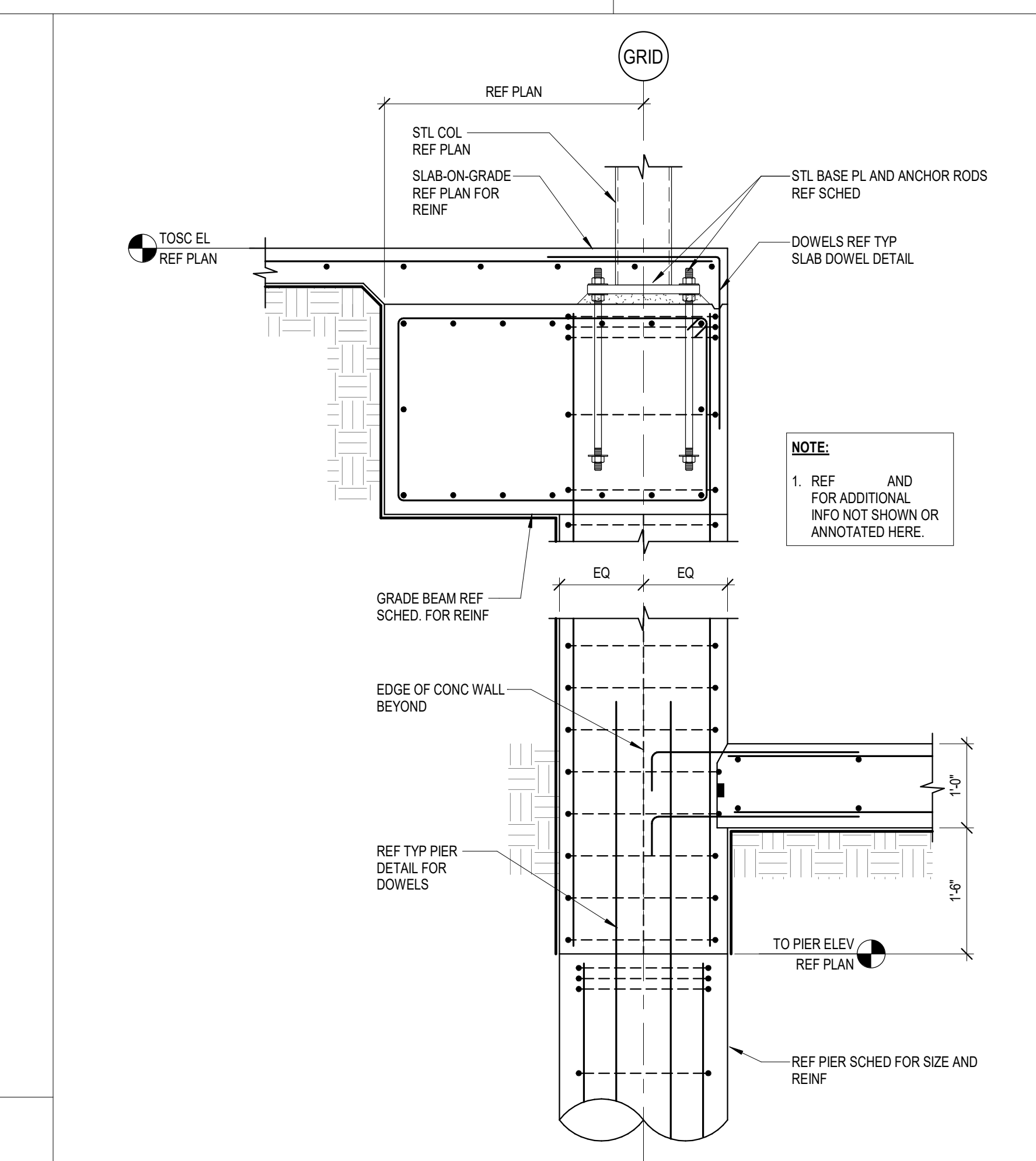
3 ELEVATOR PIT
 3/4" = 1'-0"



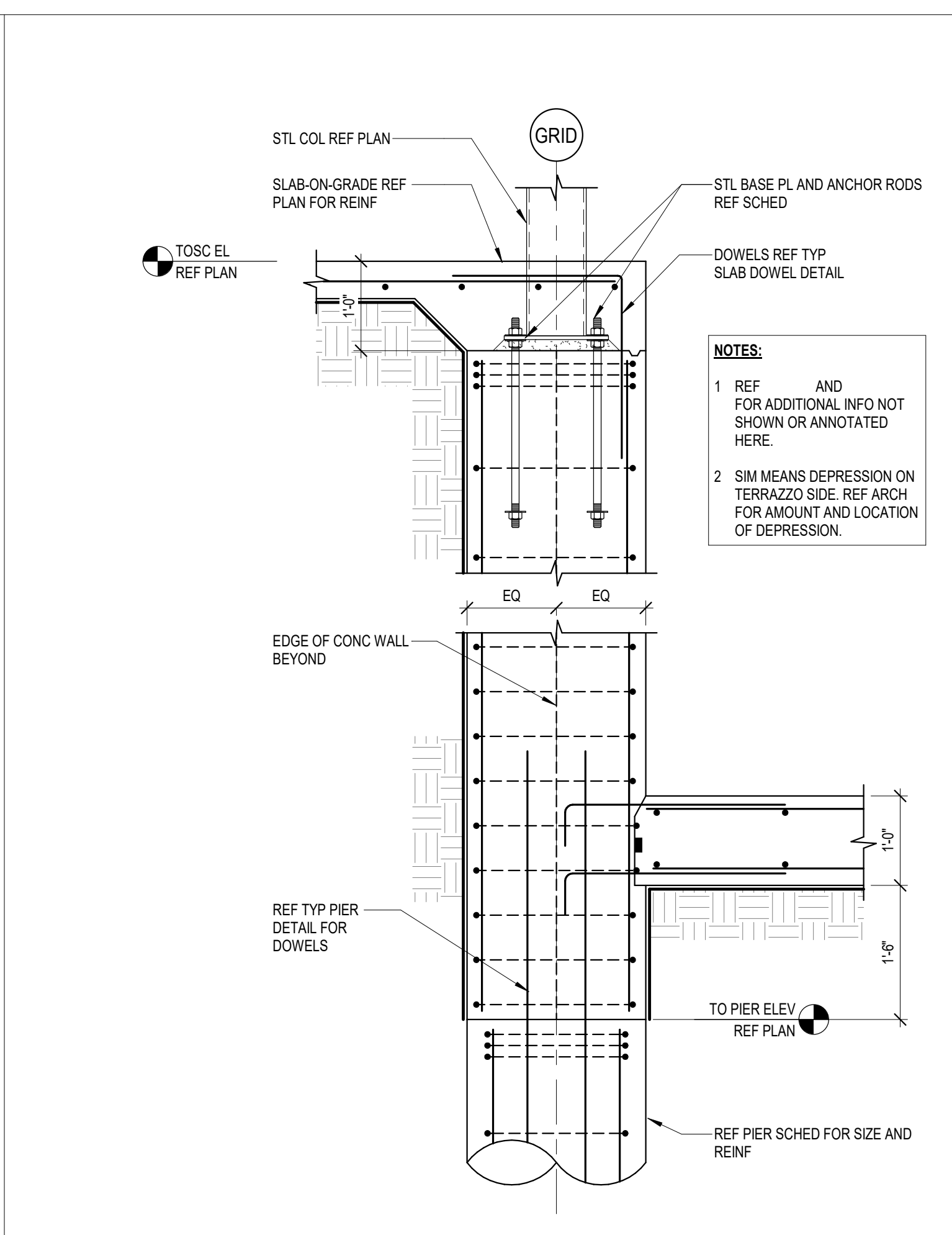
4 ELEVATOR SUMP PIT
 3/4" = 1'-0"



5 ELEVATOR PIT AT GRADE BEAM
 NO SCALE



6 ELEVATOR PIT AT GRADE BEAM AND COLUMN
 NO SCALE



7 ELEVATOR PIT AT COLUMN
 NO SCALE

File Path: C:\Users\jkubala\Documents\IST21_230063 - GSISD Press Box - Courville Stadium_john.kubala.rvt

CHECKED BY:
 Checker
 DRAWN BY:
 Author
 Plot Stamp:
 4/24/2024 10:17:15 AM

MASONRY:

- MASONRY TESTING SHALL CONSIST OF A QUALIFIED TESTING LABORATORY PROVIDING THE FOLLOWING SERVICES:
 - OBSERVE THE INSTALLATION OF MASONRY UNITS.
 - VERIFY THE QUANTITY, SIZE AND THE SPACING OF THE REQUIRED REINFORCING THAT IS SHOWN ON THE DRAWINGS.
 - INSPECT THE GROUT SPACE PRIOR TO THE CLOSING OF CLEANOUTS AND ALL GROUTING OPERATIONS. VERIFY THAT THE SPECIFIED CELLS ARE FULLY GROUTED AS NOTED.
 - MONITOR THE PROPORTIONING, MIXING AND CONSISTENCY OF MORTAR AND GROUT. PROVIDE 28 DAY COMPRESSIVE STRENGTH TESTS ON EACH GROUT MIX IN ACCORDANCE WITH ASTM C1016. PROVIDE COMPRESSIVE TESTS ON MASONRY PRISMS FOR EACH TYPE OF WALL CONSTRUCTION. IN ACCORDANCE WITH ASTM C1314. CONTRACTOR SHALL PREPARE ONE SET FOR TESTING AT 28 DAYS. TESTS ARE TO BE CONDUCTED FOR EACH 2000 SQUARE FEET OF WALL INSTALLED, BUT NOT LESS THAN TWO TESTS.

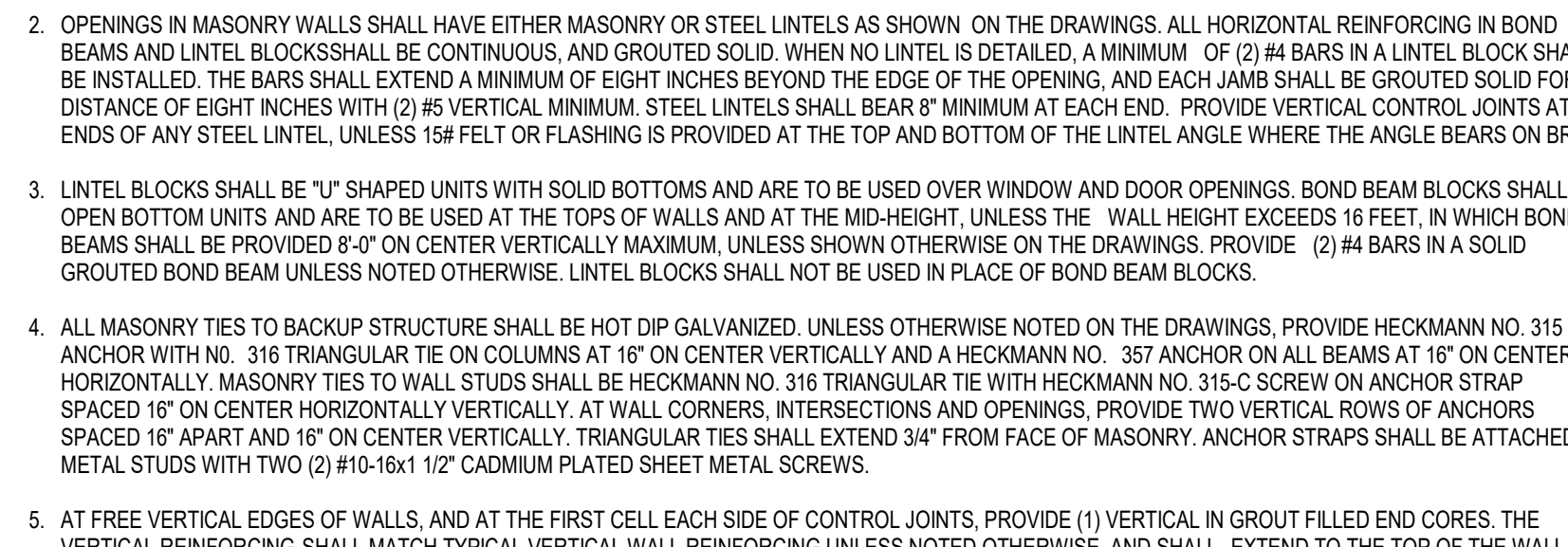
MASONRY NOTES:

- MATERIALS:**
- ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C90, TYPE 1, GRADE N, AND THE QUALITY CONTROL STANDARDS OF THE NATIONAL CONCRETE MASONRY ASSOCIATION.
 - ALL CONCRETE MASONRY UNITS SHALL BE LIGHTWEIGHT (LESS THAN 105 PCF, OPEN DRY UNIT WEIGHT).
 - ALL MASONRY UNITS SHALL HAVE A MAXIMUM LINEAR SHRINKAGE OF .06 OF 1% FROM THE SATURATED TO THE OPEN DRY CONDITION, WHEN TESTED WITH THE METHODS SET FORTH IN THE QUALITY CONTROL STANDARDS OF THE NATIONAL CONCRETE MASONRY ASSOCIATION.
 - MASONRY UNITS SHALL HAVE CURED FOR NOT LESS THAN 28 DAYS WHEN PLACED IN THE STRUCTURE.
- COMPRESSIVE STRENGTHS:**
- THE MINIMUM COMPRESSIVE STRENGTH OF MASONRY (fm) SHALL BE 2000 PSI AS DETERMINED BY THE UNIT STRENGTH METHOD OR BY THE PRISM TEST METHOD.
 - ALL MASONRY UNITS SHALL HAVE A MINIMUM NET COMPRESSIVE STRENGTH OF 2500 PSI AND A MINIMUM NET TENSILE STRENGTH OF 125 PSI, WHEN TESTED IN ACCORDANCE WITH THE METHODS SET FORTH IN THE QUALITY CONTROL STANDARDS OF THE NATIONAL CONCRETE MASONRY ASSOCIATION.
- MORTAR:**
- UNLESS NOTED OTHERWISE, MORTAR SHALL CONFORM TO ASTM C270, TYPE 'S'. MORTAR SHALL BE FRESHLY PREPARED AND UNIFORM/MIXED IN THE RATIO OF 1 PART PORTLAND CEMENT, 1/4 PART MINIMUM TO 1/2 PART MAXIMUM LINE PUTTY OR HYDRATED LIME, DAMP LOOSE SAND NOT LESS THAN 2/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE VOLUMES OF THE CEMENT AND LIME USED.
- GROUT:**
- GROUT FOR POURING SHALL BE OF FLUID CONSISTENCY AND MIXED IN THE RATIO BY VOLUMES, 1 PART PORTLAND CEMENT, 2 1/4 PARTS MINIMUM TO 3 PARTS MAXIMUM DAMP LOOSE SAND, 1 PART MINIMUM TO 2 PARTS MAXIMUM PEA GRAVEL, AND 0 TO 1/10 PART MAXIMUM HYDRATED LIME. GROUT SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI, WHEN TESTED IN ACCORDANCE WITH ASTM C476.
 - GROUT FOR PUMPING SHALL BE OF FLUID CONSISTENCY AND SHALL HAVE NO LESS THAN 7 BAGS OF CEMENT IN EACH CUBIC YARD OF GROUT. SUBMIT THE MIX FOR APPROVAL.
 - ALL CELLS WITH REINFORCING BARS SHALL BE GROUTED SOLID.
 - ALL CELLS THAT WILL HAVE DRILLED EXPANSION ANCHORS, EMBEDDED HEADED STUDS OR OTHER EMBEDDED ANCHORS MUST BE GROUTED SOLID.
- REINFORCEMENT:**
- PROVIDE HORIZONTAL JOINT REINFORCING (TRUSS OR LADDER TYPE, 9 GAGE) AT 16" OC VERTICALLY AT ALL CMU WALLS. PROVIDE PRE-FAB WIRE TIES AT ALL CORNERS AND INTERSECTIONS AT 16" OC. REINFORCEMENT SHALL CONFORM TO ASTM A62 WITH A MINIMUM YIELD STRENGTH OF 60,000 PSI, AND SHALL BE HOT DIPPED GALVANIZED.
 - OPENINGS IN MASONRY WALLS SHALL HAVE EITHER MASONRY OR STEEL LINTELS AS SHOWN ON THE DRAWINGS. ALL HORIZONTAL REINFORCING IN BOND BEAMS AND LINTEL BLOCKS SHALL BE CONTINUED AND GROUTED SOLID. WHEN NO LINTEL IS DETAIL, A MINIMUM OF (2) #4 BARS IN A LINTEL BLOCK SHALL BE INSTALLED. THE BARS SHALL EXTEND A MINIMUM OF EIGHT INCHES BEYOND THE EDGE OF THE OPENING, AND EACH JAMB SHALL BE GROUTED SOLID FOR A DISTANCE OF EIGHT INCHES WITH (2) #5 VERTICAL MINIMUM. STEEL LINTELS SHALL BEAR #3 MINIMUM AT EACH END. PROVIDE VERTICAL CONTROL JOINTS AT THE ENDS OF ANY STEEL LINTEL, UNLESS 15# FELT OR FLASHING IS PROVIDED AT THE TOP AND BOTTOM OF THE LINTEL ANGLE WHERE THE ANGLE BEARS ON BRICK.
 - LINTEL BLOCKS SHALL BE 'U' SHAPED UNITS WITH SOLID BOTTOMS AND ARE TO BE USED OVER WINDOW AND DOOR OPENINGS. BOND BEAM BLOCKS SHALL BE OPEN BOTTOM UNITS AND ARE TO BE USED AT THE TOPS OF WALLS AND AT THE MID-HEIGHT, UNLESS THE WALL HEIGHT EXCEEDS 16 FEET, IN WHICH BOND BEAM SHALL BE PROVIDED 8'-0" ON CENTER VERTICALLY MAXIMUM, UNLESS SHOWN OTHERWISE ON THE DRAWINGS. PROVIDE (2) #4 BARS IN A SOLID GROUTED BOND BEAM UNLESS NOTED OTHERWISE. LINTEL BLOCKS SHALL NOT BE USED IN PLACE OF BOND BEAM BLOCKS.
 - ALL MASONRY TIES TO BACKUP STRUCTURE SHALL BE HOT DIP GALVANIZED, UNLESS OTHERWISE NOTED ON THE DRAWINGS. PROVIDE HECKMANN NO. 315 ANCHOR WITH NO. 316 TRIANGULAR TIE ON COLUMNS AT 16" ON CENTER VERTICALLY AND A HECKMANN NO. 357 ANCHOR ON ALL BEAMS AT 16" ON CENTER HORIZONTALLY. MASONRY TIES TO WALL STUDS WITH HECKMANN NO. 316 TRIANGULAR TIE WITH HECKMANN NO. 315-C SCREW ON ANCHOR STRAP SPACED 16" ON CENTER HORIZONTALLY VERTICALLY AT WALL CORNERS, INTERSECTIONS AND OPENINGS. PROVIDE TWO VERTICAL ROWS OF ANCHORS SPACED 16" APART AND 16" ON CENTER VERTICALLY. TRIANGULAR TIES SHALL EXTEND 3/4" FROM FACE OF MASONRY. ANCHOR STRAPS SHALL BE ATTACHED TO METAL STUDS WITH TWO (2) #10-18x1 1/2" CADMIUM PLATED SHEET METAL SCREWS.
 - AT FREE VERTICAL EDGES OF WALLS, AND AT THE FIRST CELL EACH SIDE OF CONTROL JOINTS, PROVIDE (1) VERTICAL IN GROUT FILLED END CORES. THE VERTICAL REINFORCING SHALL MATCH TYPICAL VERTICAL WALL REINFORCING UNLESS NOTED OTHERWISE, AND SHALL EXTEND TO THE TOP OF THE WALL.
 - UNLESS OTHERWISE SHOWN ON THE DRAWINGS, PROVIDE A MINIMUM OF #4 VERTICAL BARS AND DOWELS IN FULLY GROUTED CELLS AT 48" O.C.
 - ALL BAR REINFORCING SHALL BE LAPPED AS FOLLOWS:
 - #3 LAP SPlice LENGTH = 19 INCHES
 - #4 LAP SPlice LENGTH = 34 INCHES
 - #5 LAP SPlice LENGTH = 45 INCHES
 - #6 LAP SPlice LENGTH = 54 INCHES
 - #7 LAP SPlice LENGTH = 63 INCHES
 - #8 LAP SPlice LENGTH = 72 INCHES
 - #9 LAP SPlice LENGTH = 82 INCHES
 - INCREASE LAP SPlice LENGTHS BY 50% FOR EPOXY COATED REINFORCING

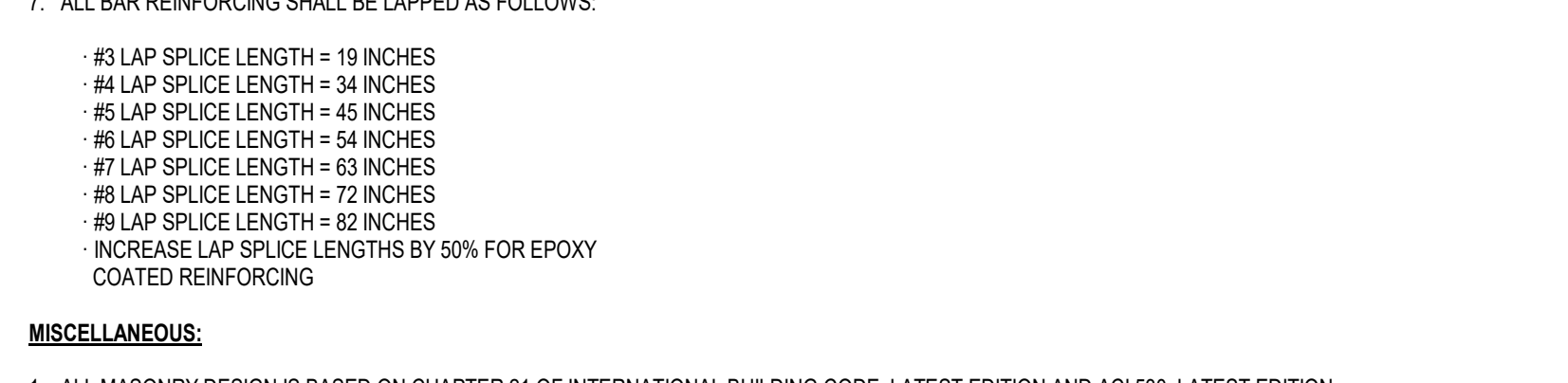
MISCELLANEOUS:

- ALL MASONRY DESIGN IS BASED ON CHAPTER 21 OF INTERNATIONAL BUILDING CODE, LATEST EDITION AND ACI 530, LATEST EDITION.
- SEE TYPICAL DETAILS FOR INTERIOR WALL BRACING, AND REINFORCING REQUIREMENTS.
- UNLESS NOTED OTHERWISE PER PLANS, SPECS OR DETAILS, MASONRY WALLS SHALL HAVE VERTICAL CONTROL JOINTS PER NCMA GUIDELINES TEK 10-20 AS FOLLOWS:
 - AT APPROXIMATELY SIXTEEN FEET ON CENTER CORNERS AT A MAXIMUM DISTANCE NOT TO EXCEED ONE-HALF THE REQUIRED MAXIMUM DISTANCE BETWEEN MAIN AND INTERSECTING WALLS CHANGES IN WALL HEIGHTS
 - ADJACENT TO LINTELS AND THROUGH OPENINGS IF NOT CROSSING VERTICAL REINFORCEMENT
 - AT PILASTERS AND CHANGES IN WALL THICKNESS
 - ALL JOINT LOCATIONS SHALL BE COORDINATED WITH THE ARCHITECT.
- UNLESS NOTED OTHERWISE PER PLANS, SPECS OR DETAILS, BRICK VENEER CONTROL JOINTS SHALL BE PER BIA GUIDELINES TECHNICAL NOTE 18A.
 - WITHOUT OPENINGS, SPACE NO MORE THAN 25 FT
 - WITH MULTIPLE OPENINGS, SPACE NO MORE THAN 20 FT
 - AT OR NEAR CORNERS
 - AT OFFSETS AND SETBACKS
 - AT WALL INTERSECTIONS
 - AT CHANGES IN WALL HEIGHTS
 - AT WALL BRACING SYSTEM CHANGES (CMU TO DRYWALL)
 - AT SUPPORT CHANGES (CONCRETE TO STEEL)
 - AT WALL FUNCTION OR CLIMATIC EXPOSURE CHANGES (INTERIOR TO EXTERIOR)
- GROUT LIFT LIMITS
 - GROUT POURS SHALL NOT EXCEED 5 FEET PER LIFT WHEN GROUTING THE CELLS OF REINFORCED CMU UNLESS CLEANOUTS ARE PROVIDED IN THE BOTTOM COURSE OF EACH 5 FOOT SECTION
 - GROUT POURS SHALL NOT EXCEED 24 FEET WHEN GROUTING THE CELLS OF HOLLOW CMU. WHEN GROUTING THE SPACE BETWEEN MULTIPLE WALLS, THE TYPICAL POUR SHALL NOT EXCEED 24 FEET FOR 3" SPACES, 12 FEET FOR 1 1/2" SPACES, AND 8 FEET FOR 2" SPACES.
 - MECHANICALLY VIBRATE ALL LIFTS IN EXCESS OF 1 FOOT.
 - ALL GROUT MUST BE PLACED WITHIN 1 1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE.
 - GROUT LIFTS SHALL NOT BE STOPPED WITHIN 1 1/2" OF BED JOINT.
 - ALL CMU WALLS LOCATED ADJACENT TO EARTH FILL MUST BE FULLY GROUTED DIRECTLY ADJACENT TO, AND AT EAST 8' ABOVE, ALL SOIL IN CONTACT WITH THE WALL.

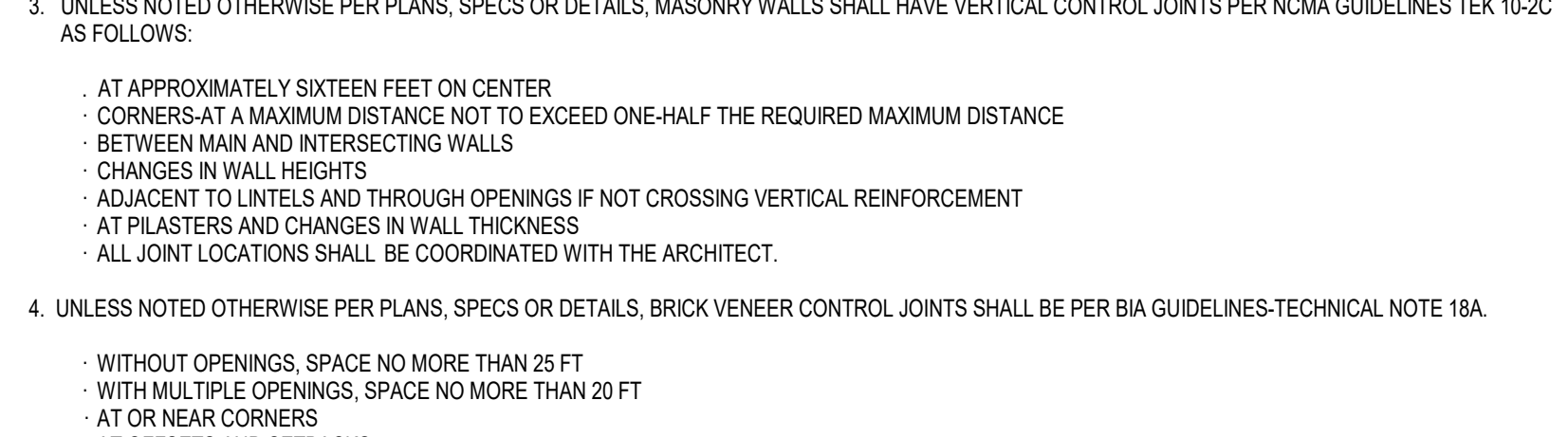
STEEL BEAM BEARING ON MASONRY WALL



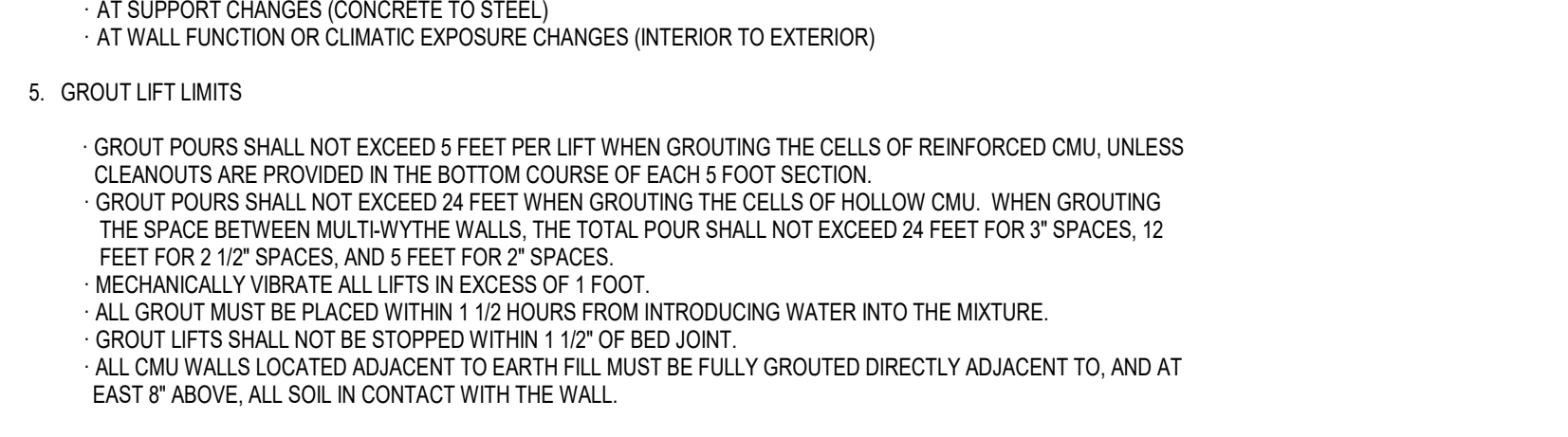
STEEEL BEAM BEARING ON MASONRY WALL



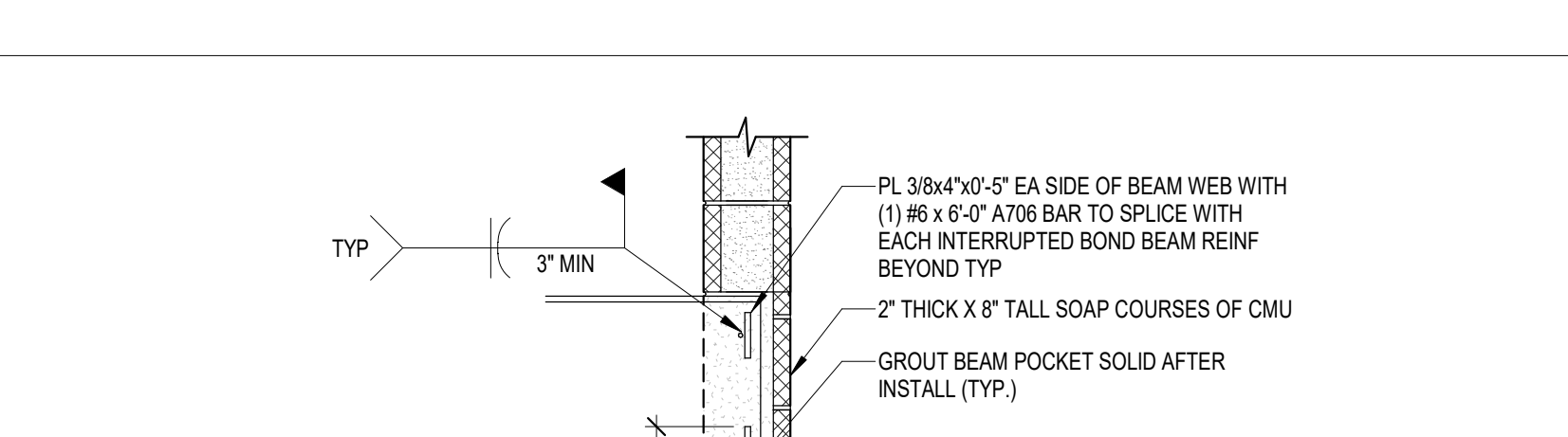
STEEL BEAM BEARING ON MASONRY WALL



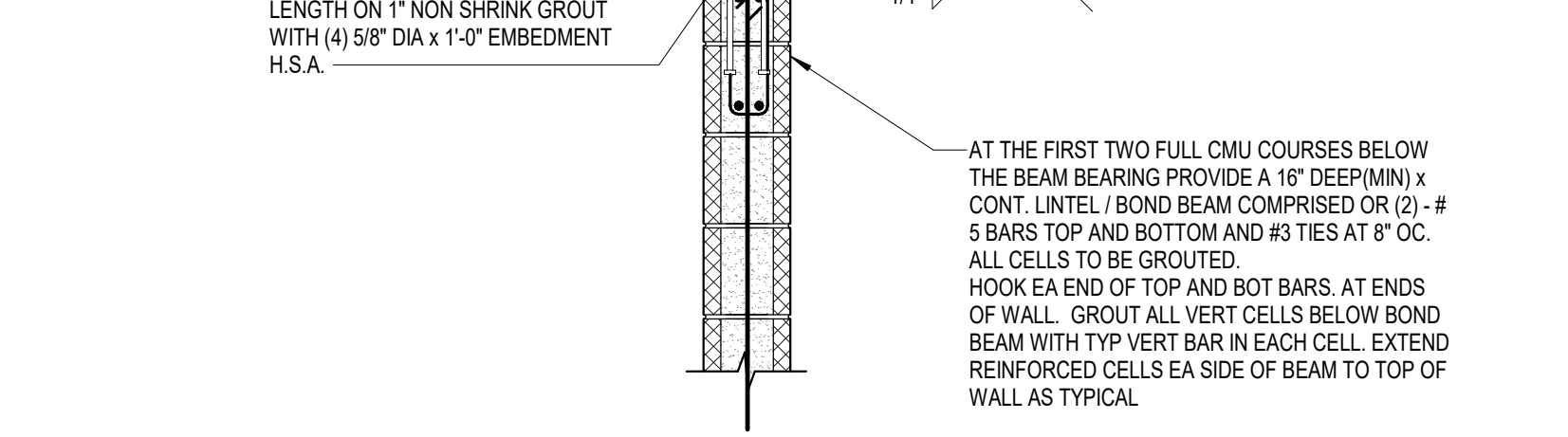
STEEL BEAM BEARING ON MASONRY WALL



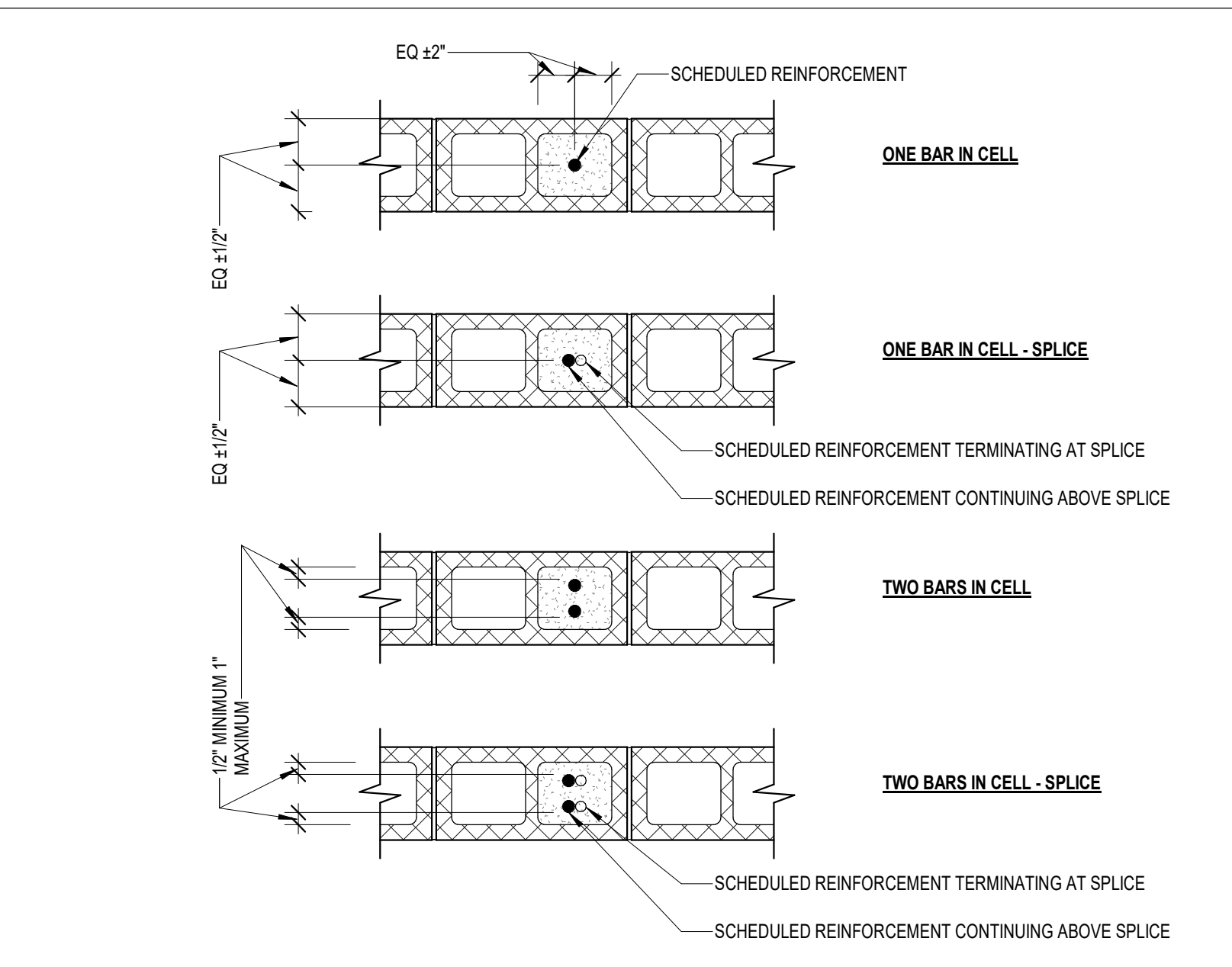
STEEL BEAM BEARING ON MASONRY WALL



STEEL BEAM BEARING ON MASONRY WALL



STEEL BEAM BEARING ON MASONRY WALL

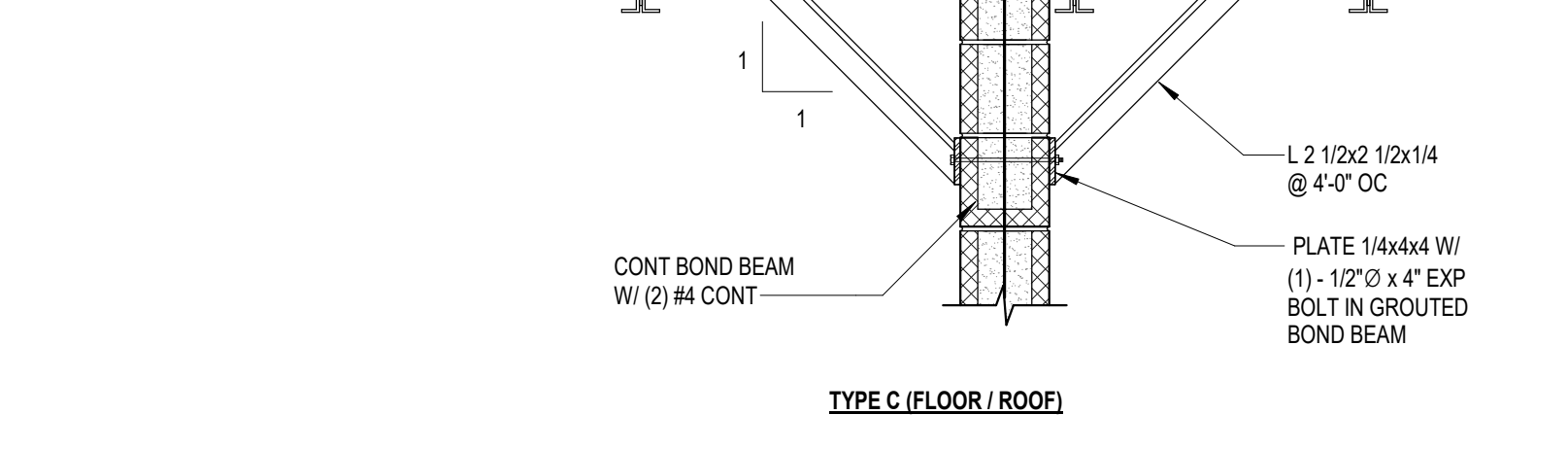
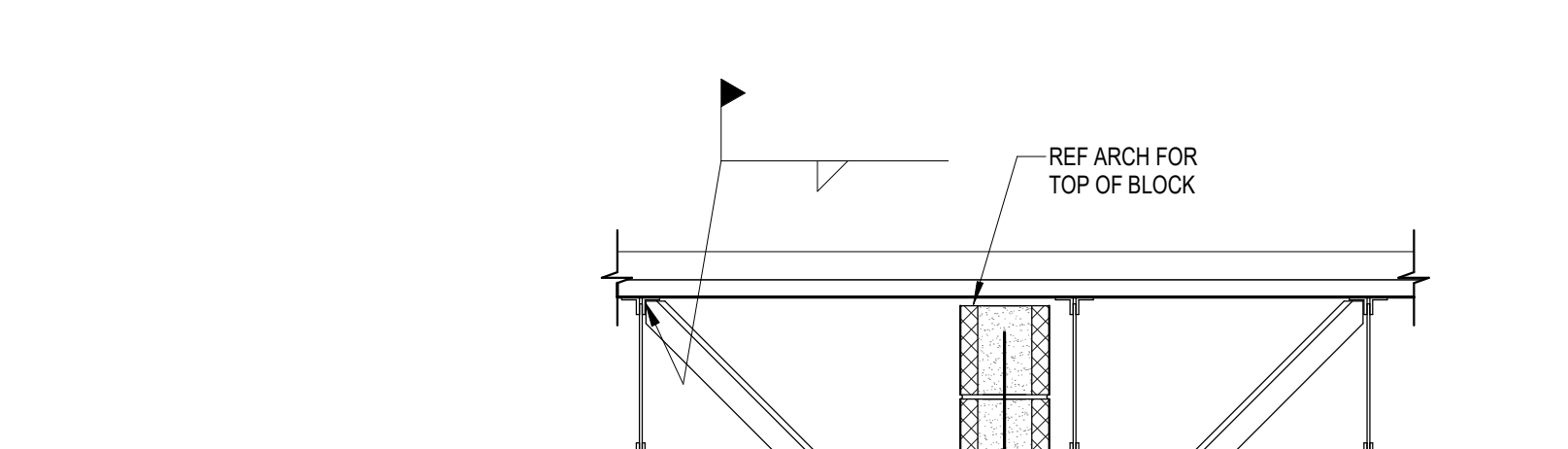
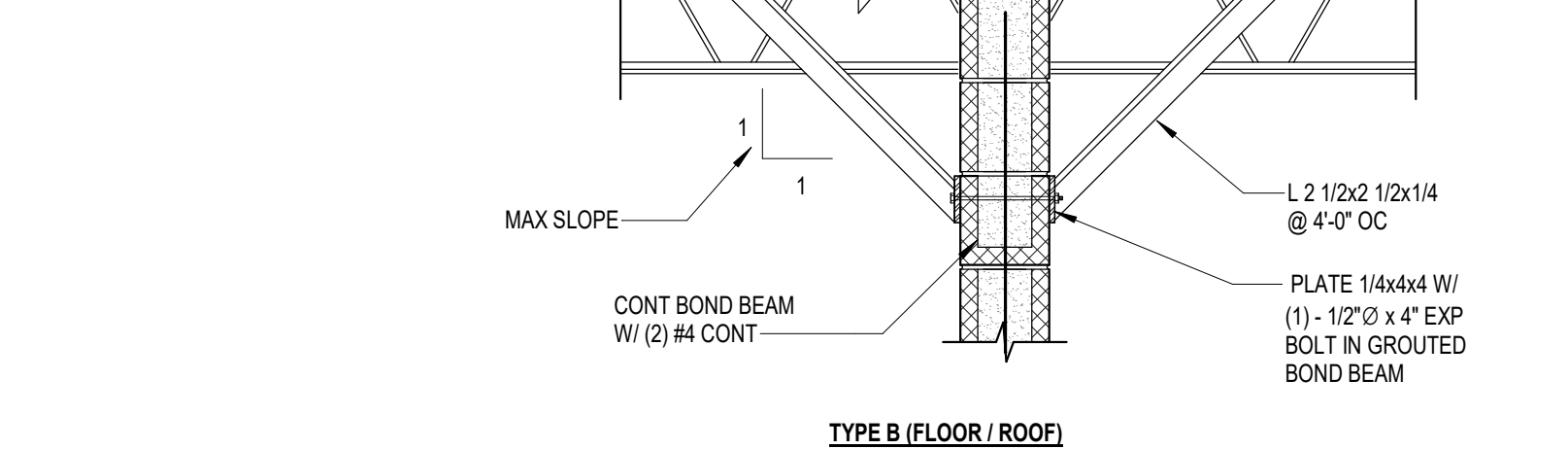
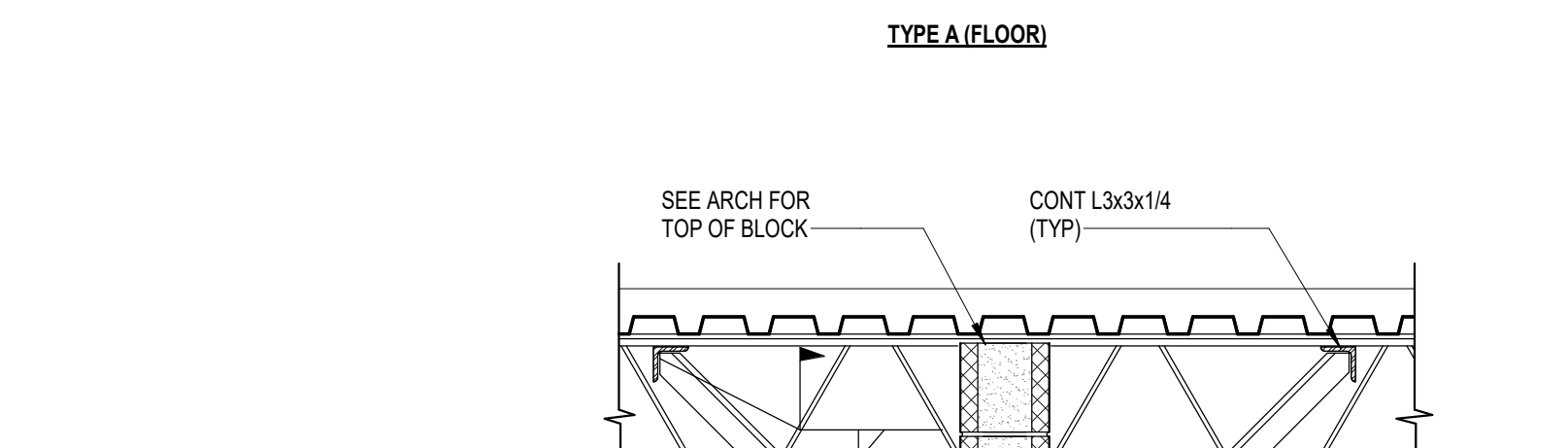
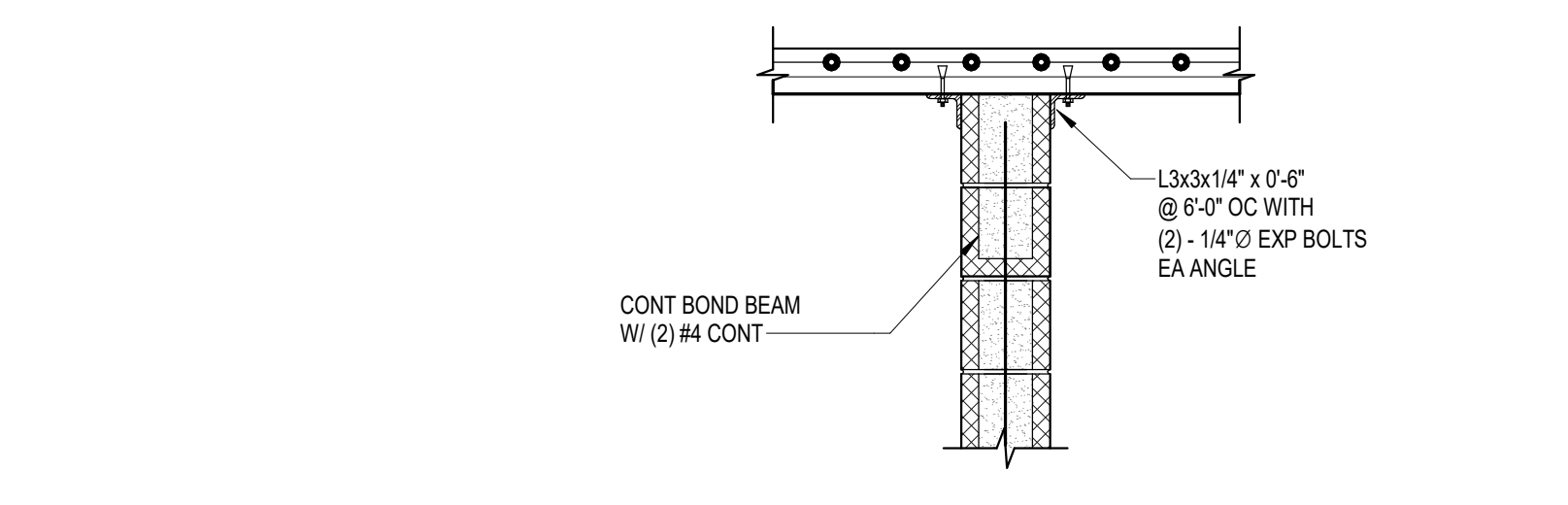


1 TYPICAL CMU VERTICAL BAR PLACEMENT
NO SCALE

BRACE FRAMING (3 5/8\" STUDS)				
STUD TYPE	STUD PROPERTIES	SPACING	MAX LENGTH	MISCELLANEOUS
SJ 20 (40 KSI)	lx = 0.541 IN ⁴	4'-0\" OC	14'-0\"	BRACE @ MID-PT FOR LENGTHS OVER 14'-0\"
	rx = 1.429 IN			
	A = 0.2136 IN ²			
	Sx = 0.273 IN ³			

VERTICAL FRAMING (3 5/8\" STUDS)				
STUD TYPE	STUD PROPERTIES	SPACING	MAX LENGTH	MISCELLANEOUS
ST 25 (33 KSI)	lx = 0.239 IN ⁴	1'-4\" OC	16'-0\"	ONE LAYER SHEATHING (MIN)
	rx = 1.415 IN			
	A = 0.163 IN ²			
	Sx = 0.113 IN ³			
ST 20 (33 KSI)	lx = 0.144 IN ⁴	1'-4\" OC	22'-0\"	ONE LAYER SHEATHING (MIN)
	rx = 1.407 IN			
	A = 0.210 IN ²			
	Sx = 0.213 IN ³			

*OR PROVIDE 1 1/2x1 1/2x1/16 GA HORIZONTAL CHANNEL FASTENED TO STUDS WITH 1 1/2x1 1/2x1/4 GA CLIP AT 4'-0\" VERTICALLY



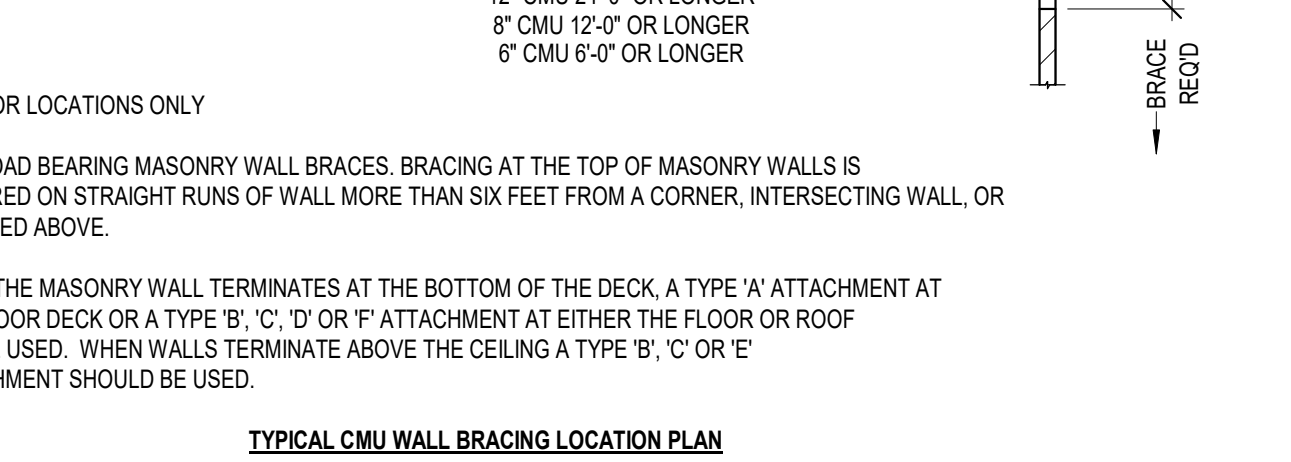
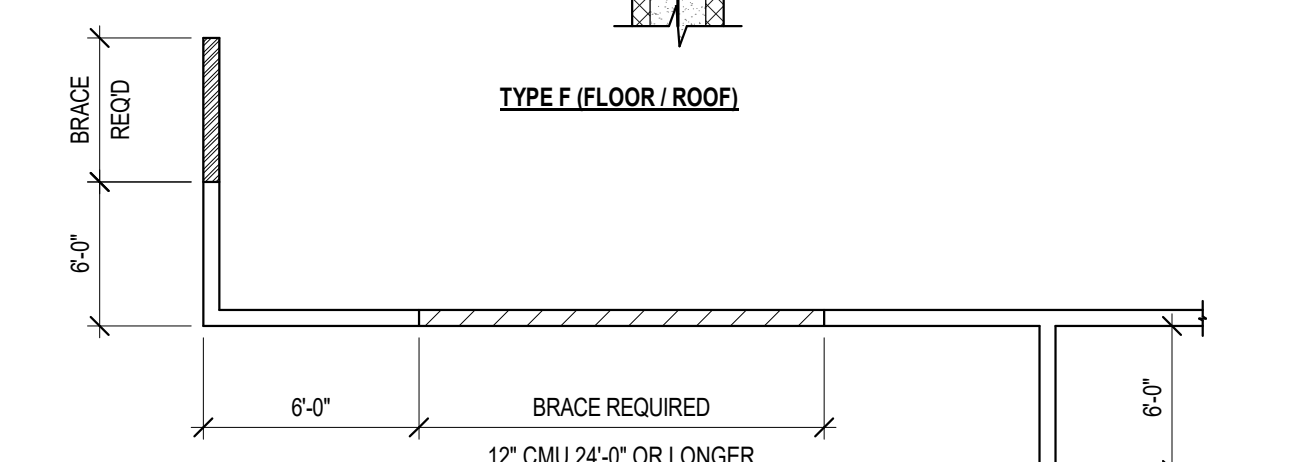
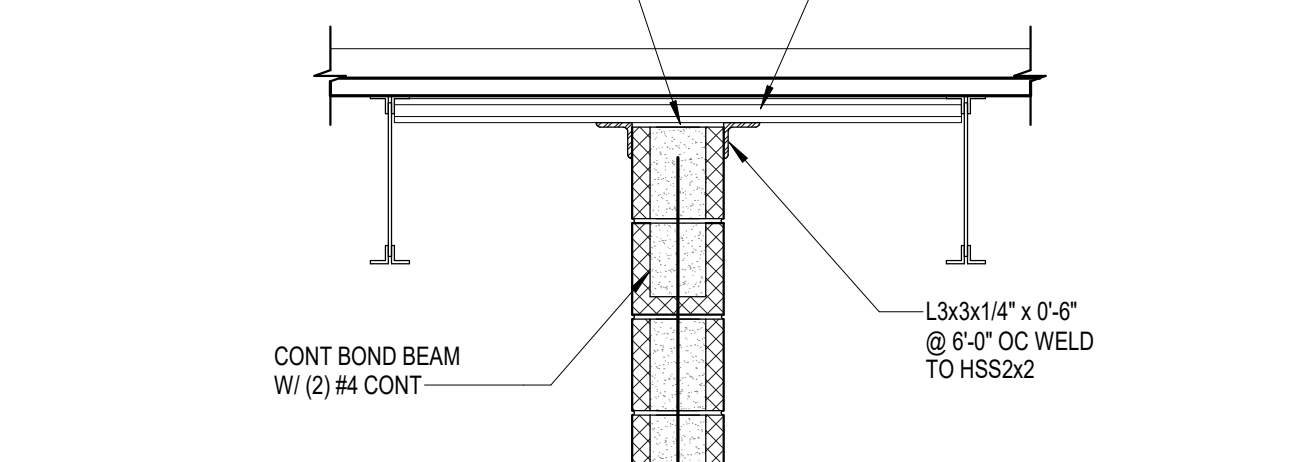
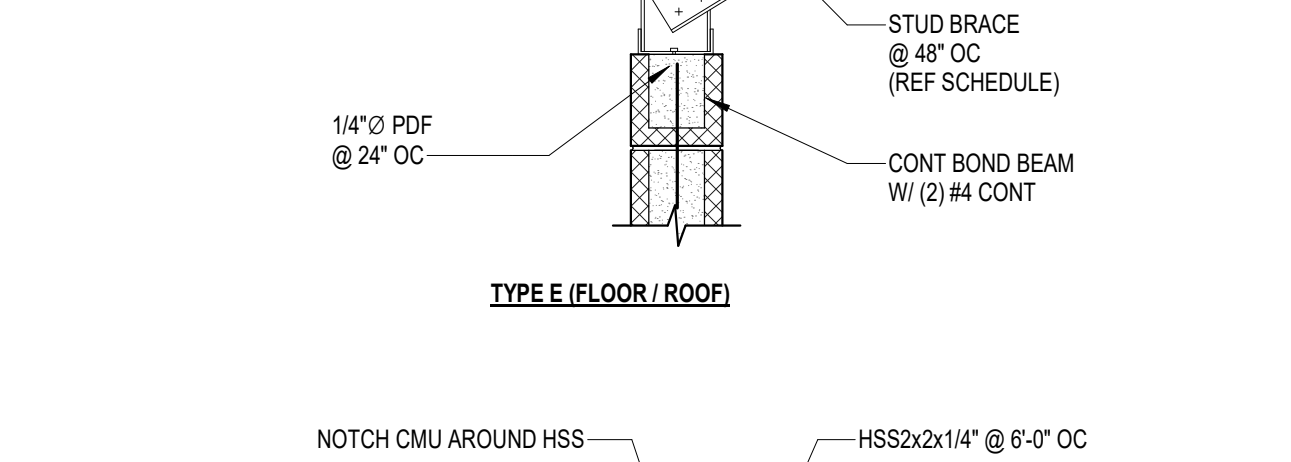
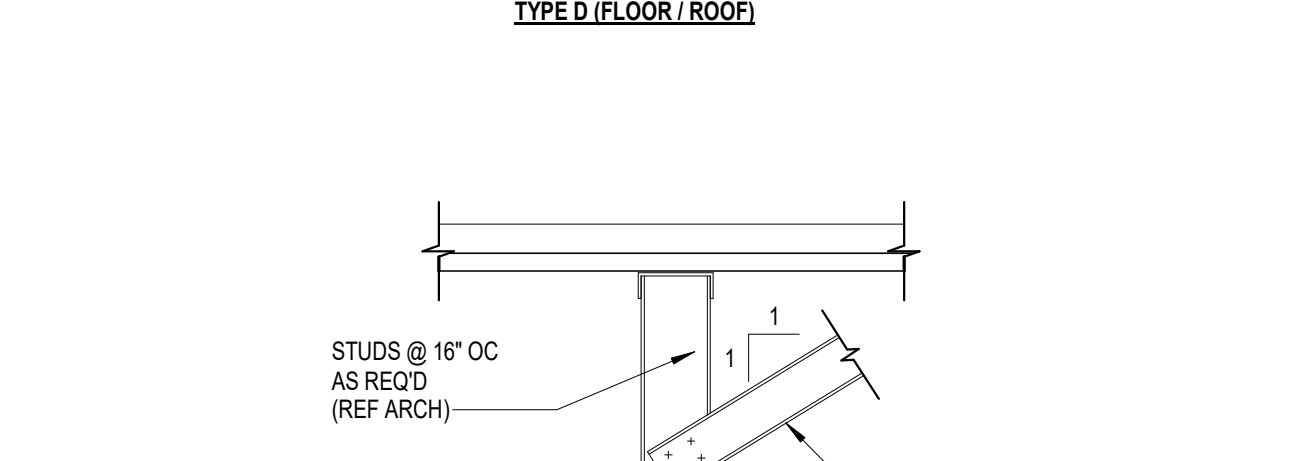
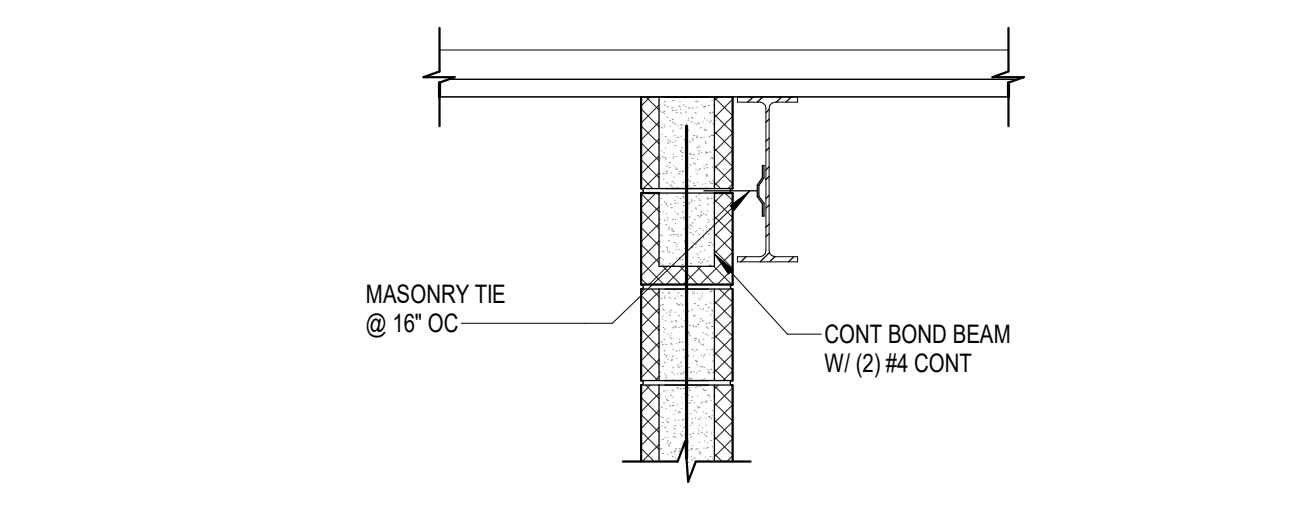
5 TYPICAL CMU WALL BRACE CONNECTIONS
NO SCALE

- NOTES:**
- REINFORCEMENT MUST BE PLACED USING REINFORCING BAR POSITIONERS THAT LOCATE THE BAR AS SPECIFIED AND PREVENT MOVEMENT OF THE BAR DURING CONSTRUCTION.
 - SPLICED REINFORCEMENT MUST BE A CONTACT LAP SPICE WITH SPLICED BARS ALIGNED PARALLEL TO THE WALL AS SHOWN.
 - THE ENGINEER MUST BE NOTIFIED PRIOR TO PLACEMENT OF REINFORCEMENT THAT IS REQUIRED TO BE PLACED OUTSIDE OF THE TOLERANCES OF THIS DETAIL SUCH AS TO AVOID INTERFERENCE WITH OTHER REINFORCEMENT, CONDUITS, OR EMBEDDED ITEMS.

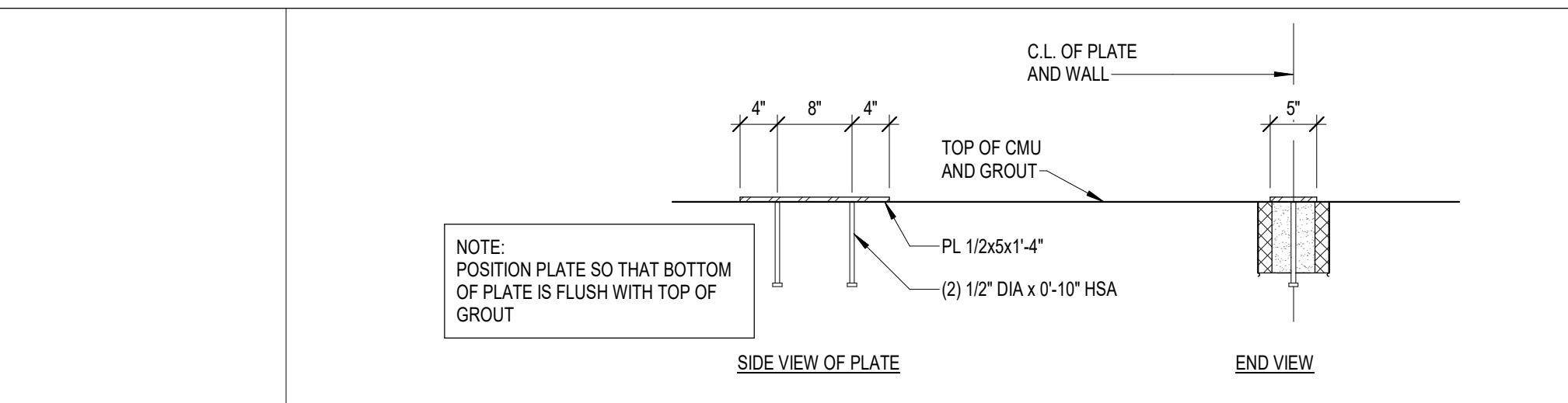
BRACE FRAMING (6\" STUDS)				
STUD TYPE	STUD PROPERTIES	SPACING	MAX LENGTH	MISCELLANEOUS
SJ 20 (40 KSI)	lx = 1.787 IN ⁴	4'-0\" OC	20'-0\"	BRACE @ MID-PT FOR LENGTHS OVER 20'-0\"
	rx = 2.253 IN			
	A = 0.2148 IN ²			
	Sx = 0.539 IN ³			

VERTICAL FRAMING (6\" STUDS)				
STUD TYPE	STUD PROPERTIES	SPACING	MAX LENGTH	MISCELLANEOUS
ST 25 (33 KSI)	lx = 0.773 IN ⁴	1'-4\" OC	20'-0\"	ONE LAYER SHEATHING (MIN)
	rx = 2.209 IN			
	A = 0.167 IN ²			
	Sx = 0.184 IN ³			
ST 20 (33 KSI)	lx = 1.385 IN ⁴	1'-4\" OC	32'-0\"	ONE LAYER SHEATHING (MIN)
	rx = 2.199 IN			
	A = 0.288 IN ²			
	Sx = 0.437 IN ³			

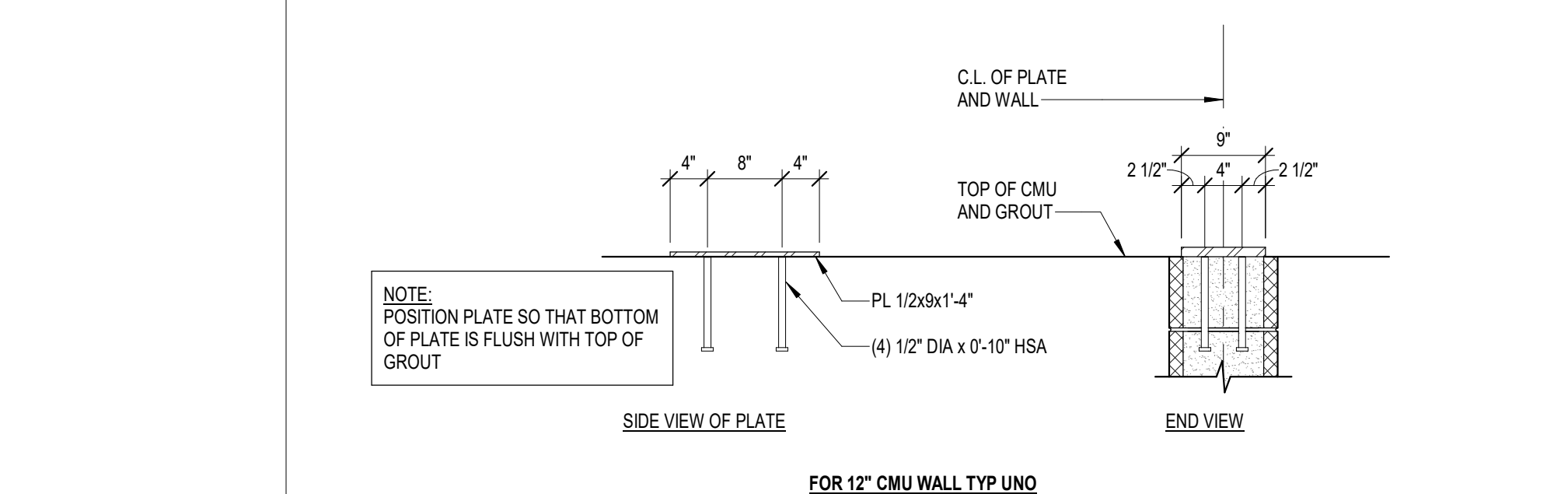
*OR PROVIDE 1 1/2x1 1/2x1/16 GA HORIZONTAL CHANNEL FASTENED TO STUDS WITH 1 1/2x1 1/2x1/4 GA CLIP AT 4'-0\" VERTICALLY



TYPICAL CMU WALL BRACING LOCATION PLAN

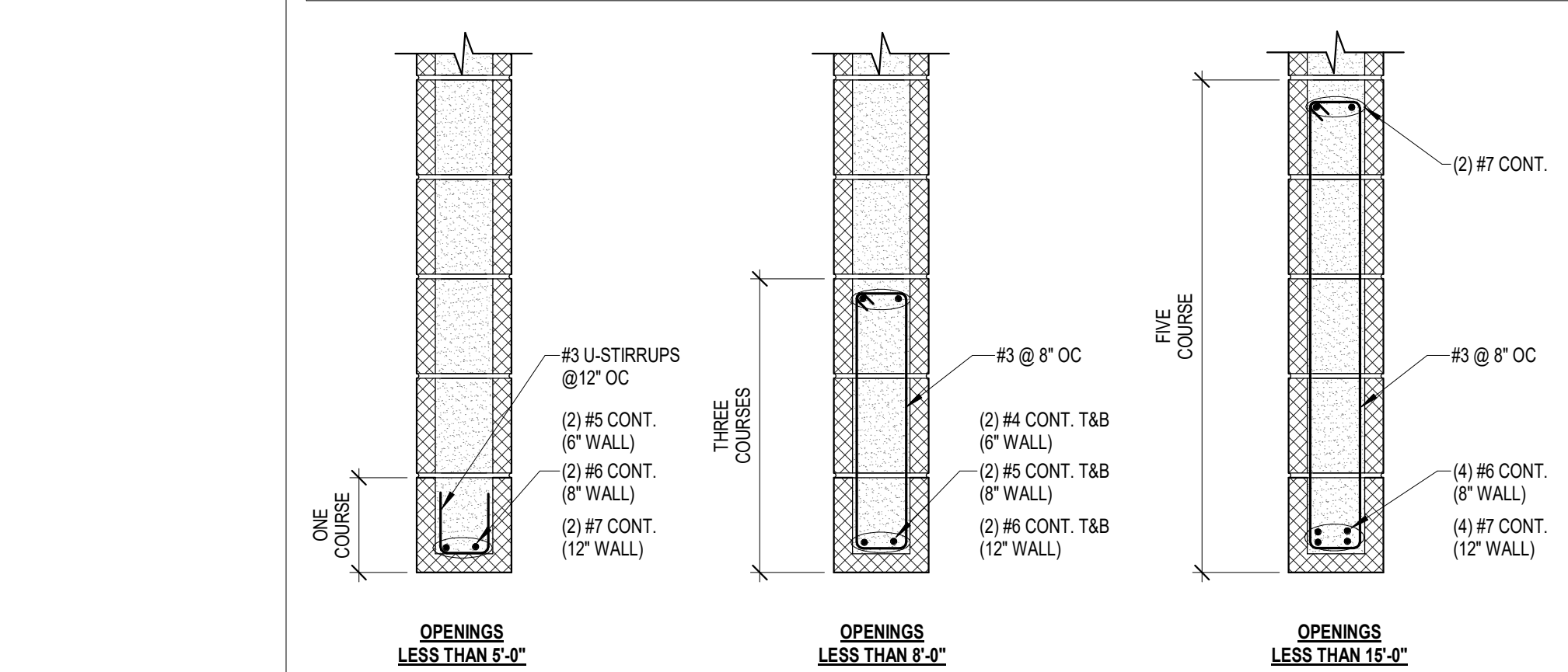


2 EMBED AT JOIST OR DECK SUPPORT ELEMENT
3/4\" = 1'-0\"

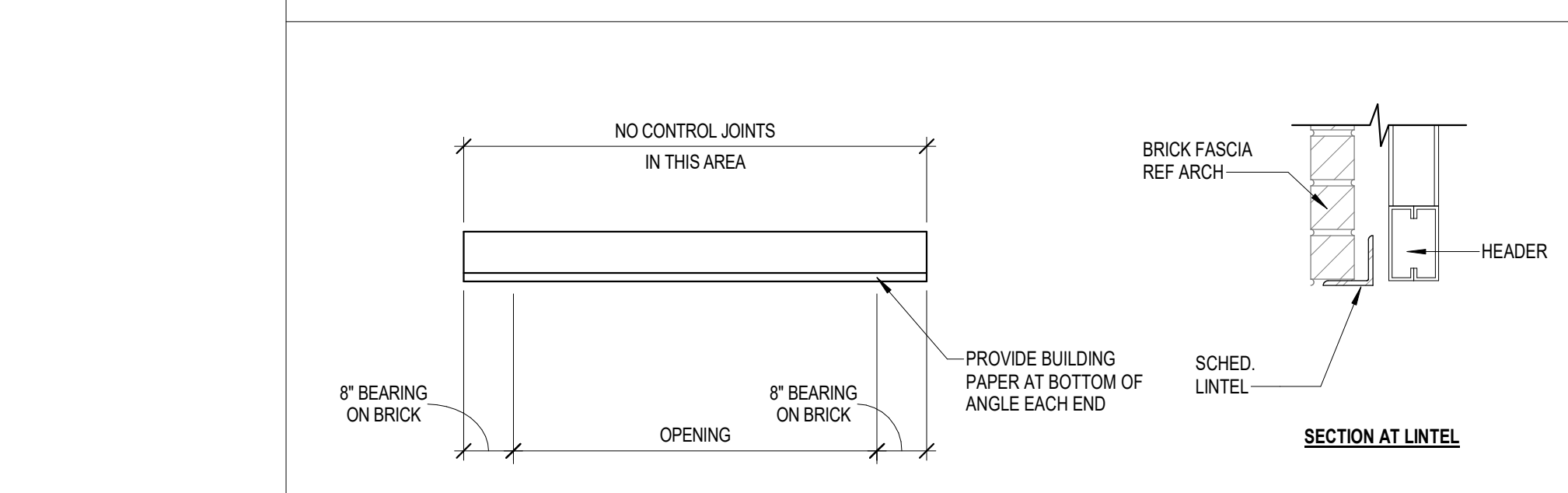


2 EMBED AT JOIST OR DECK SUPPORT ELEMENT
3/4\" = 1'-0\"

- NOTES:**
- SEE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATIONS OF OPENINGS. PROVIDE BLOCK LINTELS FOR ALL OPENINGS AT INTERIOR AND EXTERIOR WALLS FOR WHICH A STEEL LINTEL IS NOT SCHEDULED.
 - PROVIDE 1\" OF BEARING @ EACH JAMB FOR EACH FOOT OF CLEARSPAN BUT NOT LESS THAN 8\". REINFORCING SHALL EXTEND A MINIMUM OF 6\" ONTO THE BEARING.
 - FOR REINFORCED CMU WALLS AT OPENINGS, PROVIDE EXTRA REINFORCED GROUTED FULL HEIGHT CONSECUTIVE CELLS EACH SIDE OF THE OPENING EQUAL TO ONE HALF OF THE TOTAL NUMBER OF CELLS INTERRUPTED BY THE OPENING. REINFORCE EACH CELL WITH THE SAME SIZE AND NUMBER OF BARS AS SPECIFIED FOR THE INTERRUPTED CELLS. PROVIDE A MINIMUM OF (2) REINFORCED GROUTED CELLS EACH SIDE OF OPENING. REFER TO TYPICAL CMU WALL OPENING DIAGRAM AND SCHEDULE FOR ADDITIONAL INFORMATION.
 - AS AN ALTERNATIVE TO USING A FILLED CMU BLOCK LINTEL (PER THE SCHEDULE ABOVE) FOR OPENINGS 5'-0\" AND LESS, CONTRACTOR MAY USE CAST CRETE LINTEL (86).

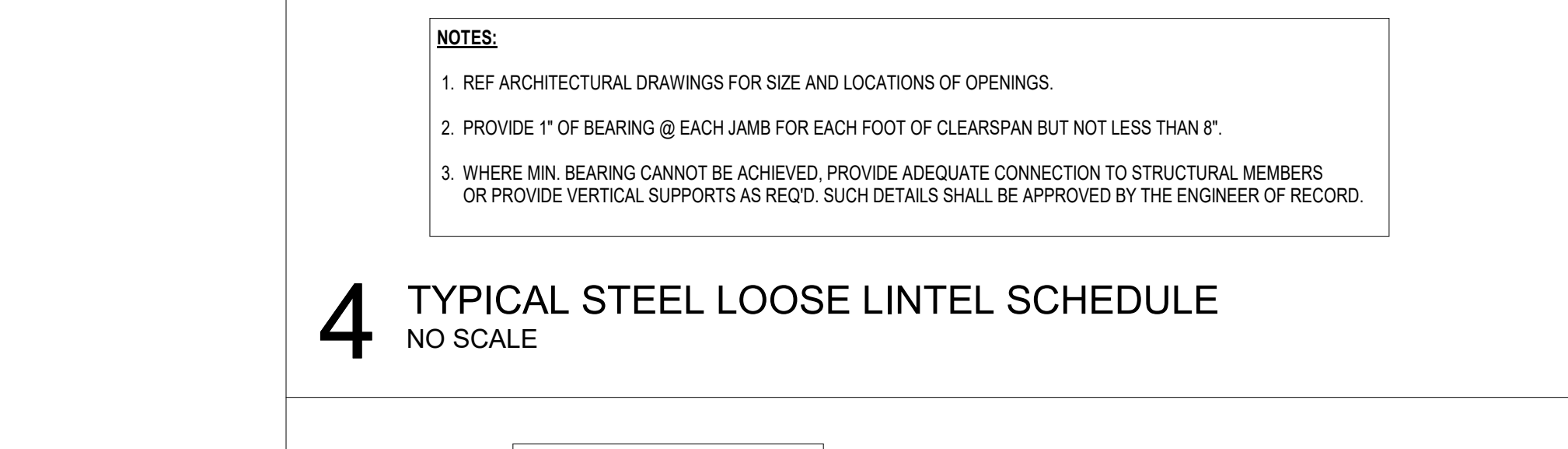


3 MASONRY LINTEL SCHEDULE
NO SCALE

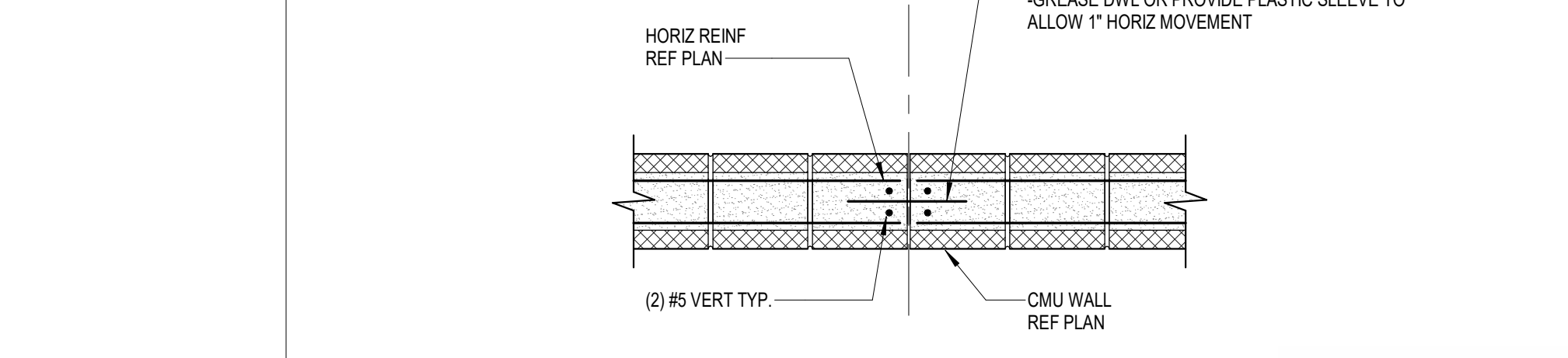


CLEAR OPENING	MINIMUM ANGLE SIZE
0 TO LESS THAN 6'-0"	L 3 1/2 x 3 1/2 x 5/16 (GALV)
6'-0" TO LESS THAN 7'-0"	L 5 x 3 1/2 x 5/16 (LLV) (GALV)
7'-0" TO LESS THAN 8'-0"	L 6 x 3 1/2 x 5/16 (LLV) (GALV)

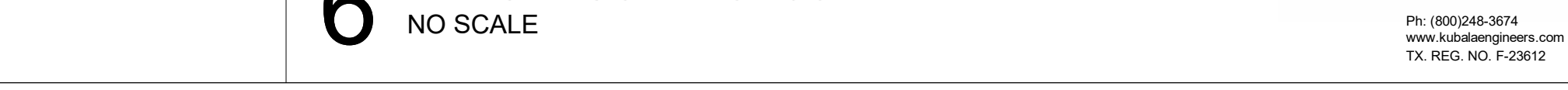
4 TYPICAL STEEL LOOSE LINTEL SCHEDULE
NO SCALE



4 TYPICAL STEEL LOOSE LINTEL SCHEDULE
NO SCALE



6 TYPICAL CONTROL JOINT DETAIL
NO SCALE



ARCHITECT: HOUSTON, TX 77046
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0088 P
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TX Firm: BR 1008

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KEY PLAN

NORTH PLAN TRUE

DISCIPLINE
STATE OF TEXAS
REGISTERED PROFESSIONAL ENGINEER
JOHN R. KUBALA
106120
06/24/2024
Kubala Engineers
F-23612

CLIENT: GALVESTON ISD

DATE: 01/31/2024 PROJECT NUMBER: 230063

No.	Description	Date
1	AS1	04/24/2024

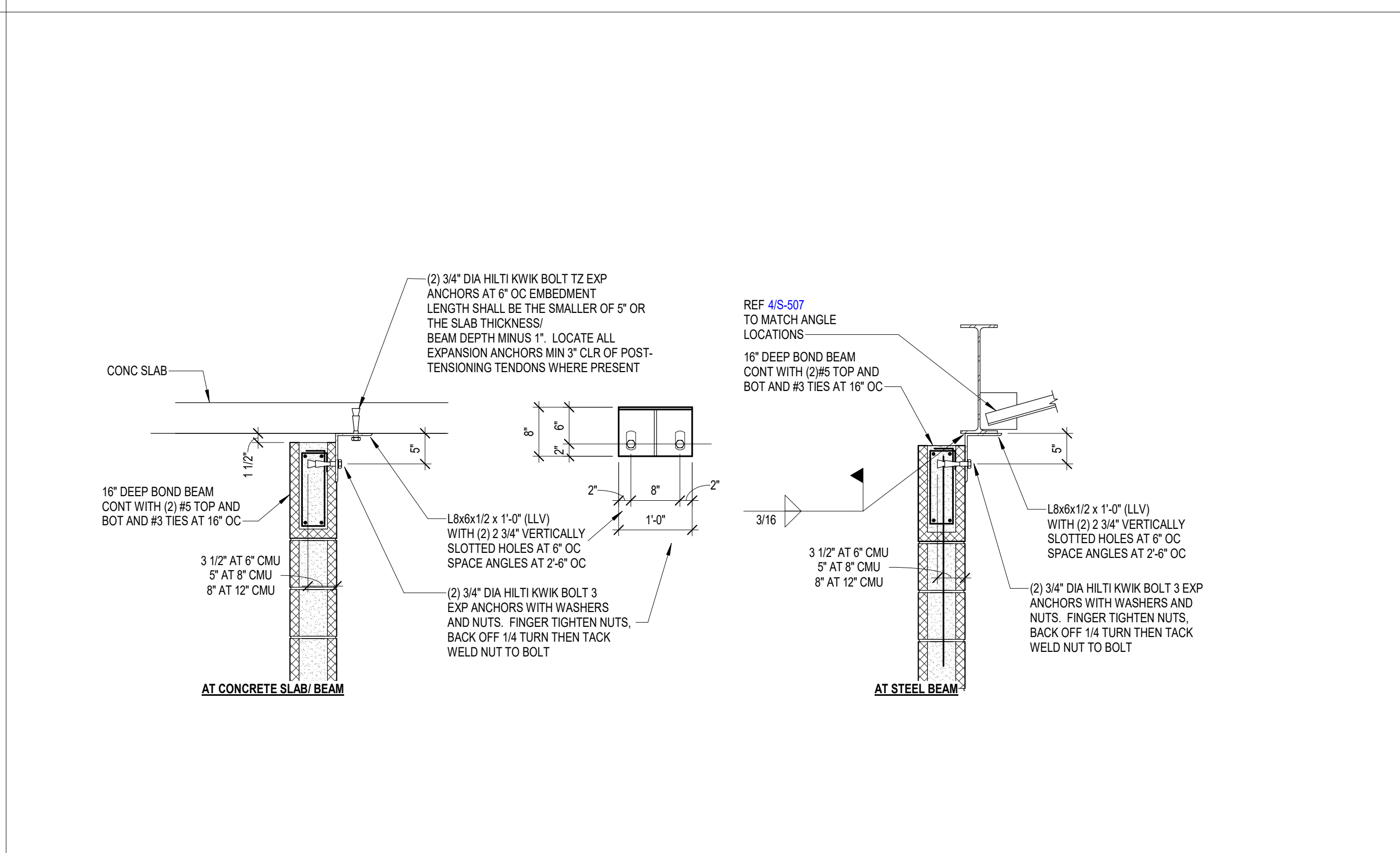
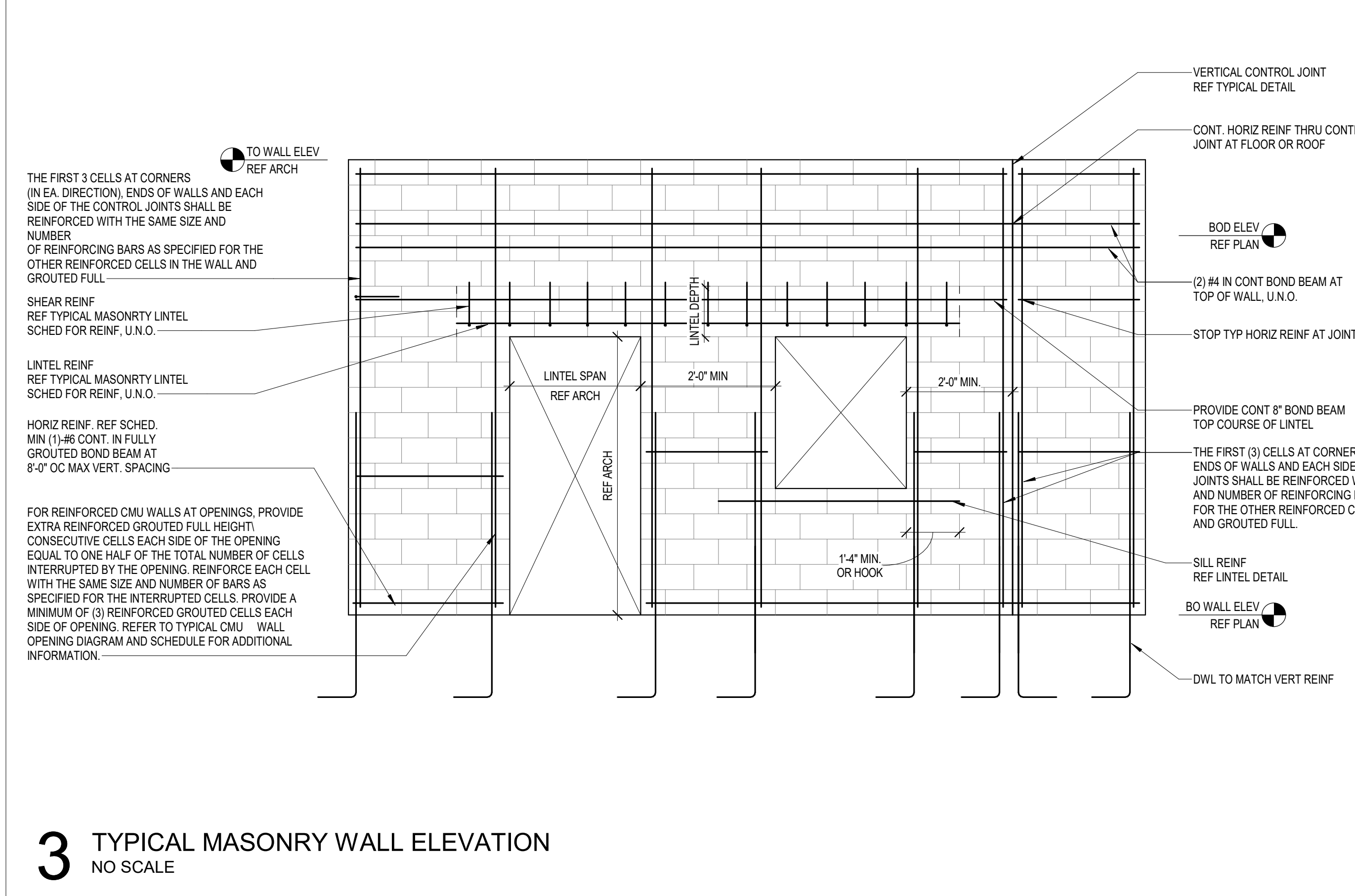
ISSUE FOR PROPOSAL

BUILDING NUMBER

GENERAL CMU NOTES AND TYP DETAILS

S-400

- NOTES:
- REFER TO ARCH AND MECH DRAWINGS FOR SIZE AND LOCATION OF OPENINGS.
 - REFER TO SCHEDULES FOR LINTEL AND JAMB REINFORCING UNLESS DETAILED OTHERWISE.
 - SPICES IN VERT REIN REF CMU WALL REIN DIAGRAM.
 - CMU WALL STARTS ABOVE FLOOR.
 - EXTEND GROUDED LINTEL A MINIMUM OF 2'-0" BEYOND FACE OF OPENING EACH SIDE FOR STRAIGHT LINTEL REIN AND 1'-0" FOR LINTEL REIN WITH STANDARD ACI HOOK.
 - USE LINTEL BLOCKS ONLY FOR BOTTOM COURSE OF LINTEL BEAMS OVER OPENINGS. LINTEL SHALL REMAIN SHORED UNTIL MASONRY CONSTRUCTION ABOVE HAS REACHED 100% OF THE SPECIFIED 28 DAY COMPRESSIVE STRENGTH.
 - CONTINUE VERT WALL REIN OVER OPENING. ANCHOR VERT REIN INTO LINTEL BEAM WITH STANDARD ACI 90° HOOK.
 - ALL VERT BARS AT DOOR JAMBS TO BE FULL HEIGHT.
 - #5 SILL REIN. SOLID GROUT SILL.
 - WHERE HORIZONTAL REIN IS TERMINATED BY OPENING OR CONTROL JOINT PROVIDE STANDARD ACI HOOK WITH VERT WALL REIN IN THE END CELL.
 - CONTINUOUS BOND BEAM REF CMU WALL REIN DIAGRAM.

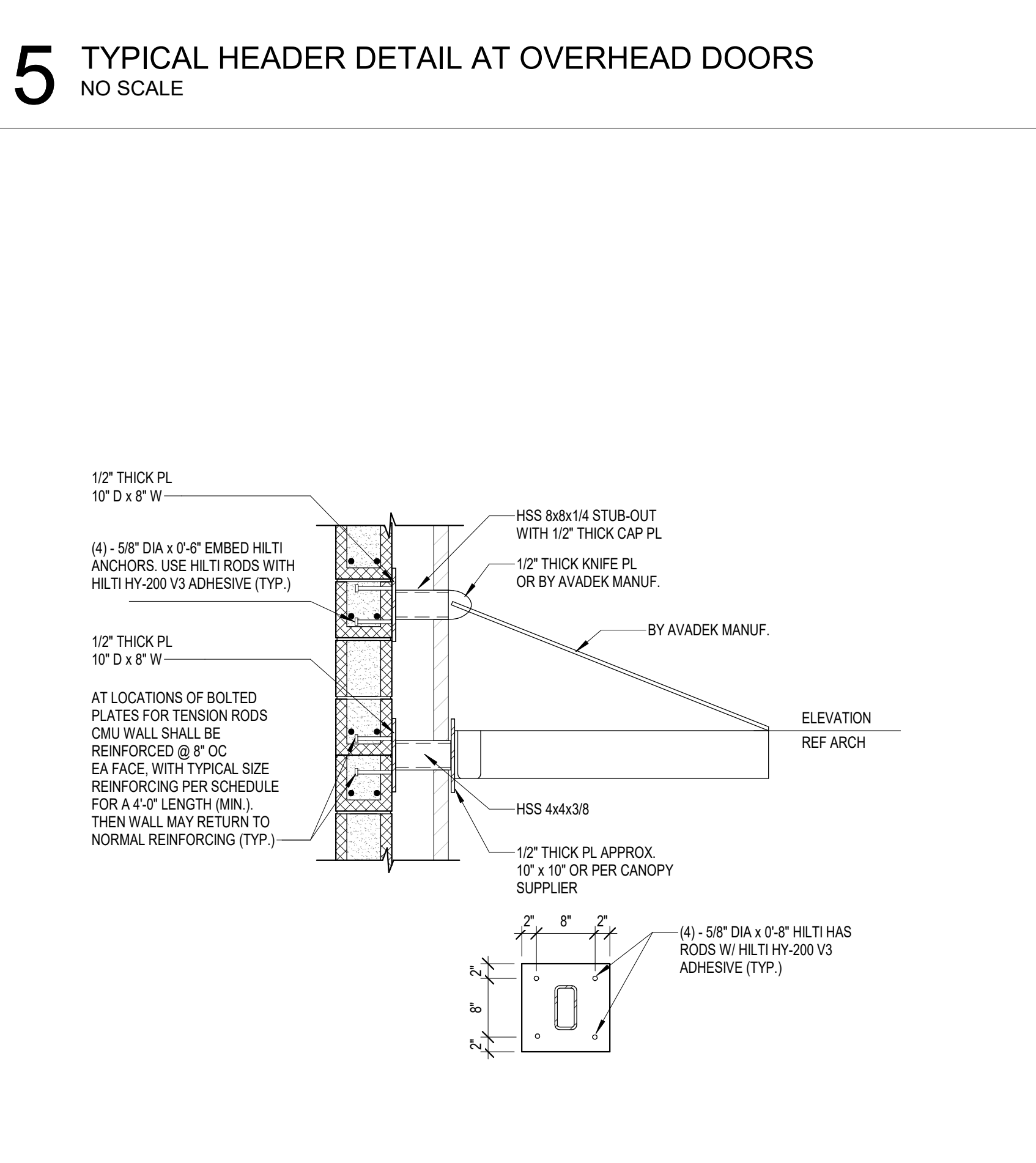
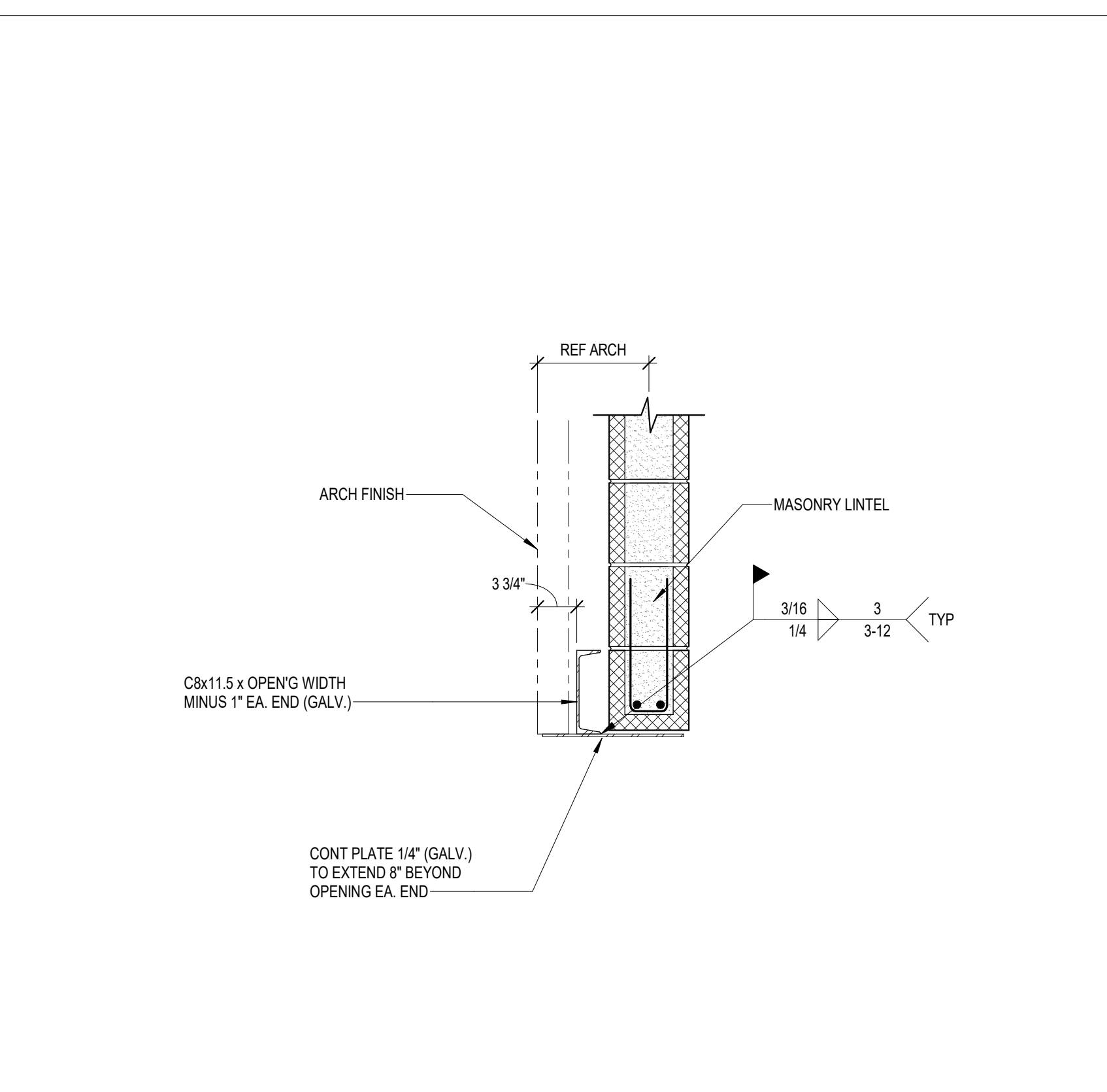
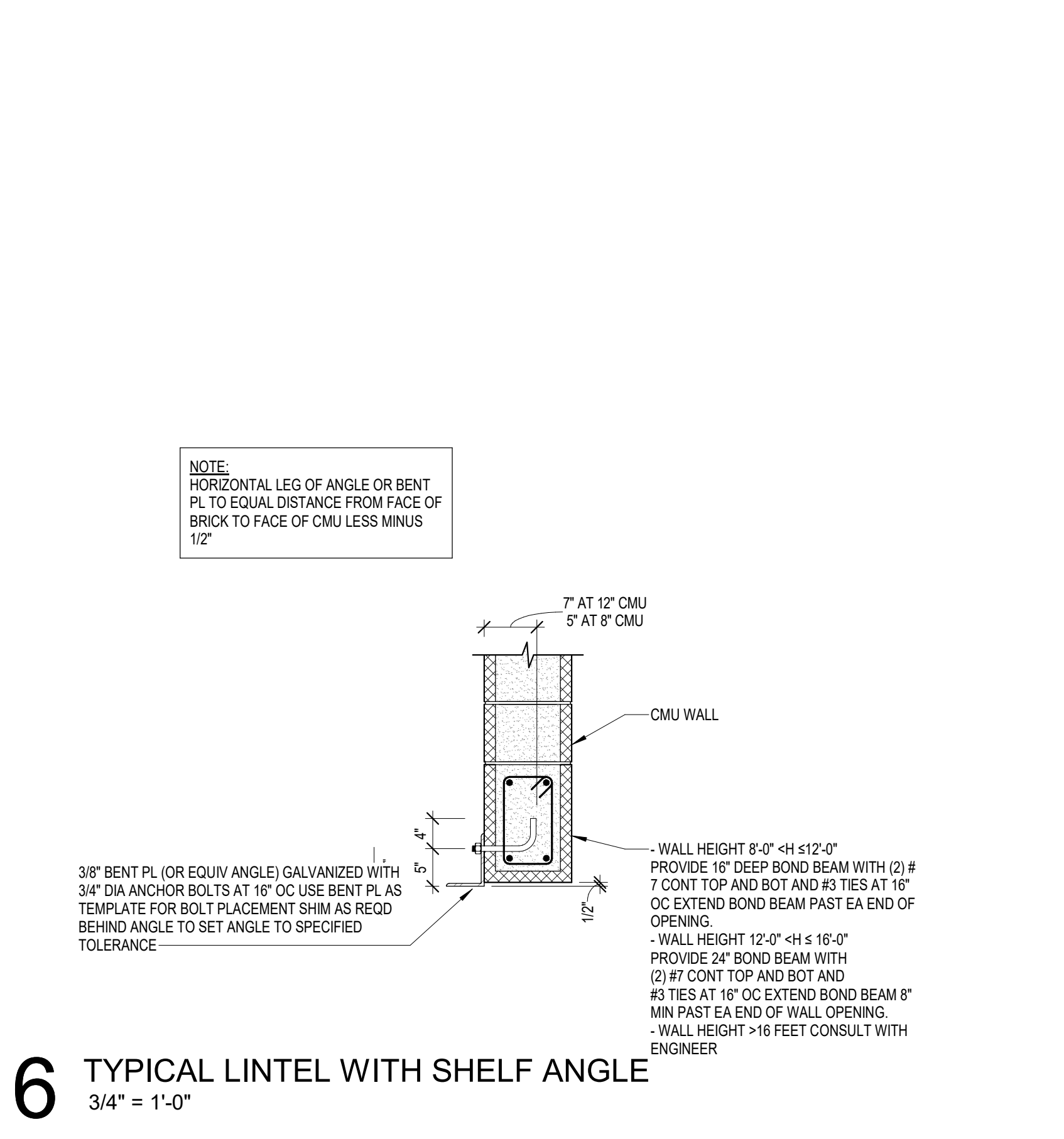


2 TYPICAL BRACING AT TOP OF STRUCTURAL CMU SHEAR WALLS AND NON STRUCTURAL EXTERIOR CMU WALLS
3/4" = 1'-0"

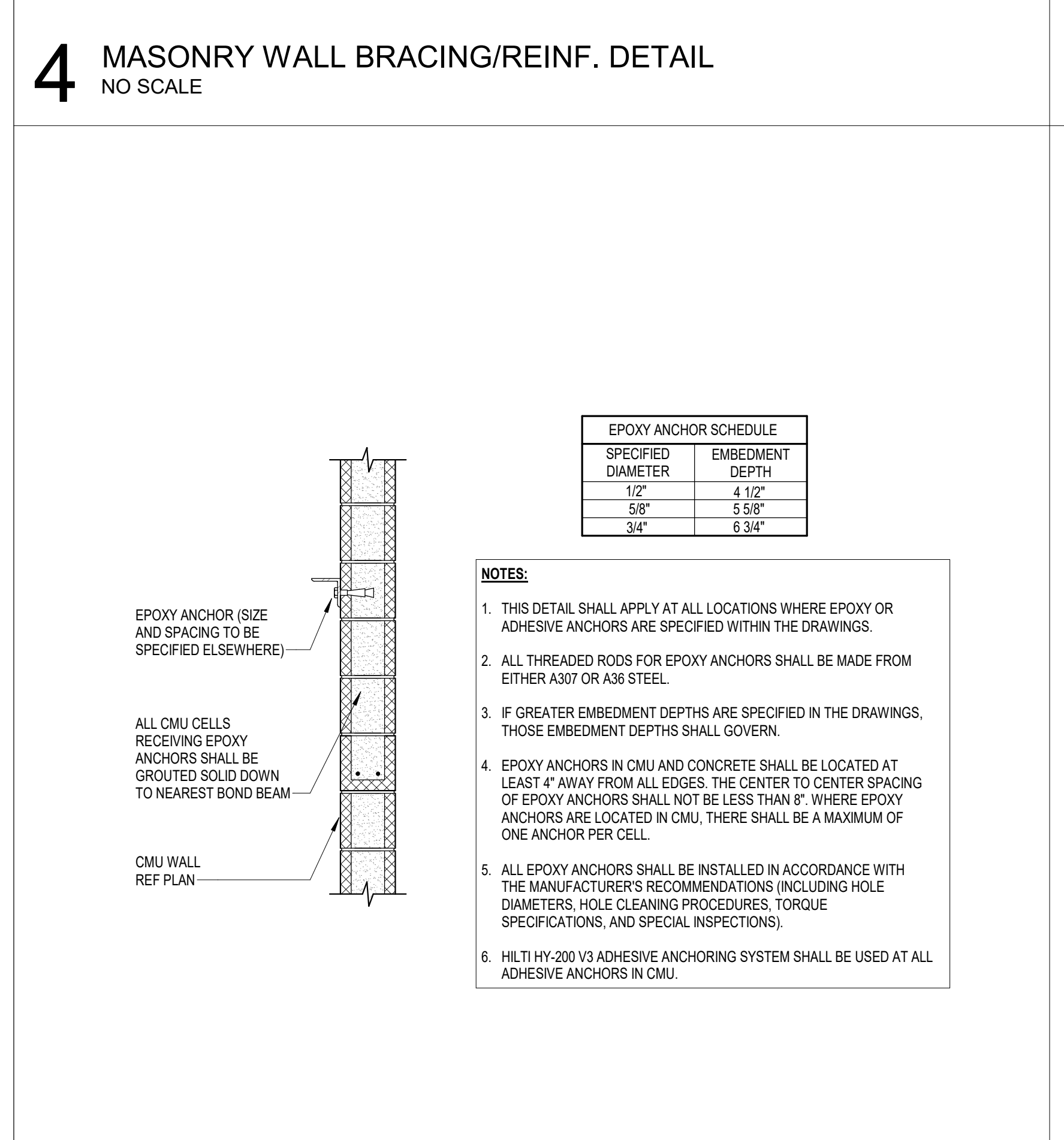
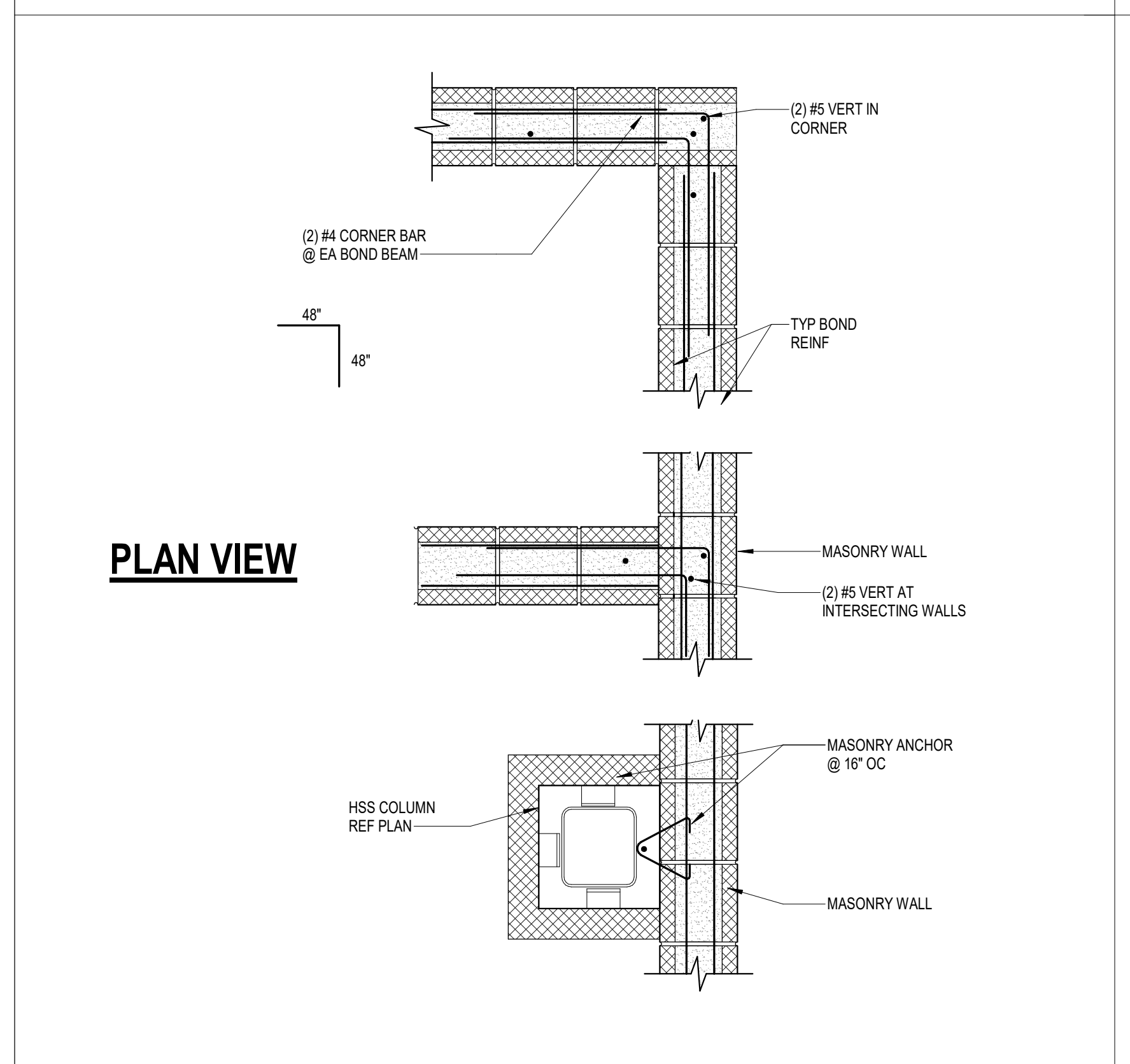
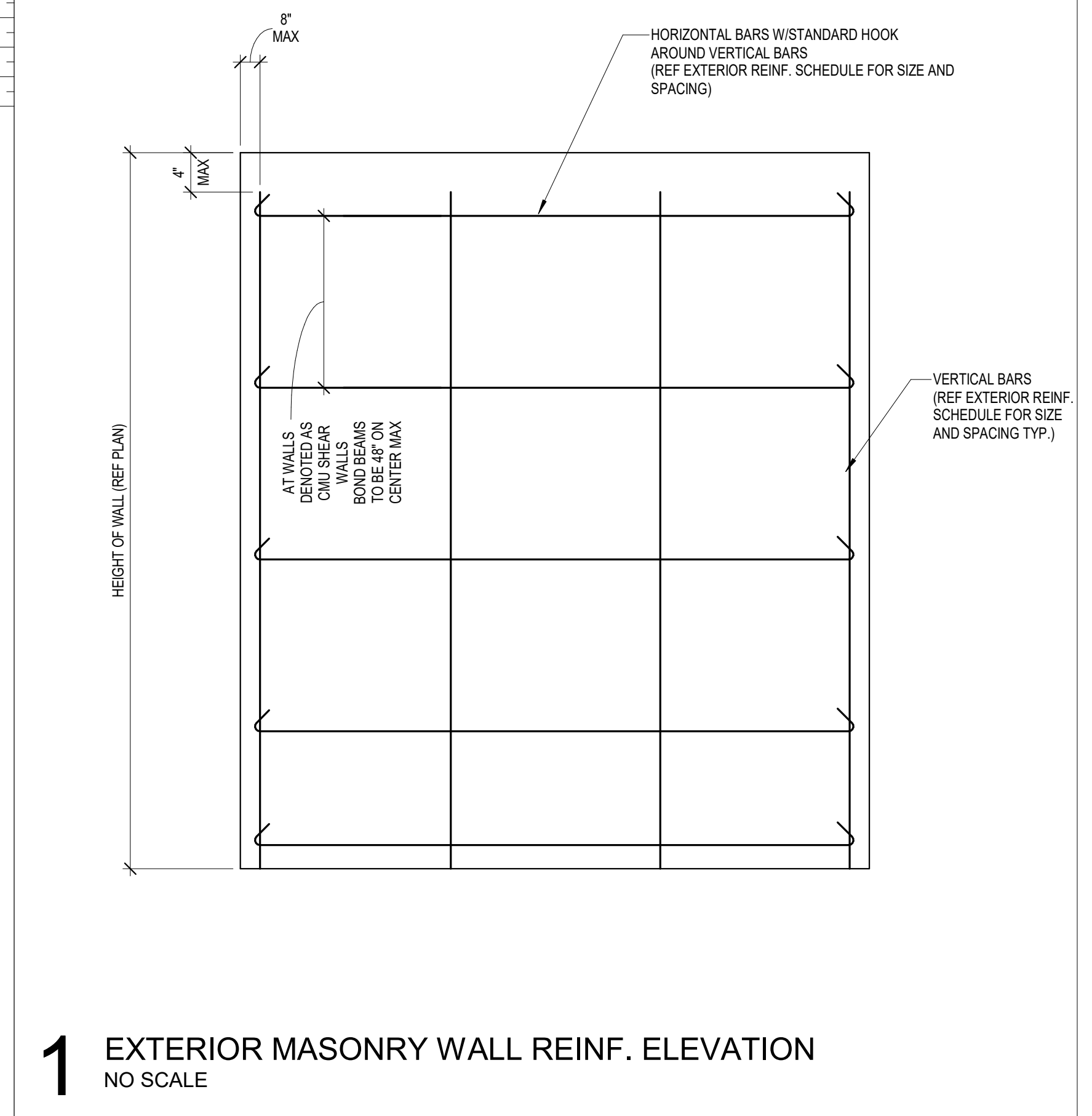
SW TAG	CMU (SIZE)	MEAN ROOF HEIGHT (SIMPLE SPAN)	VERT REIN (IN FULLY GROUDED CELLS)	HORIZ REIN (IN FULLY GROUDED CELLS)	JAMB / JOINT REIN (IN FULLY GROUDED CELLS)
EXTERIOR WALL	SW1	8'	16'-0"	#6 @ 24" OC EF	#5 @ 48" OC
		>16'-0" 28'-0"	#6 @ 18" OC EF	#5 @ 32" OC	(2) #6 PER CELL IN FIRST 3 CELLS
INTERIOR WALL	SW2	12'	16'-0"	#6 @ 40" OC EF	#5 @ 32" OC
		>16'-0" 28'-0"	#6 @ 18" OC EF	#5 @ 32" OC EF	(2) #6 PER CELL IN FIRST 3 CELLS
INTERIOR WALL	SW1	8'	16'-0"	#6 @ 32" OC	#5 @ 32" OC
		>16'-0" 28'-0"	#6 @ 18" OC EF	#5 @ 32" OC	(2) #6 PER CELL IN FIRST 3 CELLS

- NOTES:
- UNLESS NOTED OTHERWISE, REFER TO THE SCHEDULE ABOVE FOR VERTICAL WALL REINFORCING REQUIREMENTS AT ALL STRUCTURAL CMU WALLS.
 - THE STRUCTURAL MASONRY SHOWN ON THE PLANS IS PART OF THE LATERAL LOAD RESISTING SYSTEM OF THE BUILDING. THE STRUCTURAL DETAILS, INCLUDING CLIP ANGLES, DOWELS AND ADDITIONAL SECONDARY FRAMING MEMBERS, ETC., SHOWN ARE CRITICAL TO THE LATERAL PERFORMANCE OF THE BUILDING. THE TEMPORARY STEEL FRAME BRACING PROVIDED DURING CONSTRUCTION SHALL NOT BE REMOVED UNTIL ALL STRUCTURAL LATERAL BRACING SYSTEMS (INCLUDING STRUCTURAL MASONRY WALLS) HAVE BEEN INSTALLED AND CONNECTED TO THE STEEL FRAMING.
 - THE FIRST (3) CELLS AT CORNERS (IN EA DIRECTION), ENDS OF WALLS AND EACH SIDE OF THE CONTROL JOINTS SHALL BE REINFORCED WITH THE SAME SIZE AND NUMBER OF REINFORCING BARS AS SPECIFIED FOR THE OTHER REINFORCED CELLS IN THE WALL AND GROUDED FULL.
 - AT OPENINGS, PROVIDE EXTRA REINFORCED GROUDED FULL HEIGHT CONSECUTIVE CELLS EACH SIDE OF THE OPENING EQUAL TO ONE HALF THE TOTAL NUMBER OF THE CELLS INTERRUPTED BY THE OPENING. REINFORCE EACH CELL WITH THE SAME SIZE AND NUMBER OF BARS AS SPECIFIED FOR THE INTERRUPTED CELLS. PROVIDE A MINIMUM OF (2) REINFORCED GROUDED CELLS EACH SIDE OF OPENING. REFER TO TYPICAL CMU WALL OPENING DIAGRAM AND SCHEDULE FOR ADDITIONAL INFORMATION.
 - HOLD VERTICAL REINFORCING IN POSITION AT TOP AND BOTTOM AND AT 8'-0" OC MAXIMUM.
 - REFER TO GENERAL NOTES, SPECIFICATIONS AND SECTIONS/DETAILS FOR INFORMATION NOT SHOWN.
 - VERTICAL BARS MAY BE SPLICED IN 8'-0" (+ or -) LENGTHS. SPLICES LOCATED IN ADJACENT CELLS SHALL BE STAGGERED SUCH THAT NOT MORE THAN 50 PERCENT OF THE BARS ARE SPLICED AT THE SAME LOCATION.

9 TYPICAL STRUCTURAL MASONRY SHEAR WALL REIN. SCHEDULE
NO SCALE



10 AVADEK CANOPY CONNECTION DETAIL
3/4" = 1'-0"



7 TYPICAL EPOXY ANCHOR DETAIL
NO SCALE

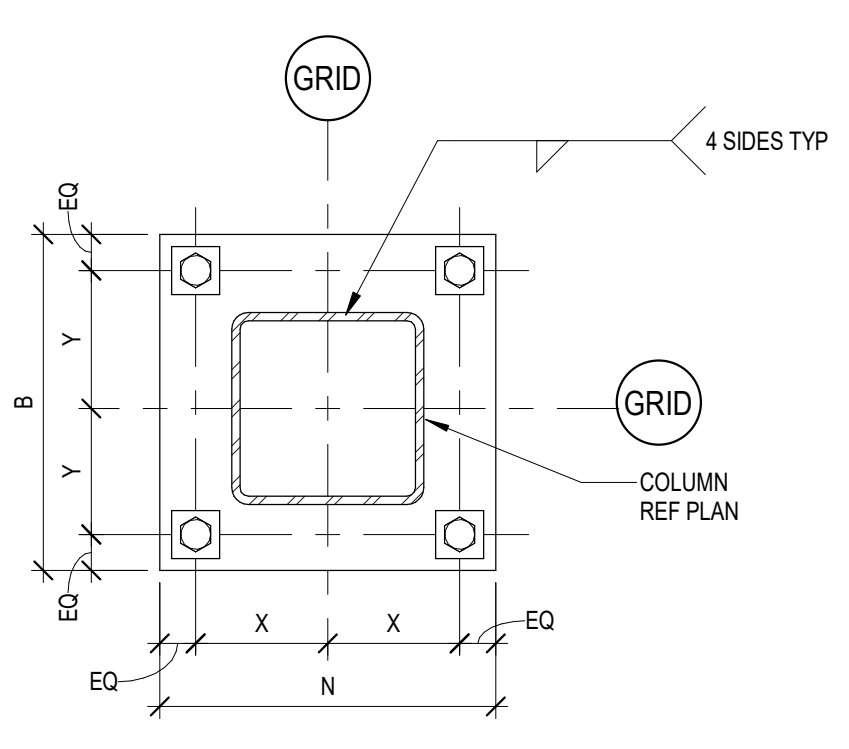
STRUCTURAL STEEL:

- MATERIAL:**
- STRUCTURAL STEEL IS TO CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS, AND IS TO BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC SPECIFICATIONS:
 - A. STEEL WIDE FLANGE AND WT SECTIONS.....ASTM A992 OR ASTM A572 - GRADE 50
 - B. MISCELLANEOUS STEEL SECTIONS (ANGLES, CHANNELS AND BARS).....ASTM A36
 - C. HOLLOW STEEL SECTIONS.....ASTM A500, GRADE B OR C, (46 KSI MIN)
 - D. STEEL PIPE SECTIONS.....ASTM A53, GRADE B
 - E. BOLTS AND NUTS (HEAVY-HEX).....ASTM A325
 - F. SHEAR STUD CONNECTORS.....ASTM A108
 - G. ANCHOR RODS.....ASTM F1554, GRADE 36, GRADE 55 OR GRADE 105
 - H. PLATES.....ASTM A36 OR A572 GRADE 50
 - I. WASHERS.....ASTM F436
 - ALL STEEL SHALL BE DOMESTICALLY (INCLUDING CANADA) MILLED AND FABRICATED. FOREIGN STEEL SHALL NOT BE UTILIZED WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER. THE APPROVAL PROCESS SHALL TAKE PLACE DURING BIDDING TIME. ANY REQUEST TO UTILIZE FOREIGN STEEL MADE AFTER BIDDING SHALL BE REJECTED.
 - STRUCTURAL STEEL, MISCELLANEOUS METAL, AND EMBEDS EXPOSED TO THE EXTERIOR ARE TO BE HOT DIP GALVANIZED AFTER FABRICATION, EXCEPT AS NOTED ON THE DRAWINGS. TUBE SHAPE MEMBERS EXPOSED TO THE EXTERIOR SHALL HAVE CAP PLATES SEAL WELDED, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
 - STRUCTURAL STEEL IS TO BE PREPARED AND PRIMED ACCORDING TO THE PROJECT SPECIFICATIONS. STRUCTURAL STEEL LOCATED IN CRAWL SPACES OR OTHERWISE INACCESSIBLE AREAS IS TO RECEIVE 2 COATS OF PRIMER.
 - CONTINUITY PLATES (FULL DEPTH COLUMN STIFFENERS ALIGNED WITH BEAM FLANGES, OR FULL DEPTH BEAM STIFFENERS ALIGNED WITH COLUMN FLANGES) SHALL MATCH THE STEEL GRADE OF THE BASE MEMBER.
 - IN ADDITION TO THE STEEL INDICATED ON THE DRAWINGS, THE CONTRACTOR SHALL INCLUDE THREE (3) PERCENT OF TOTAL TONNAGE OF FABRICATED STEEL (LABOR FOR ERECTION SHALL BE INCLUDED) DURING THE PROCESS OF WORK AS MAY BE DIRECTED BY THE ARCHITECT/ENGINEER OF RECORD. IF STEEL IS NOT USED DURING THE COURSE OF THE PROJECT, OWNER IS TO RECEIVE A CREDIT FOR THE PORTION NOT USED.
- CONNECTIONS:**
- ALL STRUCTURAL STEEL DETAILS AND CONNECTIONS SHALL CONFORM TO STANDARDS OF THE AISC. DOUBLE CONNECTIONS THROUGH COLUMN WEBS, BEAM TO BEAM CONNECTIONS AND BEAMS THAT FRAME OVER THE TOP OF COLUMNS REQUIRE A BEAM ERECTION SEAT OR A STAGGERED CONNECTION WITH AT LEAST ONE INSTALLED BOLT REMAINING IN PLACE TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED.
 - CONNECTIONS THAT ARE NOT DETAILED ON THE DRAWINGS SHALL BE SELECTED FROM THE TABLES IN PART 10 OF THE LATEST EDITION OF THE MANUAL OF STEEL CONSTRUCTION OF THE AISC. TABLE 10-1 MAY BE USED FOR ALL BOLTED DOUBLE ANGLE CONNECTIONS, TABLE 10-2 MAY BE USED FOR WELDED/BOLTED DOUBLE ANGLE CONNECTIONS, TABLE 10-3 MAY BE USED FOR ALL WELDED DOUBLE ANGLE CONNECTIONS. BEAM REACTIONS SHALL BE ONE-HALF THE TOTAL ALLOWABLE UNIFORM LOAD GIVEN IN TABLE 3-6 THROUGH 3-9 IN PART 3 OF THE MANUAL OF STEEL CONSTRUCTION OF AISC. CONNECTIONS FOR COMPOSITE BEAMS SHALL HAVE THE STANDARD AISC CAPACITY INCREASED BY 35 PERCENT.
 - PROVIDE ALL NECESSARY HOLES IN STRUCTURAL STEEL MEMBERS FOR ATTACHMENT OF ALL NON-STRUCTURAL ITEMS (IE: HOLES FOR WINDOW HEAD ANCHORS). SEE ARCHITECTURAL DRAWINGS FOR ANY REQUIREMENTS.
 - SPLICING OF STRUCTURAL STEEL MEMBERS MUST BE APPROVED BY THE STRUCTURAL ENGINEER, IF NOT ALREADY SHOWN ON THE DRAWINGS.
 - SHOP BOLTED CONNECTIONS ARE PERMISSIBLE IF SUFFICIENT BOLT CLEARANCE IS AVAILABLE FOR TIGHTENING OF HIGH STRENGTH BOLTS. CLEARANCES SHALL BE IN ACCORDANCE WITH TABLE 7-16 AND 7-17 OF THE LATEST EDITION OF THE MANUAL OF STEEL CONSTRUCTION OF THE AISC. ALL STEEL MEMBERS AND ASSEMBLIES SHALL BE SHOP FABRICATED TO THE GREATEST EXTENT POSSIBLE. TRUSSES SHALL BE FULLY SHOP ASSEMBLED. FIELD SPLICES FOR SHIPPING PURPOSES SHALL ONLY BE AS APPROVED BY THE ENGINEER OF RECORD. THE STEEL FABRICATOR AND THE STEEL ERECTOR SHALL COORDINATE THE SHOP FABRICATION, SHIPPING AND ERECTION OF ALL STRUCTURAL MEMBERS AND ASSEMBLIES.
 - ALL CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS MUST CONFORM TO ASTM A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE. MINIMUM SIZE SHALL BE 3/4 INCH DIAMETER. STRUCTURAL STEEL CONNECTIONS SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS SPECIFIED ON S-503. BOLTS SHALL BE DIRECT TENSION INDICATING BOLTS CONFORMING TO ASTM F1952 WITH HARDENED WASHERS UNDER THE NUT AND SACRIFICIAL SPLINES. HEX NUTS MUST CONFORM TO ASTM A563 AND WASHERS MUST CONFORM TO ASTM F436.
 - ALL MOMENT CONNECTIONS SHALL BE FULL WELDED CONNECTIONS DESIGNED TO DEVELOP THE FULL CROSS-SECTION OF THE MEMBER. STIFFENER PLATES, WHERE SHOWN, ARE MANDATORY AND MAY NOT BE OMITTED. MOMENT CONNECTIONS ARE INDICATED ON THE PLANS BY A TRIANGULAR BULB ON THE END OF THE BEAM, OR BY THE LETTERS "MC". CANTILEVER BEAMS MOMENT CONNECTED TO THE FRAME SHALL BE THE SAME SIZE AS THE BACK-UP SPAN IF NO SIZE IS GIVEN.
 - WHERE HORIZONTAL FORCES ARE INDICATED ON PLAN AS "H", "K", "L", THE LICENSED PROFESSIONAL ENGINEER WORKING FOR THE FABRICATOR SHALL SIZE THE CONNECTION TO TRANSFER THE HORIZONTAL FORCE IN ADDITION TO THE REQUIRED VERTICAL REACTION. SLOTTED HOLES ARE NOT PERMITTED AT CONNECTIONS WITH HORIZONTAL FORCES.
 - EMBED PLATES TO BE INSTALLED IN THE FOUNDATION AND/OR SLAB SHALL BE SUBMITTED FOR REVIEW WITH THE ANCHOR BOLTS.
 - BOLTS SHALL BE TIGHTENED BY THE AISC "SNUG TIGHT" METHOD UNLESS NOTED OTHERWISE.
 - CANTILEVER BEAMS MOMENT CONNECTED TO THE FRAME SHALL BE THE SAME SIZE AS THE BACK-UP SPAN IF NO SIZE IS GIVEN.
 - SHelf ANGLES SHOWN AS CONTINUOUS IN THE SECTIONS SHALL BE INSTALLED IN 20'-0" MAXIMUM LENGTHS, LEAVING A 1/4" GAP BETWEEN ENDS AND AT CORNERS. LOCATE GAPS TO MATCH MASONRY CONTROL JOINTS. AT BUILDING EXPANSION JOINT, LEAVE A GAP TO MATCH EXPANSION JOINT WIDTH.
 - CONNECT MISCELLANEOUS STEEL MEMBERS USING FILLET WELDS SUFFICIENT TO DEVELOP THE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS SHOWN OTHERWISE.
 - STEEL MEMBERS SHOWN TO BE CURVED SHALL BE ROLLED IN A MANNER THAT WILL NOT CAUSE DISTORTION OR BUCKLING. SHOULD ALTERATIONS TO THE MEMBER SIZE, SUCH AS A THICKER FLANGE OR WEB, BE REQUIRED TO ENSURE THIS OUTCOME, THE ADDITIONAL STEEL SHALL BE PROVIDED AT NO ADDITIONAL COST TO THE CONTRACT.

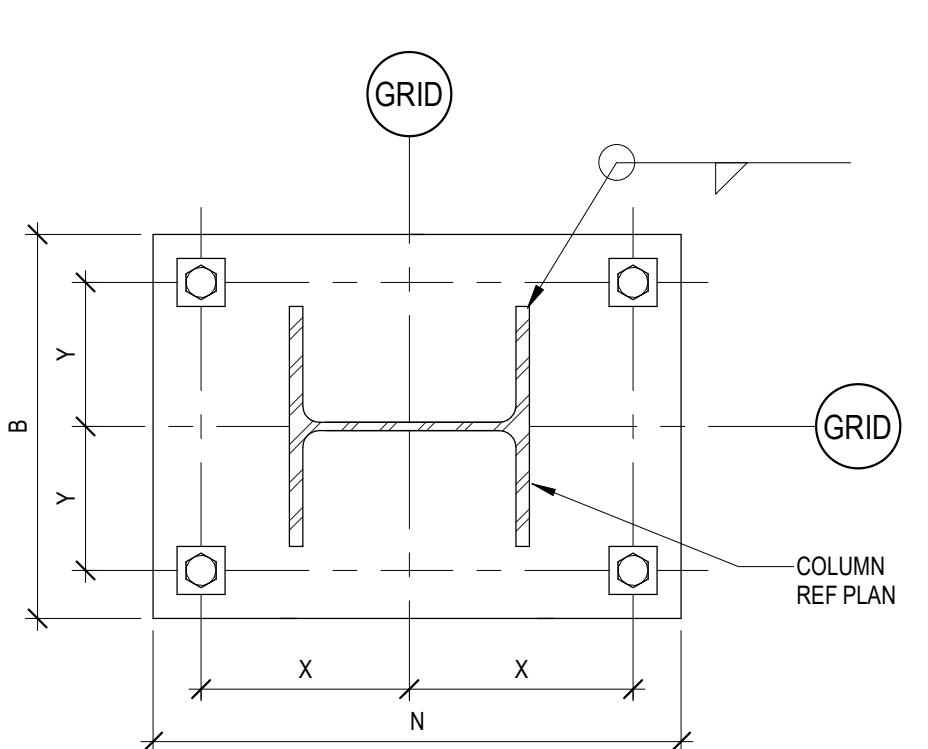
- WELDING:**
- ALL WELDING MUST CONFORM TO THE AMERICAN WELDING SOCIETY ANSIAWS D1.1 STANDARDS, AND SHALL CONFORM TO THE STANDARDS OF THE LATEST EDITION OF THE MANUAL OF STEEL CONSTRUCTION. ALL WELDERS MUST BE CERTIFIED IN ACCORDANCE WITH AWS D1.1. WELDING OF REINFORCING BARS SHALL COMPLY TO THE AMERICAN WELDING SOCIETY AWS D1.4. SHORT CIRCUIT TRANSFER FOR THE GAS METAL ARC WELDING PROCESS IS NOT PERMITTED.
 - ELECTRODES FOR ALL FIELD AND SHOP WELDING SHALL BE CLASS E70XX. ELECTRODES FOR MOMENT CONNECTIONS SHALL BE CLASS E7018 WITH A CHARPY TOUGHNESS OF AT LEAST 20 FT-LBS AT -20 DEGREES FAHRENHEIT.
 - ALL MISCELLANEOUS WELDS SHALL BE MINIMUM SIZE FILLET ALL AROUND AND MUST BE IN ACCORDANCE WITH AISC. WELDING OF CONTINUOUS MEMBERS SHALL BE A MINIMUM OF 3/16 INCH FILLET STITCH WELDS AT 12 INCHES ON EACH SIDE, UNLESS SHOWN OTHERWISE ON THE DRAWINGS. COLUMN BASE PLATES, STIFFENER PLATES AND CAP PLATES SHALL BE WELDED ALL AROUND.
 - HEADED STUDS SHALL BE WELDED TO EMBED PLATES BY A METHOD IN WHICH THE CONNECTION CAN DEVELOP THE FULL TENSION AND SHEAR CAPACITY OF THE STUD.

BASE PLATE NOTES:

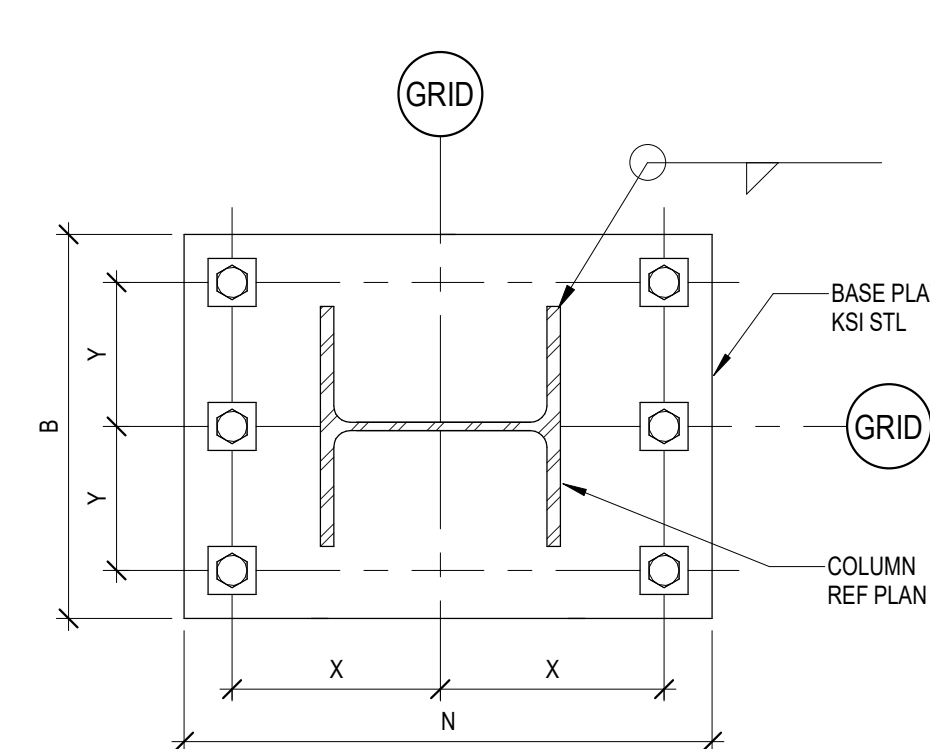
- ALL ANCHOR RODS SHALL BE F1554 GRADE 55 UNLESS NOTED OTHERWISE.
- SIZE WELDS PER AISC MINIMUM FILLET REQUIREMENTS, 5/16" MINIMUM.
- UNLESS NOTED OTHERWISE, ALL ANCHOR ROD NUTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.
- USE OVERSIZED HOLES FOR ANCHOR RODS ACCORDING TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION. PLATE WASHERS WITH STANDARD HOLES SHALL BE INSTALLED OVER OVERSIZED HOLES.
- CONTRACTOR SHALL PROVIDE TEMPLATE FOR ANCHOR ROD INSTALLATION.
- NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES:
ALL GROUT USED UNDER STEEL COLUMN BASE PLATES OR BEARING PLATES SHALL BE A NON-METALLIC, SHRINKAGE RESISTANT COMPOUND CONFORMING TO ASTM C1590 AND THE CORPS OF ENGINEERS SPECIFICATION CRD-C-421. THE GROUT SHALL HAVE A MINIMUM STRENGTH OF 6,000 PSI WHEN BEARING ON 3,000 PSI CONCRETE OR LESS, A STRENGTH OF 8,000 PSI WHEN BEARING ON CONCRETE BETWEEN 3,000 AND 4,000 PSI, AND UNLESS NOTED OTHERWISE ON THE DRAWINGS, A STRENGTH OF 8,000 PSI WHEN BEARING ON CONCRETE GREATER THAN 4,000 PSI. 100 PERCENT OF VOID UNDER ALL BASE PLATES IS TO BE GROUTED. ALL BASE PLATES WITH A DIMENSION GREATER THAN 24" SHALL HAVE TWO (2) DIAMETER GROUT HOLES. IF THE SPACE UNDER A COLUMN BASE PLATE IS LESS THAN 14", A PRESSURE INJECTION SYSTEM SHALL BE USED.



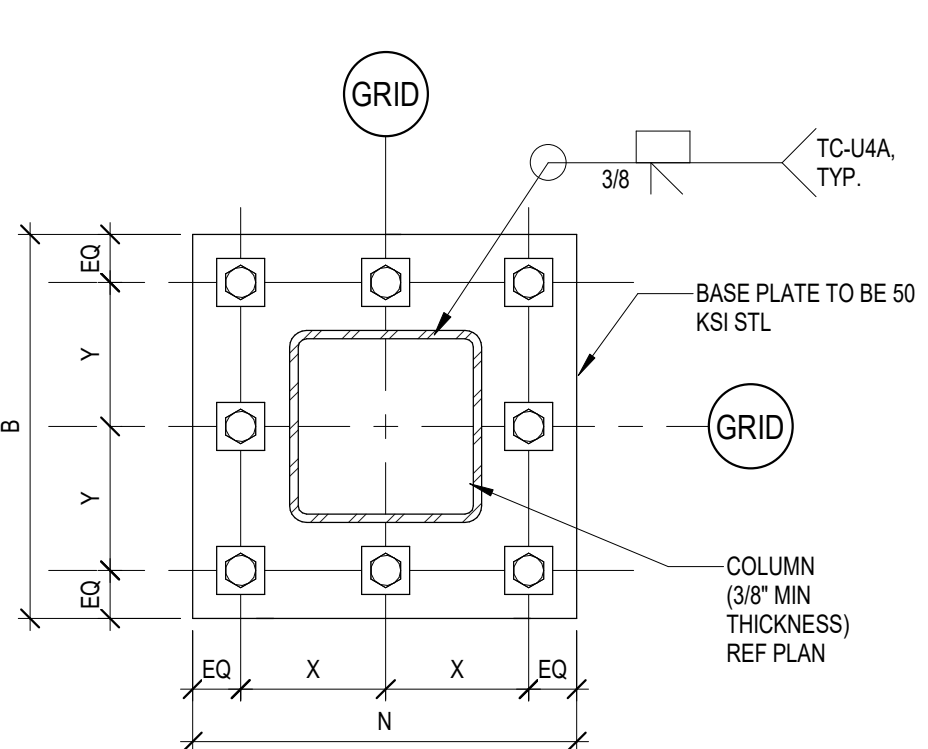
TYPE 1 HSS BASE PLATE
NTS



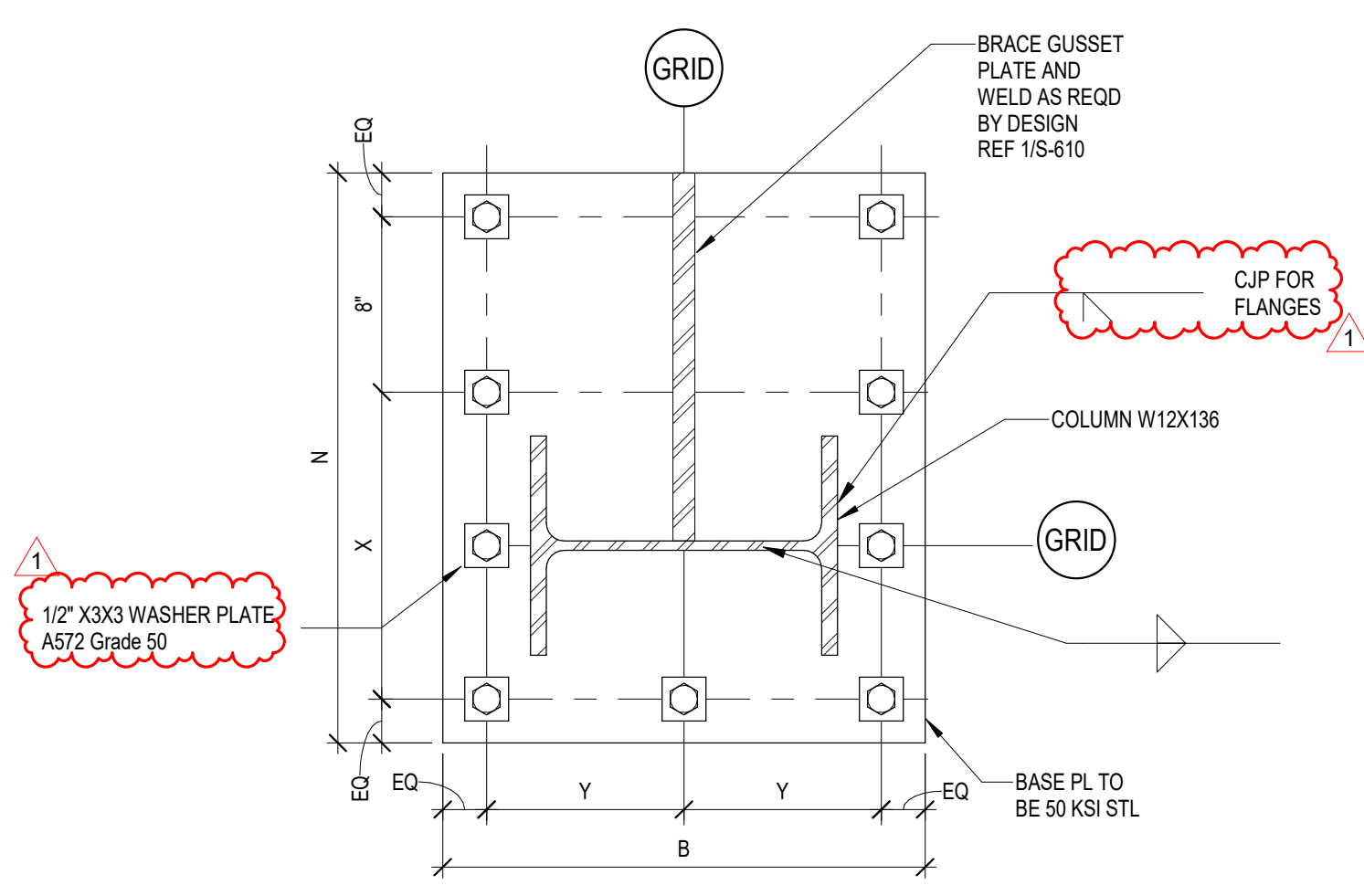
TYPE 2 WIDE FLANGE BASE PLATE
NTS



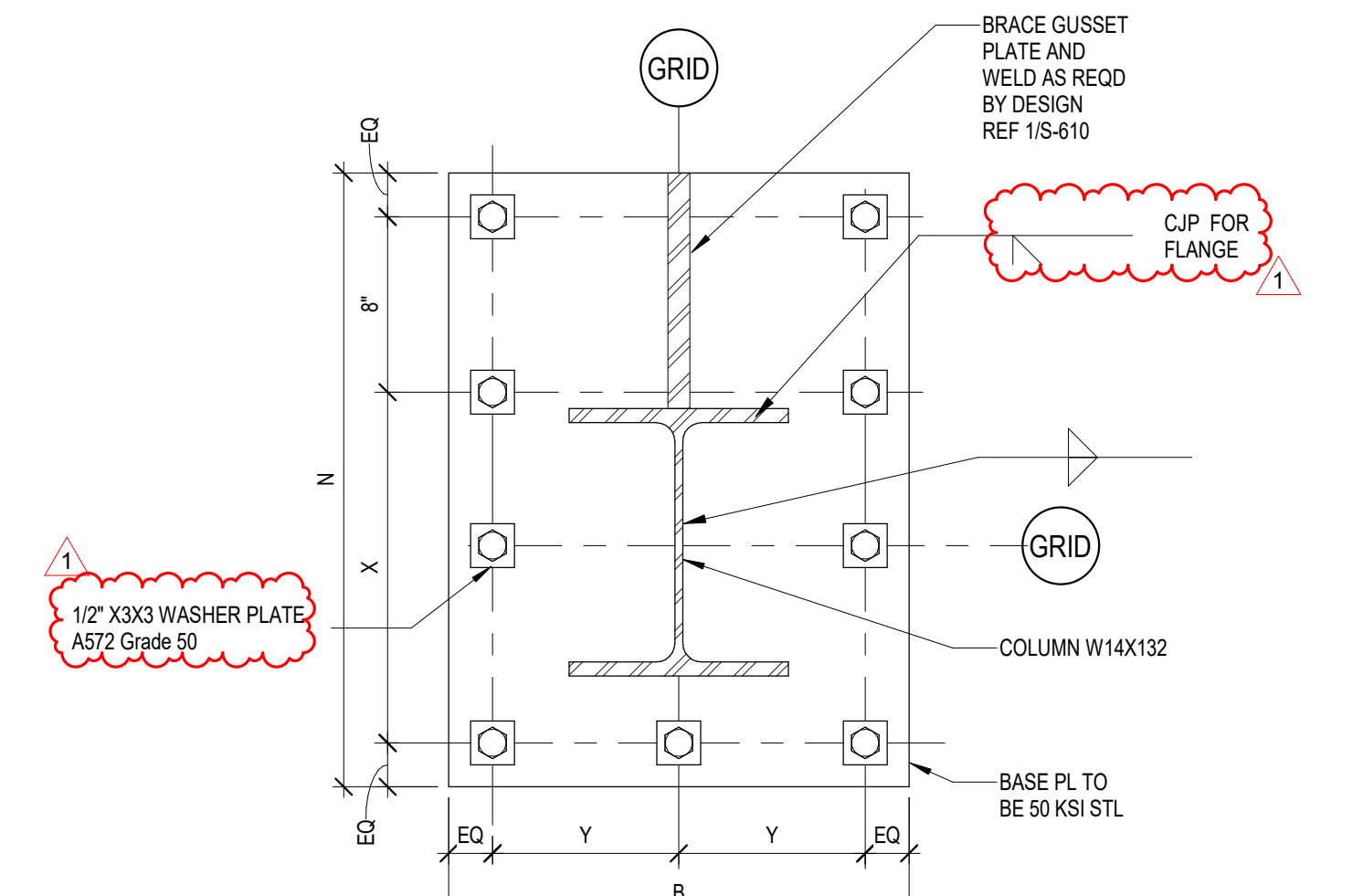
TYPE 3 WIDE FLANGE MOMENT BASE PLATE
NTS



TYPE 4 MOMENT BASE PLATE
NTS



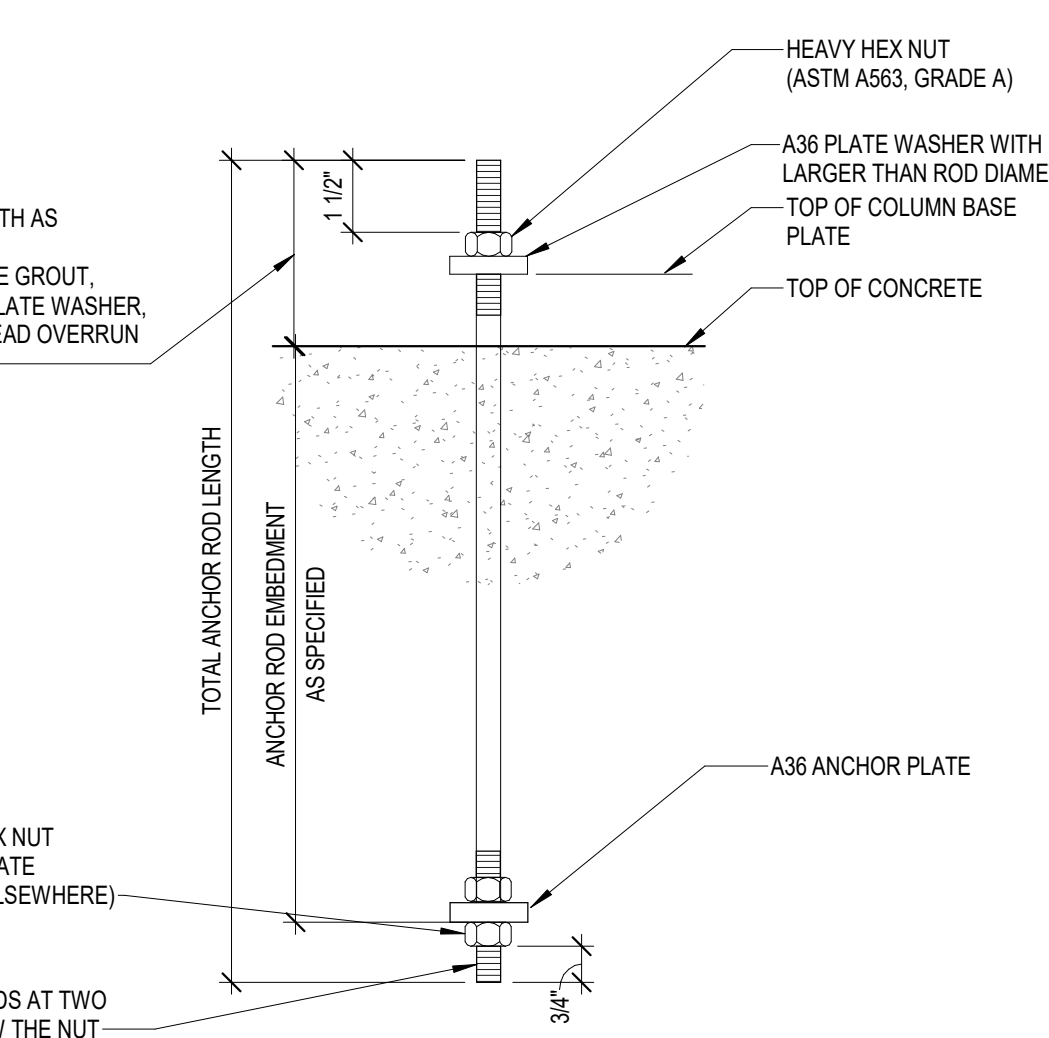
TYPE 5 BRACE BASE PLATE
NTS



TYPE 6 BRACE BASE PLATE
NTS

MARK	COLUMN SIZE	BASE PLATE TYPE	BASE PLATE DIMENSIONS			ANCHOR RODS			NOTES			
			N LENG (IN)	B WIDTH (IN)	T THICKNESS (IN)	NUMBER	DIA (IN)	EMBED LENGTH (IN)		ANCHOR ROD GRADE		
-	W14x132	6	32	24	2.3/4	19	9/12	9	1	18	55	BASE PLATE TO BE 50 KSI STEEL
-	W12x136	5	26	22	2.3/4	14	9	9	1	18	55	BASE PLATE TO BE 50 KSI STEEL

- NOTES:**
- USE OVERSIZED HOLES FOR ANCHOR RODS ACCORDING TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
 - PLATE WASHERS WITH STANDARD HOLES SHALL BE INSTALLED OVER OVERSIZED HOLES.
 - CONTRACTOR SHALL PROVIDE TEMPLATE FOR ANCHOR ROD INSTALLATION.
 - MINIMUM GROUT THICKNESS PER GROUT THICKNESS SCHEDULE.
 - SIZE WELDS PER AISC MINIMUM FILLET REQUIREMENTS, 5/16" MINIMUM.



ANCHOR ROD DIAMETER	BASE PLATE HOLE DIAMETER (IN)	CIRCULAR OR SQUARE PLATE WASHER SIZE (IN)	PLATE WASHER THICKNESS (IN)
3/4"	1 5/16	2	1/4
7/8"	1 9/16	2 1/2	5/16
1"	1 13/16	3	3/8
1 1/4"	2 1/16	3	1/2
1 1/2"	2 5/16	3 1/2	1/2
1 3/4"	2 3/4	4	5/8
2"	3 1/4	5	3/4
2 1/2"	3 3/4	5 1/2	7/8

2 ANCHOR ROD DETAIL NO SCALE

GROUT THICKNESS SCHEDULE	
ANCHOR BOLT SIZE (DIA.)	MINIMUM GROUT THICKNESS
LESS THAN 1 1/4"	2"
GREATER THAN 1 1/4"	3"

- NOTES:**
- REFER TO SPECIFICATIONS AND GENERAL NOTES FOR GROUT REQUIREMENTS.
 - GROUT MAY BE BEVELED OR FORMED BUT SHALL EXTEND A MIN OF 1 1/2" BEYOND FACE OF BASE PLATE.

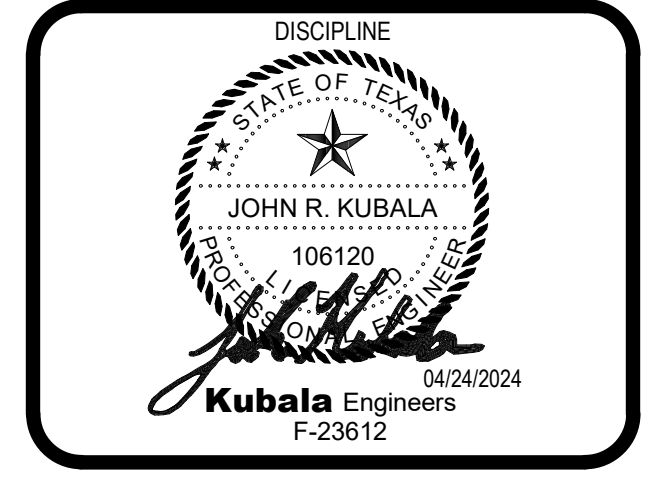
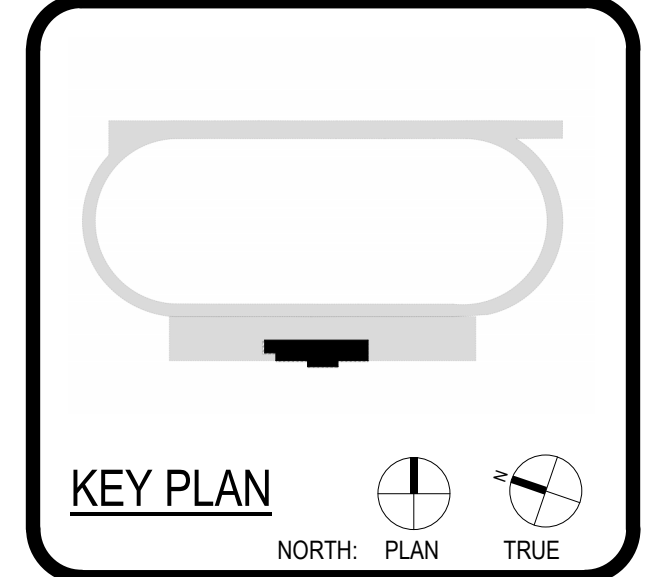
3 GROUT SCHEDULE NO SCALE

1 COLUMN BASE PLATE AND ANCHOR ROD SCHEDULE NO SCALE



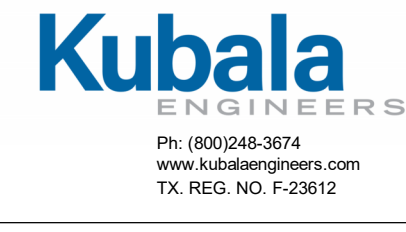
ARCHITECT: HOUSTON PBK Architects, Inc.
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0688 P
TX Firm: BR 1608
TX Firm: BR 1608
LEAD ENGINEER: 1-11-2024-2025

GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX
1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL



CLIENT: GALVESTON ISD		
DATE: 01/31/2024	PROJECT NUMBER: 230063	
DRAWING HISTORY		
No.	Description	Date
1	ASI #1	04/24/2024

ISSUE FOR PROPOSAL
BUILDING NUMBER
GENERAL STEEL NOTES AND TYPICAL DETAILS



GENERAL INFO.:

- FOR ANY STEEL BEAM OR COLUMN THAT DOES NOT MEET THE MINIMUM SIZE REQUIRED DUE TO THE U.L. DESIGN NUMBER (SELECTED BY THE ARCHITECT), THE THICKNESS OF THE SPRAYED FIRE PROTECTION MATERIAL MUST BE INCREASED AS REQUIRED BY THE FORMULA SHOWN IN THE U.L. FIRE RESISTANCE DIRECTORY (LATEST EDITION).
- AT BRICK SUPPORT ANGLES DURING CONSTRUCTION, THE BRICK SHALL BE INSTALLED WITHOUT SHORING THE SUPPORT ANGLE. SHORING THE BRICK DURING CONSTRUCTION CAN CAUSE HORIZONTAL BED JOINT CRACKING WHEN THE SHORES ARE REMOVED.
- HEADED ANCHORS/STUDS SHALL BE MANUFACTURED FROM COLD DRAWN MATERIALS PER ASTM A108. ANCHORS/STUDS SHALL BE OF GRADE 50 WITH SOLID FLUX FILLED HEADS. ANCHORS/STUDS SHALL BE AUTOMATICALLY END WELDED WITH IN ACCORDANCE WITH AWS D1.1. ANCHORS/STUDS FOR EMBEDDED PLATES AND OTHER ANCHORS SHALL BE SHOP WELDED. STUDS FOR COMPOSITE BEAMS MUST BE FIELD WELDED.
- AT BUILDINGS WHERE SPLICED COLUMNS ARE REQUIRED, THE STEEL COLUMNS MUST BE SPLICED AT A MINIMUM OF 4'-0" ABOVE THE FINISH FLOOR. COLUMNS SHALL BE SPLICED AT EVERY OTHER LEVEL. AT WIDE FLANGE COLUMNS, PROVIDE HOLES IN THE WEBS FOR 3/4" DIAMETER SAFETY CABLES. AT TUBE OR PIPE COLUMNS, PROVIDE PLATES WITH HOLES WELDED TO THE COLUMNS FOR SAFETY CABLE CONNECTIONS. A 1/2" X 3/4" DECK SUPPORT ANGLE SHALL BE PROVIDED ON ALL SIDES OF THE COLUMN.
- ALL STRUCTURAL STEEL OUTSIDE OF THE BUILDING ENVELOPE SHALL BE HOT DIPPED GALVANIZED WITH A MINIMUM ZINC COATING CLASS OF G90, MEETING THE REQUIREMENTS OF ASTM 123, AND SHALL BE APPLIED AFTER FABRICATION. ALL FIELD WELDS SHALL BE GROUND SMOOTH AND TOUCHED UP WITH A ZINC RICH PAINT.
- THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS SHALL COMPLY TO OSHA 29 CFR 1926 SUBPART R, SAFETY STANDARDS FOR STEEL ERECTION.
- THE DRAWINGS AND SPECIFICATIONS MAY NOT INDICATE OR DESCRIBE ALL OF THE WORK REQUIRED FOR THE PERFORMANCE AND COMPLETION OF THE PROJECT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF ALL MISCELLANEOUS METAL ITEMS INDICATED, DESCRIBED, OR IMPLIED ON THE STRUCTURAL AND/OR ARCHITECTURAL DRAWINGS. MISCELLANEOUS STEEL ITEMS WITHIN AN ASSEMBLY AND NOT ATTACHED TO THE STRUCTURE ARE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS. WHETHER OR NOT THEY ARE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS, SUCH ASSEMBLIES INCLUDE, BUT ARE NOT LIMITED TO: EXTERIOR AND INTERIOR WALL ASSEMBLIES, GELING ASSEMBLIES, PARTITION ASSEMBLIES, SHELF AND CABINET ASSEMBLIES AND ALL OTHER SIMILAR ASSEMBLIES. ANY MISCELLANEOUS METAL ITEMS INDICATED ON THE ARCHITECTURAL DRAWINGS AND NOT SHOWN ON STRUCTURAL DRAWINGS SHALL BE A MINIMUM OF 1/4"x1/2", C7x8.8, 3/8" PLATE OR HSS4x4x3/8" UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.
- THE GENERAL CONTRACTOR AND THE ELEVATOR SUPPLIER SHALL REVIEW THE STRUCTURAL DRAWINGS FOR FLOOR TO FLOOR AND FLOOR TO ROOF HEIGHTS FOR THE REQUIRED DISTANCE BETWEEN BRACE POINTS. FOR THEIR RAIL DESIGN, PROVIDE A MINIMUM HOST BEAM SIZE. IF NO OTHER SIZE IS PROVIDED ON THE PLANS, BRACING REQUIREMENTS FOR THE TOP OF THE RAIL OR INTERMEDIATE RAIL BRACE POINTS, SHALL BE COORDINATED WITH THE STRUCTURAL DRAWINGS. IF BRACING IS REQUIRED BUT NOT SHOWN, PROVIDE A MINIMUM OF 1/2"x1/2" VERTICAL POST OR HANGER ATTACHED TO THE STRUCTURE WITH 1/8"x1/8" BEAMS OR C7x8.8 CHANNELS SPANNING BETWEEN THE FRAMING MEMBERS. ALL MISCELLANEOUS STEEL MEMBERS REQUIRED TO ATTACH RAILS AT THE RAIL BRACE POINTS SHALL BE SUPPLIED BY THE ELEVATOR SUPPLIER. ELEVATOR MACHINE BEAM OR SHIVE BEAM LOADS SHALL BE COORDINATED WITH THE STRUCTURAL DRAWINGS. LOADS AND LOAD LOCATIONS VARYING FROM THE LOADS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE VERIFIED WITH THE STRUCTURAL ENGINEER OF RECORD. THE FRAMING AND INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS IS BASED ON:
 - ELEVATOR MODEL NO. PER SPEC.
 - MANUFACTURED BY PER SPEC.

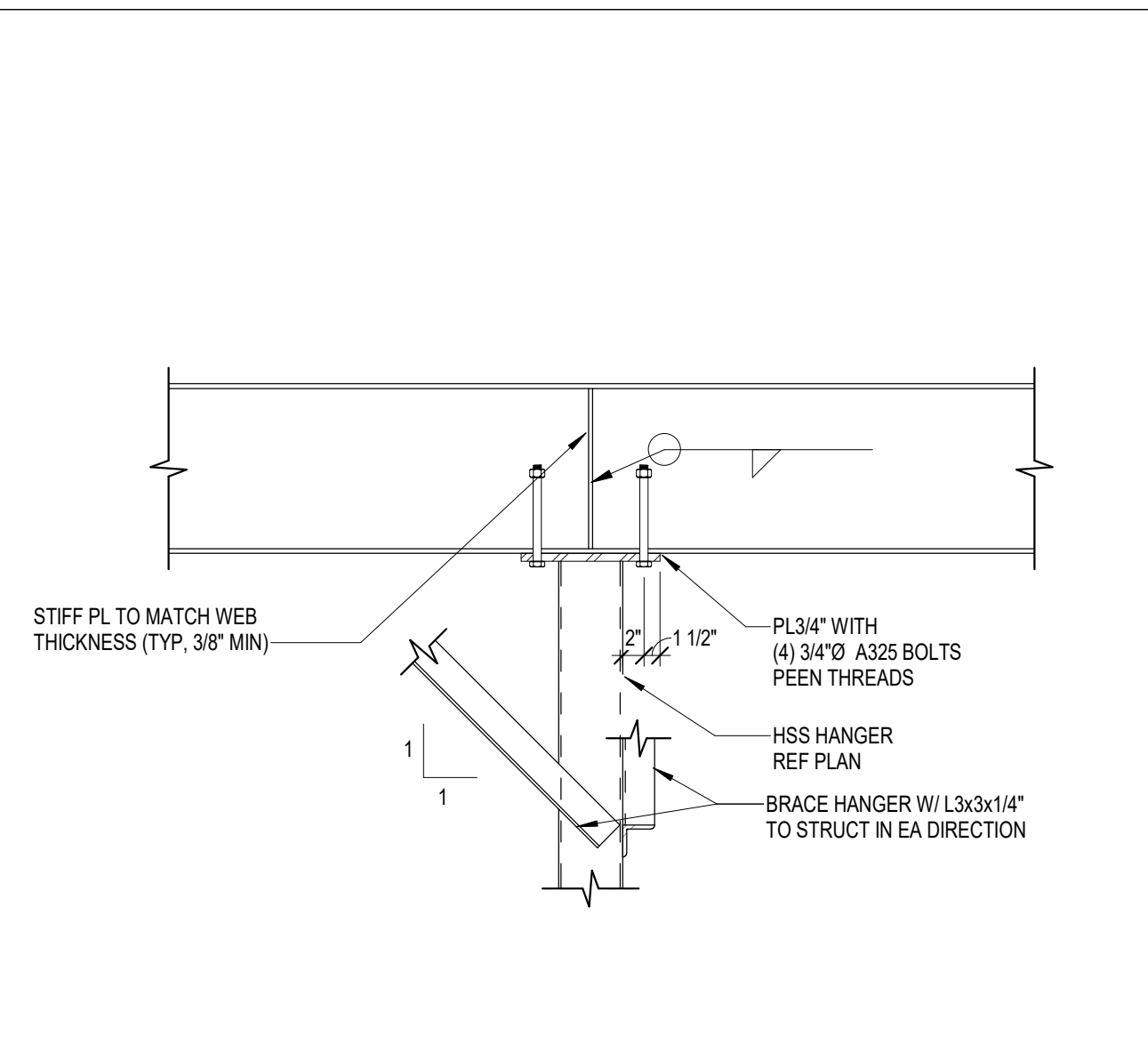
IF THE ELEVATOR MANUFACTURER AND/OR MODEL NUMBER CHANGES, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL REVISIONS TO THE FOUNDATIONS AND THE FLOOR AND ROOF FRAMING AS REQUIRED DUE TO THE CHANGE IN THE ELEVATOR TYPE.

IMPORTANT NOTE TO PROPOSERS:

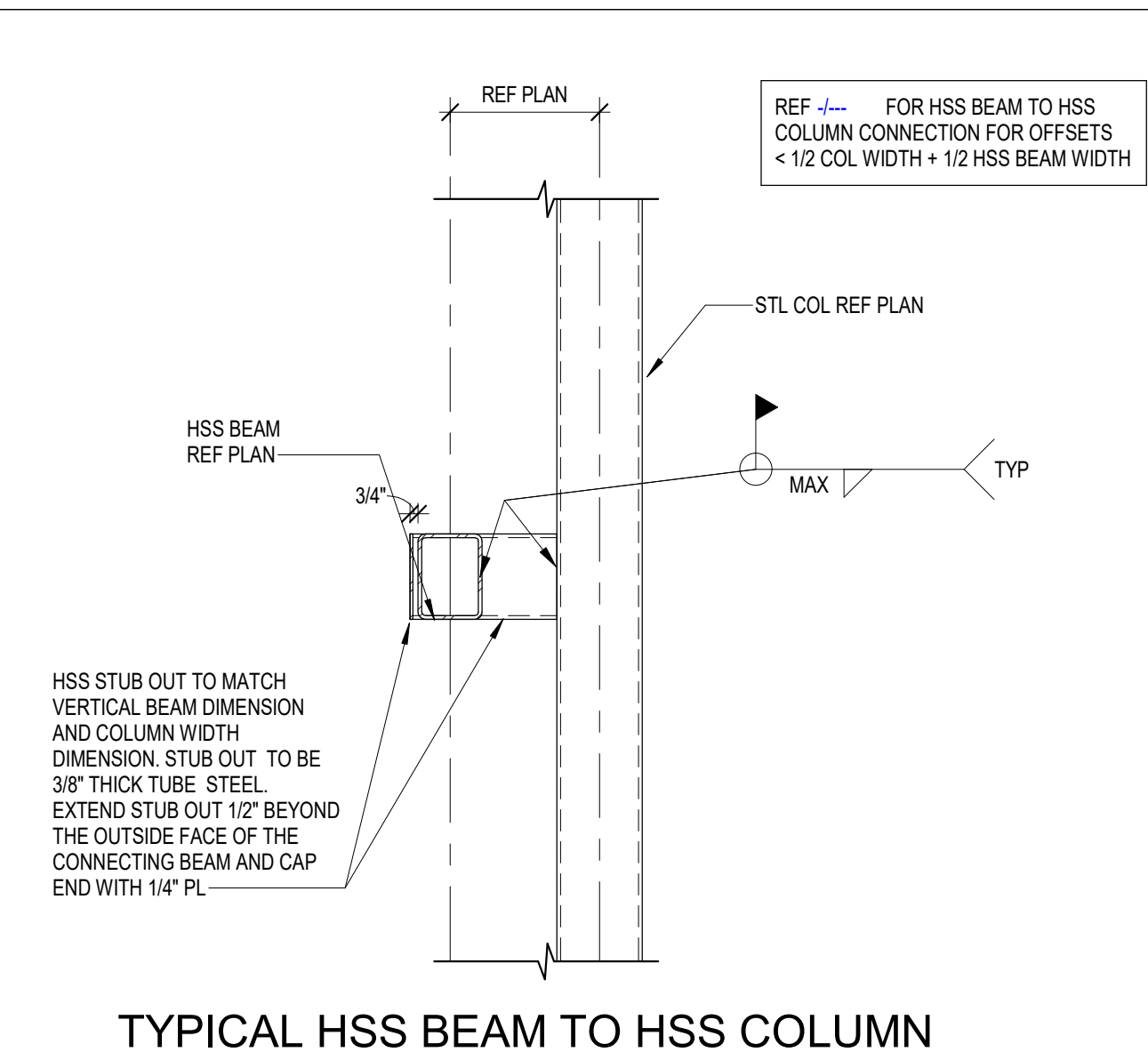
- THESE DRAWINGS AND SPECIFICATIONS DO NOT NECESSARILY INDICATE ALL OF THE WORK REQUIRED FOR THE COMPLETION OF THE PROJECT. THESE DRAWINGS DO NOT NECESSARILY INDICATE ALL SECONDARY FRAMING WHICH MAY BE REQUIRED BASED ON THE ARCHITECTURAL DRAWINGS.
- WHERE MISCELLANEOUS METAL ITEMS ARE IMPLIED OR INDICATED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF THESE ITEMS. THIS INCLUDES ANY MISCELLANEOUS METAL ITEMS INDICATED ON THE ARCHITECTURAL DRAWINGS AND NOT SHOWN ON THE STRUCTURAL DRAWINGS AND ANY ITEMS SHOWN ON THE STRUCTURAL DRAWINGS AND NOT NOTED. PROPOSERS SHALL ASSUME FOR PROPOSAL PURPOSES ONLY THE SIZE OF THESE ITEMS ARE AS FOLLOWS:
 - ANGLES: L6x6x3/8
 - CHANNELS: C12x20.7
 - PLATES: 1/2" THICK
 - ANCHOR RODS: 1" DIAMETER X 18" LONG
 - EPOXY ANCHORS: 1/2" DIAMETER AT 48" OC MAX SPACING
 - WIDE FLANGE BEAMS: W24x55
 - WIDE FLANGE COLUMNS: W12x79
 - HSS COLUMNS AND BEAMS: HSS8x8x3/8

THE CONTRACTOR SHALL SUBMIT AN RFI AND THE ENGINEER SHALL APPROVE THE SIZE OF THE MEMBER BEFORE CONSTRUCTION OR FABRICATION.

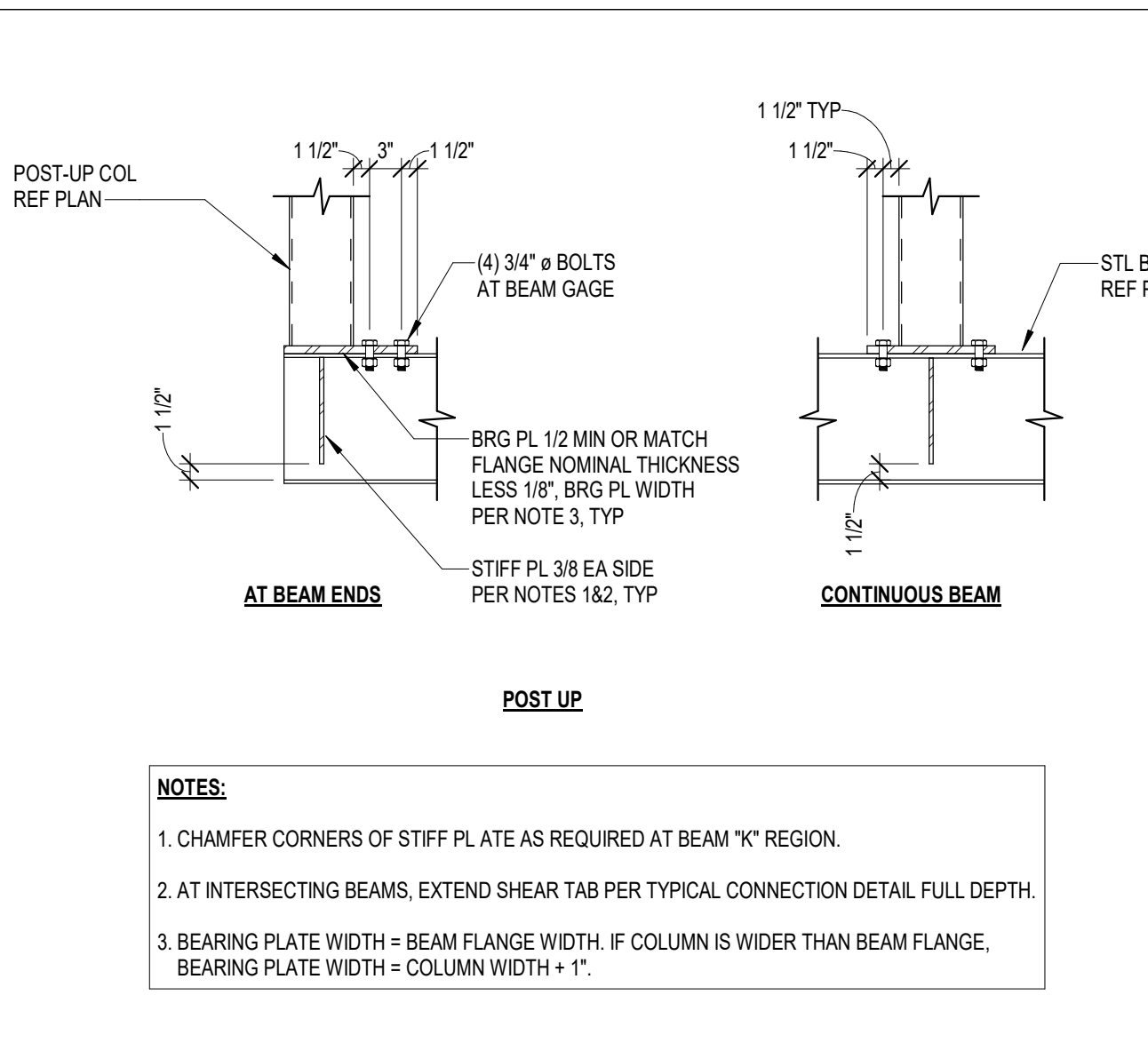
- PROVIDE SUPPORT FOR EDGES OF ROOF AND FLOOR DECK WHETHER SHOWN ON THE DRAWINGS OR NOT. PROVIDE CONTINUOUS SCREED ANGLE OR BENT PLATE FOR SLAB EDGES.



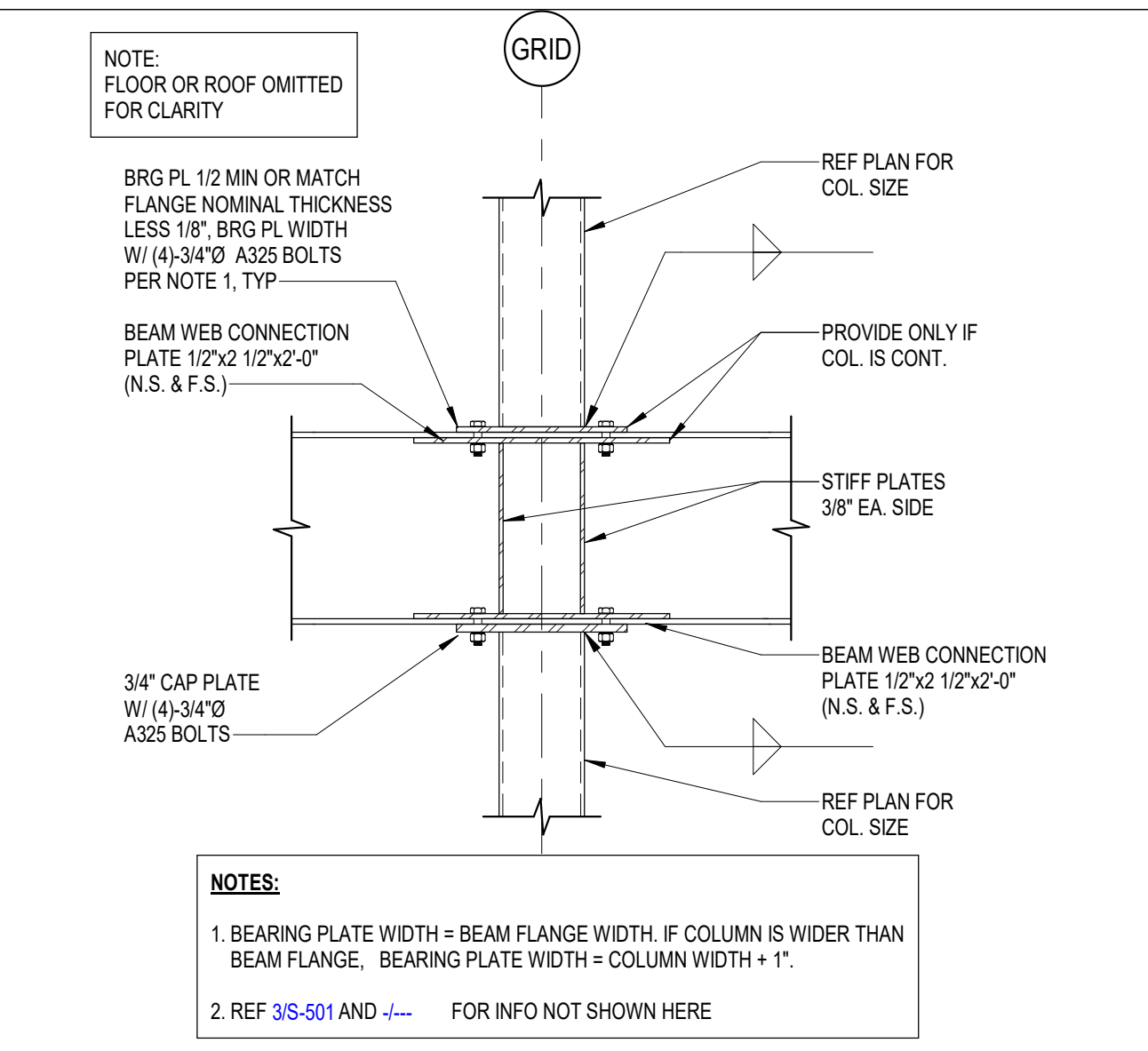
1 TYPICAL HANGER DETAIL
NO SCALE



2 TYPICAL HSS BEAM TO HSS COLUMN CONNECTION FOR OFFSETS > 1/2 COL WIDTH + 1/2 HSS BEAM WIDTH
3/4" = 1'-0"



3 TYPICAL POST-UP DETAIL
NO SCALE



4 TYPICAL CANTILEVERED BEAM OVER COLUMN CONNECTION
3/4" = 1'-0"

IF THE ELEVATOR MANUFACTURER AND/OR MODEL NUMBER CHANGES, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL REVISIONS TO THE FOUNDATIONS AND THE FLOOR AND ROOF FRAMING AS REQUIRED DUE TO THE CHANGE IN THE ELEVATOR TYPE.

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THE CONTRACTOR SHALL SUBMIT AN RFI AND THE ENGINEER SHALL APPROVE THE SIZE OF THE MEMBER BEFORE CONSTRUCTION OR FABRICATION.

- PROVIDE SUPPORT FOR EDGES OF ROOF AND FLOOR DECK WHETHER SHOWN ON THE DRAWINGS OR NOT. PROVIDE CONTINUOUS SCREED ANGLE OR BENT PLATE FOR SLAB EDGES.

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THE CONTRACTOR SHALL SUBMIT AN RFI AND THE ENGINEER SHALL APPROVE THE SIZE OF THE MEMBER BEFORE CONSTRUCTION OR FABRICATION.

- PROVIDE SUPPORT FOR EDGES OF ROOF AND FLOOR DECK WHETHER SHOWN ON THE DRAWINGS OR NOT. PROVIDE CONTINUOUS SCREED ANGLE OR BENT PLATE FOR SLAB EDGES.

IF THE ELEVATOR MANUFACTURER AND/OR MODEL NUMBER CHANGES, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL REVISIONS TO THE FOUNDATIONS AND THE FLOOR AND ROOF FRAMING AS REQUIRED DUE TO THE CHANGE IN THE ELEVATOR TYPE.

IMPORTANT NOTE TO PROPOSERS:

- THESE DRAWINGS AND SPECIFICATIONS DO NOT NECESSARILY INDICATE ALL OF THE WORK REQUIRED FOR THE COMPLETION OF THE PROJECT. THESE DRAWINGS DO NOT NECESSARILY INDICATE ALL SECONDARY FRAMING WHICH MAY BE REQUIRED BASED ON THE ARCHITECTURAL DRAWINGS.
- WHERE MISCELLANEOUS METAL ITEMS ARE IMPLIED OR INDICATED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATION AND INSTALLATION OF THESE ITEMS. THIS INCLUDES ANY MISCELLANEOUS METAL ITEMS INDICATED ON THE ARCHITECTURAL DRAWINGS AND NOT SHOWN ON THE STRUCTURAL DRAWINGS AND ANY ITEMS SHOWN ON THE STRUCTURAL DRAWINGS AND NOT NOTED. PROPOSERS SHALL ASSUME FOR PROPOSAL PURPOSES ONLY THE SIZE OF THESE ITEMS ARE AS FOLLOWS:
 - ANGLES: L6x6x3/8
 - CHANNELS: C12x20.7
 - PLATES: 1/2" THICK
 - ANCHOR RODS: 1" DIAMETER X 18" LONG
 - EPOXY ANCHORS: 1/2" DIAMETER AT 48" OC MAX SPACING
 - WIDE FLANGE BEAMS: W24x55
 - WIDE FLANGE COLUMNS: W12x79
 - HSS COLUMNS AND BEAMS: HSS8x8x3/8

THE CONTRACTOR SHALL SUBMIT AN RFI AND THE ENGINEER SHALL APPROVE THE SIZE OF THE MEMBER BEFORE CONSTRUCTION OR FABRICATION.

- PROVIDE SUPPORT FOR EDGES OF ROOF AND FLOOR DECK WHETHER SHOWN ON THE DRAWINGS OR NOT. PROVIDE CONTINUOUS SCREED ANGLE OR BENT PLATE FOR SLAB EDGES.

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 - ANGLES: L6x6x3/8
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 - PLATES: 1/2" THICK
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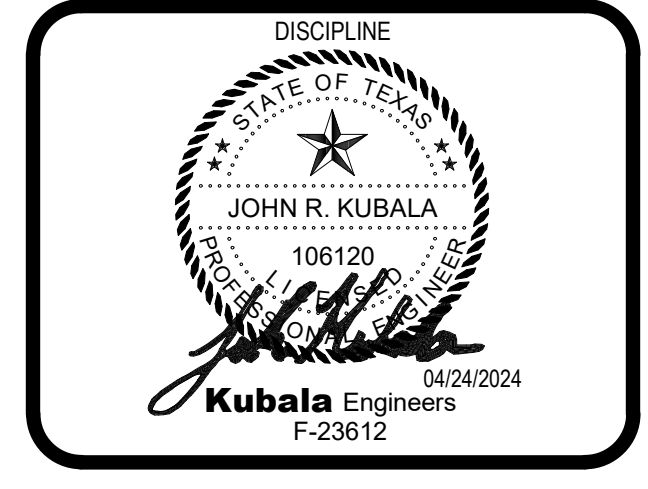
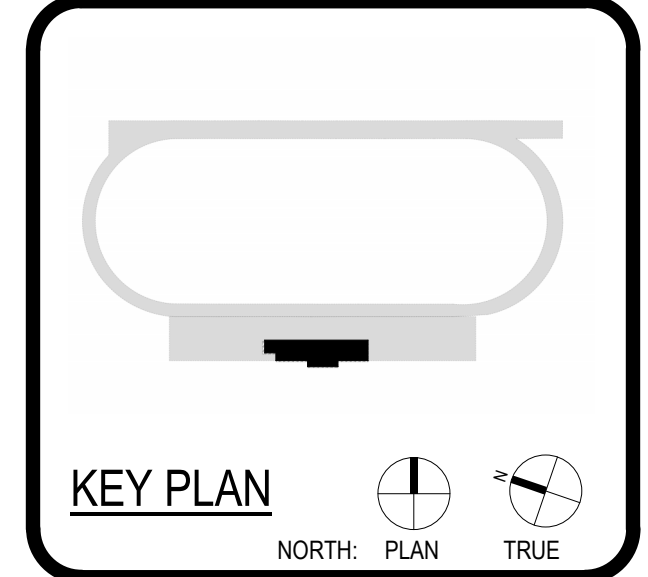
ARCHITECT: PKB Architects, Inc.
HOUSTON, TX 77046
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0688 P
713-961-4571 F
TX Firm: BR 1608

ENGINEERS: KUBALA ENGINEERS
1113 S. GULF SHORES BLVD.
HOUSTON, TX 77058
LEAD ENGINEER: JOHN R. KUBALA
1-713-965-0688

GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX

1429 27TH ST.
GALVESTON, TX, 77550

ISSUE FOR PROPOSAL



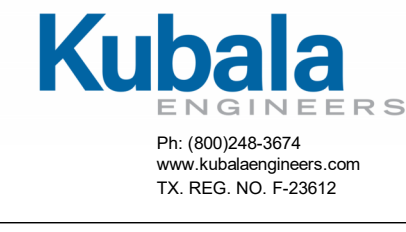
CLIENT: GALVESTON ISD		
DATE: 01/31/2024	PROJECT NUMBER: 230063	
DRAWING HISTORY		
No.	Description	Date
1	AS#1	04/24/2024

ISSUE FOR PROPOSAL

BUILDING NUMBER

GENERAL STEEL NOTES AND TYP DETAILS

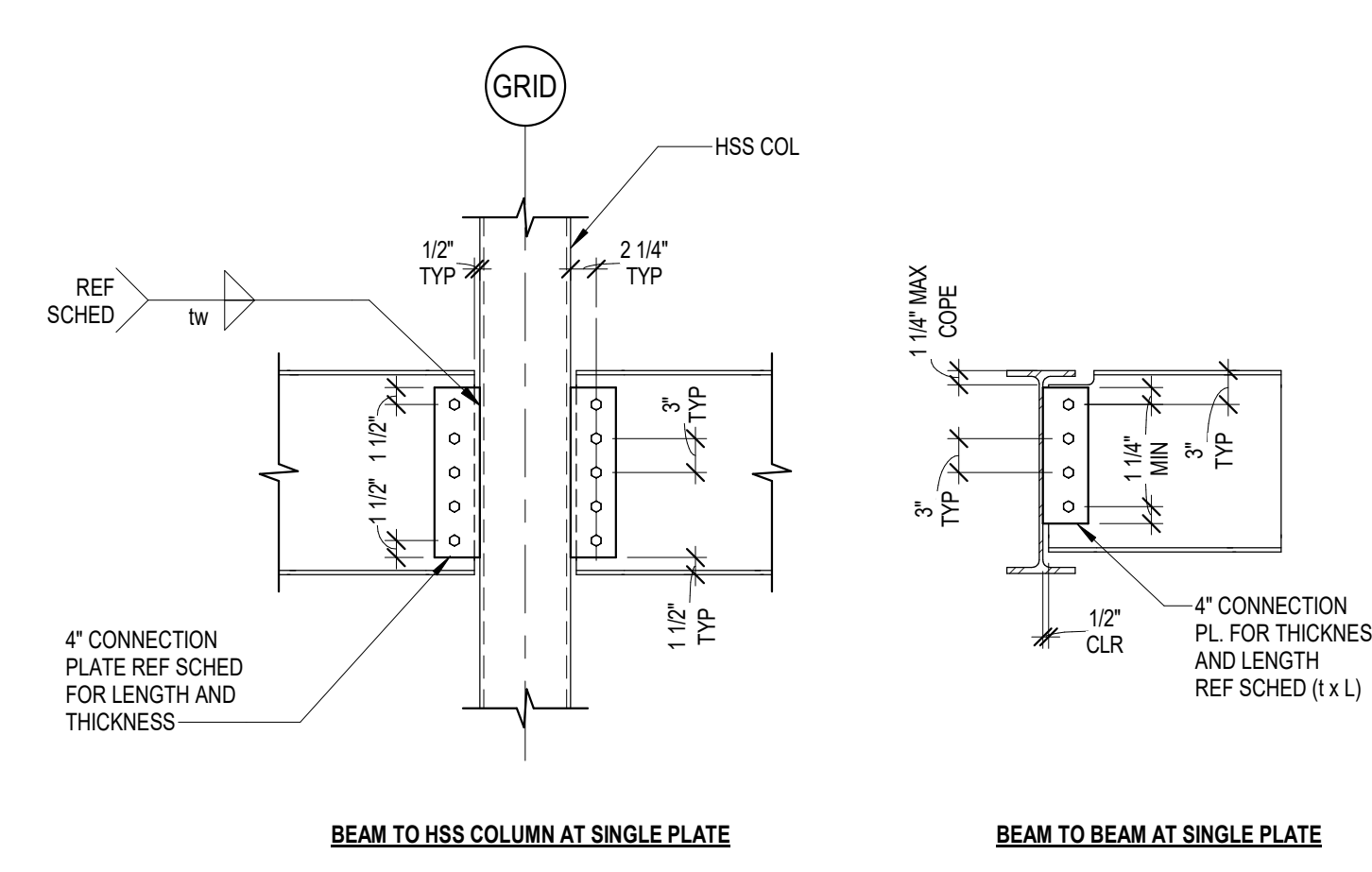
S-501



SINGLE PLATE CONNECTION SCHEDULE

BEAM SIZE	SHEAR PLATE (x L)	WELD SIZE (M)	A325-N BOLT DIAMETER	NO. OF BOLTS PER VERT. ROW	MAX CAPACITY (KIPS) (ASD)
W8	3/8" x 5 1/2	3/16"	3/4"	2	9
W10	3/8" x 5 1/2	1/4"	7/8"	2	14
W12	3/8" x 5 1/2	1/4"	7/8"	3	23
W14	3/8" x 5 1/2	1/4"	7/8"	3	28
W16	3/8" x 11 1/2	1/4"	7/8"	4	40
W18	1/2" x 11 1/2	1/4"	7/8"	4	48
W21	1/2" x 14 1/2	1/4"	1"	5	64
W24	1/2" x 17 1/2	1/4"	1"	6	83
W27	5/8" x 20 1/2	1/4"	1"	7	113
W30	5/8" x 20 1/2	1/4"	1"	7	119
W33	5/8" x 23 1/2	5/16"	1"	8	142
W36	5/8" x 26 1/2	5/16"	1"	9	160

- NOTES:**
- ALLOWABLE CAPACITIES ARE IN KIPS (SERVICE LEVEL - UNFACTORED).
 - CAPACITIES BASED ON GRADE 50 BEAMS, A325-N BOLTS, AND A36 STEEL.
 - HOLES SHALL BE STANDARD (STD) OR SHORT-SLOPPED (SSLT).
 - ALL INFORMATION SHOWN IN THE SCHEDULE ABOVE IS TO BE USED AS A MINIMUM REQUIREMENT FOR ALL CONNECTIONS.
 - ALTERNATE CONNECTION DETAILS ARE ACCEPTABLE PROVIDED SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS ARE SUBMITTED, AND ALL CONNECTION DESIGNS ARE PERFORMED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.



2 TYPICAL SHEAR TAB BOLTED CONNECTION SCHEDULE
NO SCALE

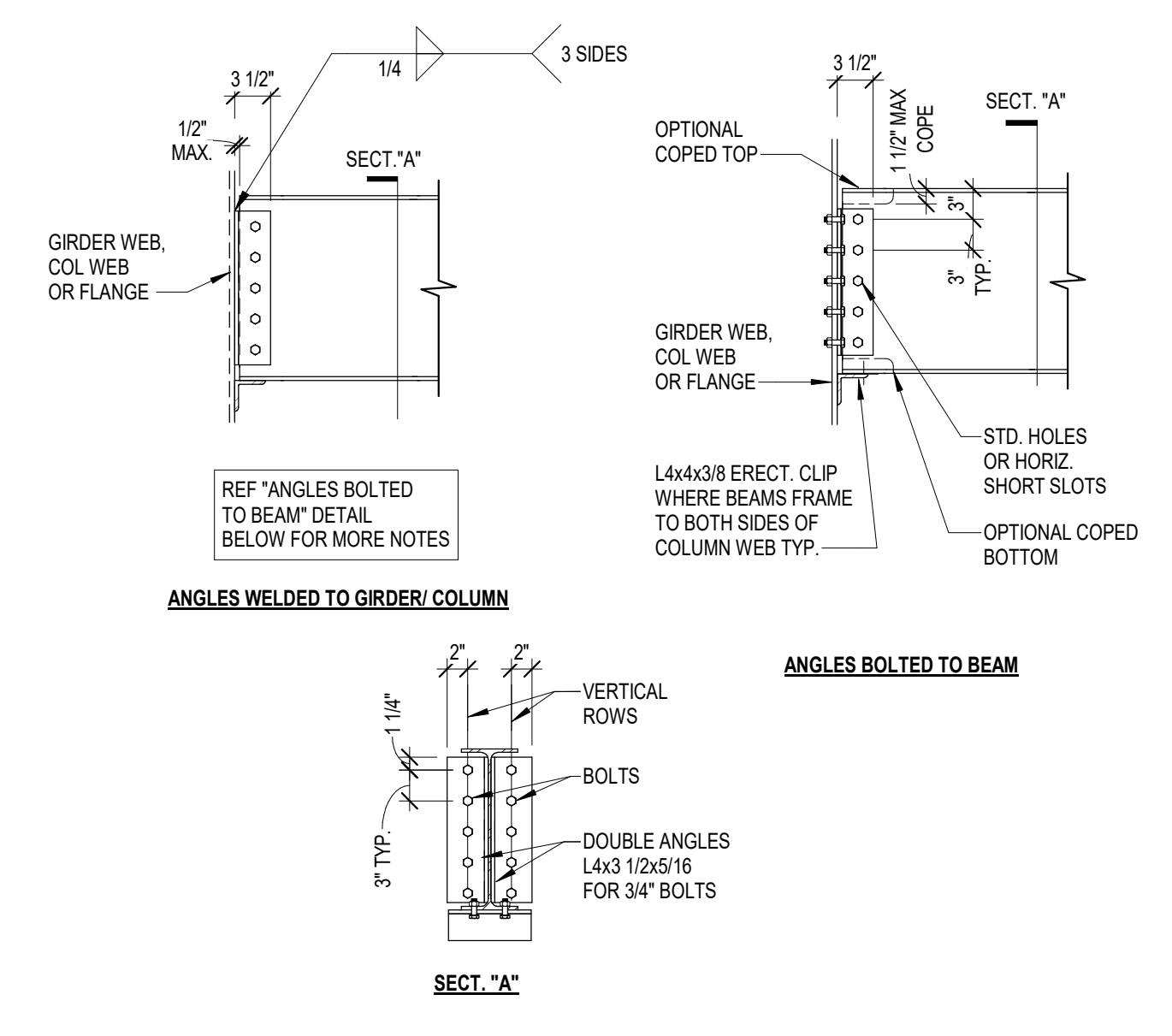
THIS CONNECTION SHALL BE USED ONLY WHEN CONNECTING BEAMS TO COLUMNS OR WHEN CONNECTING BEAMS TO GIRDERS WHERE GIRDER HAS BEAM CONNECTION ON ONE SIDE ONLY IF USED AT LOCATIONS WHERE A BEAM CONNECTION IS OPPOSED BY A BEAM CONNECTION ON THE OTHER SIDE OF THE SUPPORTING MEMBER. AN OSHA-APPROVED METHOD MUST BE USED FOR TEMPORARY ERECTION SUPPORT OF BOTH MEMBERS.

DOUBLE-ANGLE CONNECTION SCHEDULE

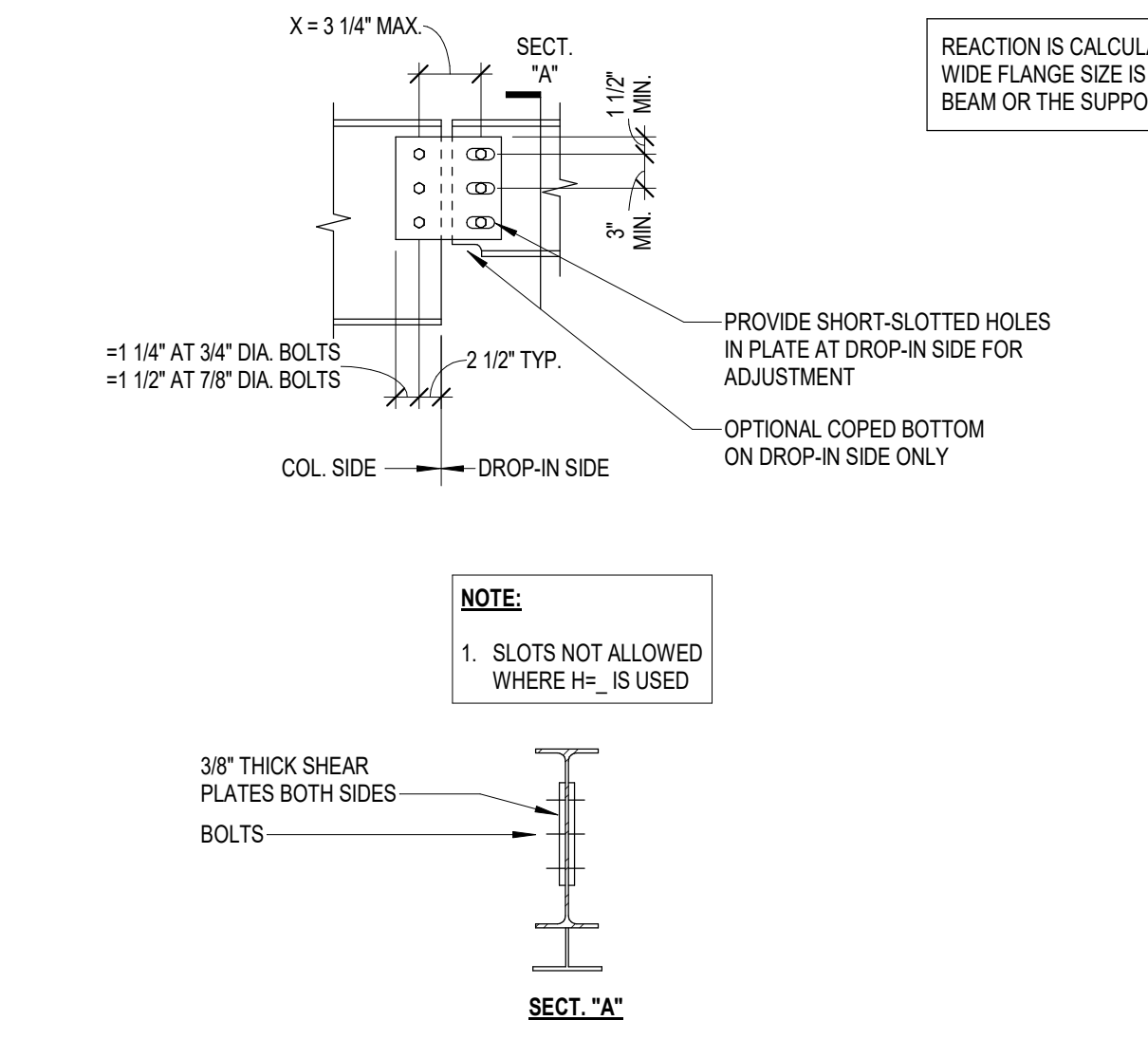
BEAM SIZE	NO. OF BOLTS PER VERT. ROW	ANGLE LENGTH (IN)	MAX CAPACITY (KIPS) (ASD)
W8	2	5 1/2	9
W10	2	5 1/2	14
W12	3	8 1/2	23
W14	3	8 1/2	28
W16	4	11 1/2	40
W18	4	11 1/2	48
W21	5	14 1/2	64
W24	6	17 1/2	83
W27	6	17 1/2	113
W30	7	20 1/2	119
W33	8	23 1/2	142
W36	9	26 1/2	160
W40	10	29 1/2	193
W44	10	29 1/2	195

- NOTES:**
- ALLOWABLE CAPACITIES ARE IN KIPS (SERVICE LEVEL - UNFACTORED).
 - CAPACITIES BASED ON GRADE 50 BEAMS, A325-N BOLTS, AND A36 ANGLES.
 - HOLES SHALL BE STANDARD (STD) OR SHORT-SLOPPED (SSLT).
 - ALL INFORMATION SHOWN IN THE SCHEDULE ABOVE IS TO BE USED AS A MINIMUM REQUIREMENT FOR ALL CONNECTIONS.
 - ALTERNATE CONNECTION DETAILS ARE ACCEPTABLE PROVIDED SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS ARE SUBMITTED, AND ALL CONNECTION DESIGNS ARE PERFORMED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

1 TYPICAL DOUBLE ANGLE CONNECTION
NO SCALE



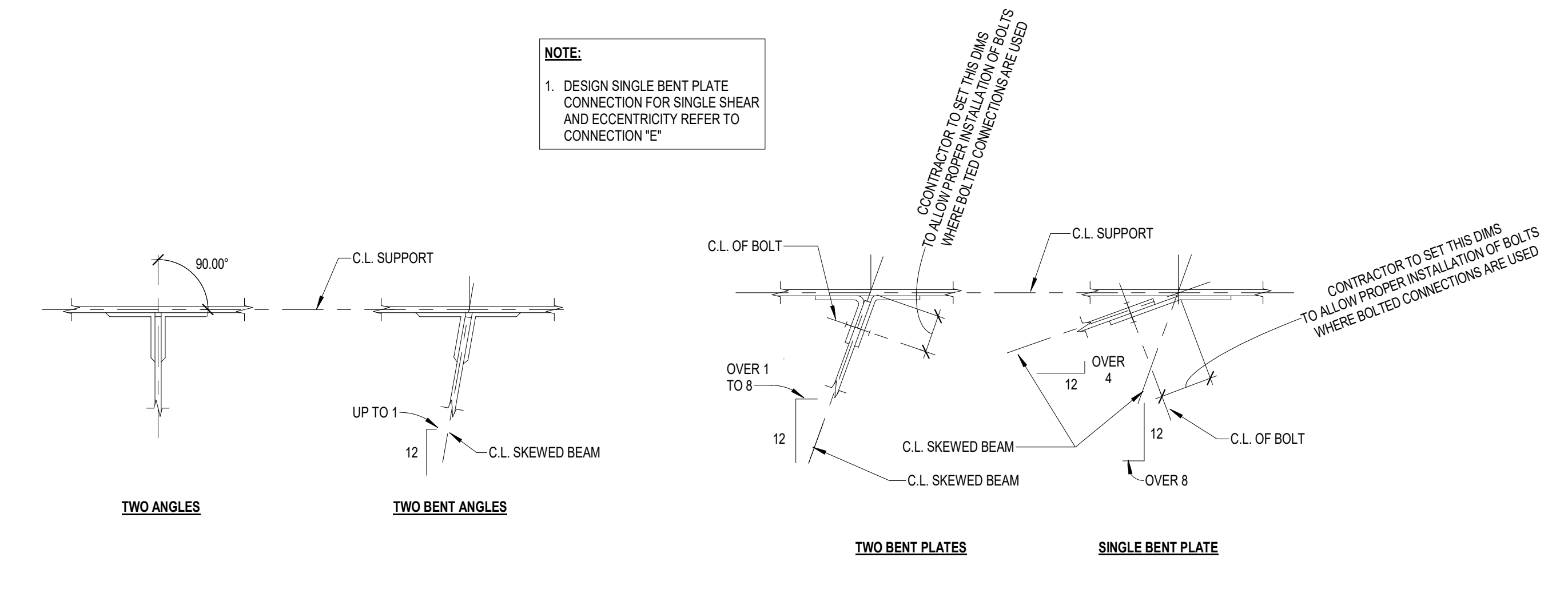
3 TYPICAL SHEAR SPLICE CONNECTION
NO SCALE



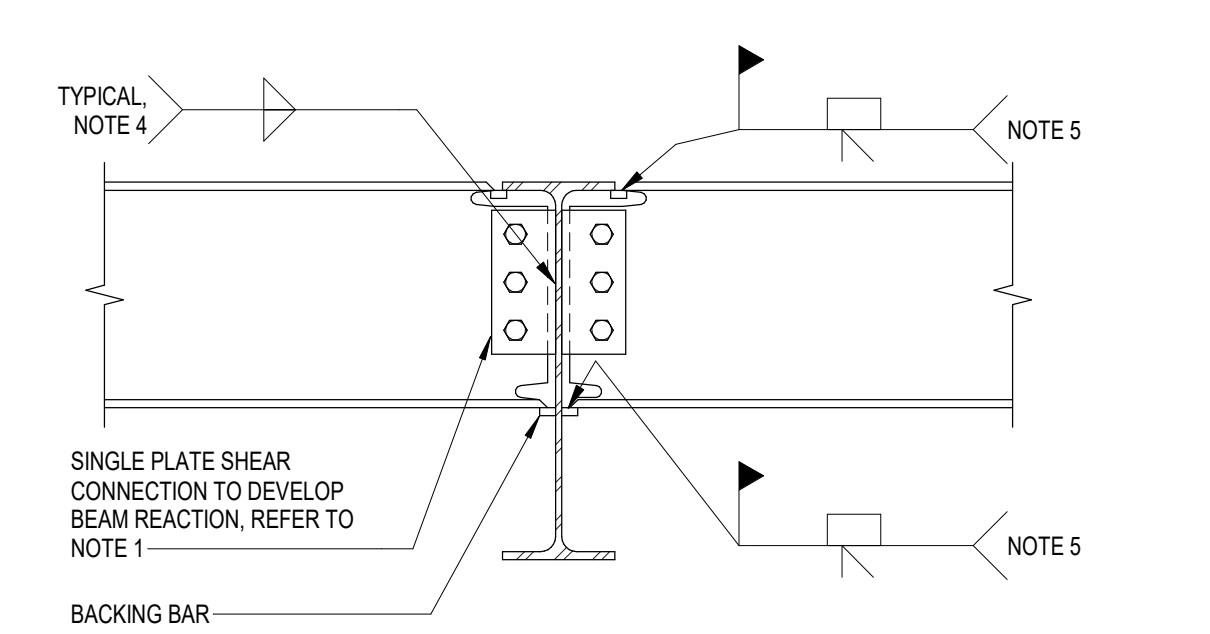
CONNECTION LOAD CAPACITY

WIDE FLANGE BEAM SIZE	# BOLTS PER VERT ROW	MINIMUM BEAM WEB THICKNESS (IN)	MAX REACTION (KIPS) (ASD)	
			3/4" DIA. BOLT X=3 1/4" BOLTS	7/8" DIA. BOLT X=3 3/4" BOLTS
W8x10	2	170	15	14
W10x12	2	190	18	24
W12x14	3	200	30	29
W12x16	3	220	33	32
W8 TO W12	2	230	18	30
W12 TO W18	3	235	36	34
W16 TO W24	4	250	53	51
W18 TO W30	5	300	70	67
W21 TO W44	6	350	97	92
W24 TO W44	7	395	126	119
W30 TO W44	8	470	150	137
W33 TO W44	9	550	173	204
W36 TO W44	10	600	195	261
W40 TO W44	11	630	217	287
W44	12	710	236	313

4 TYPICAL SKEWED SIMPLE FRAMING CONNECTION - BEAM TO BEAM
NO SCALE

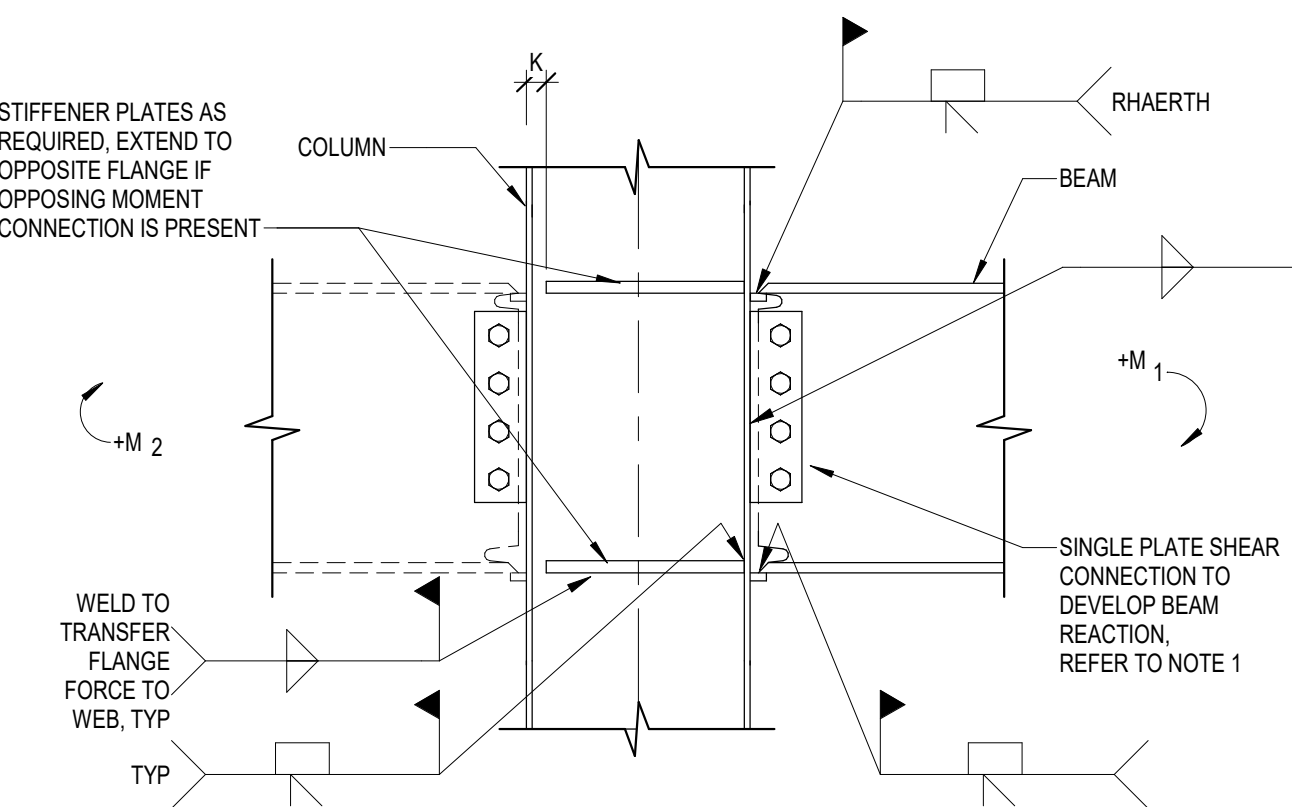


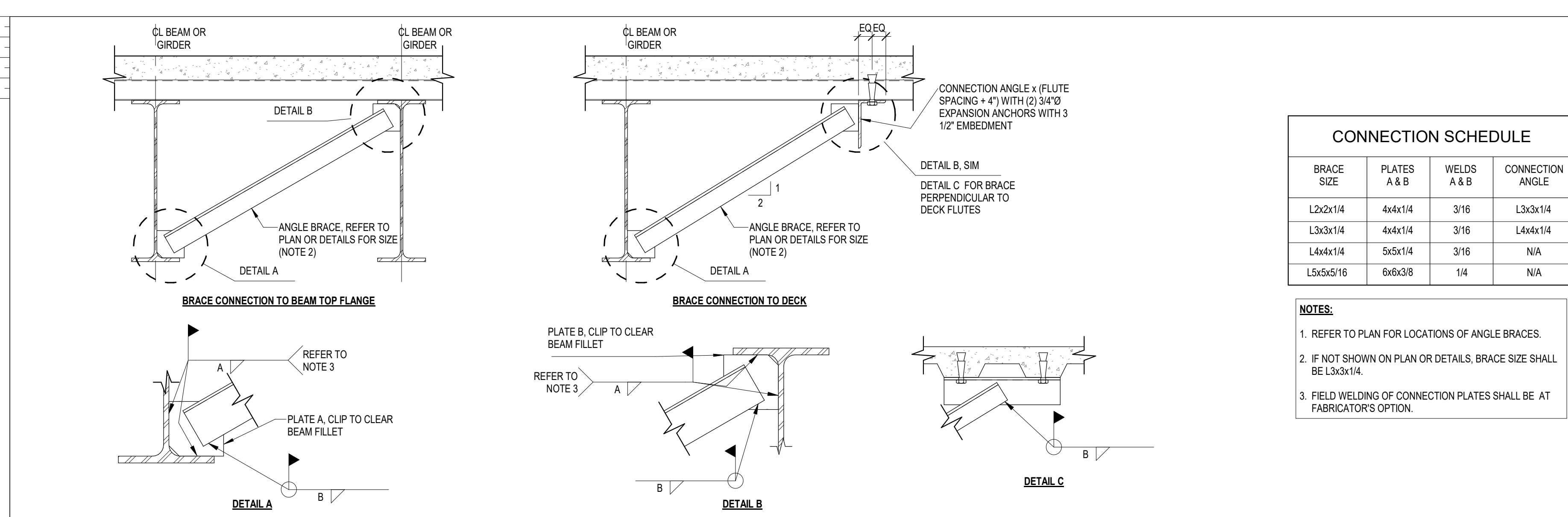
5 TYPICAL BEAM-TO-BEAM MOMENT CONNECTION DETAIL
NO SCALE



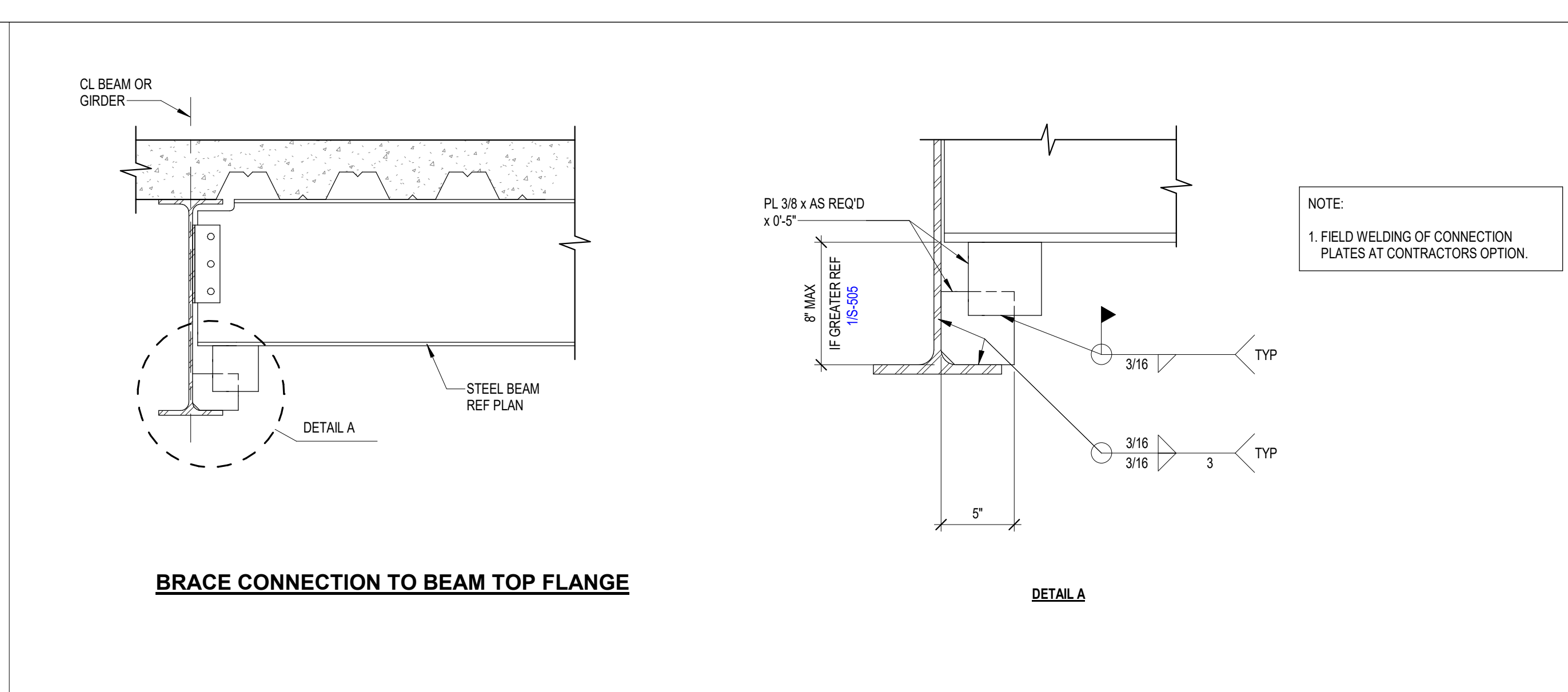
- NOTES:**
- BOLTS IN WEB CONNECTION MAY BE SNUG-TIGHTENED OR PRETENSIONED A325 OR A490 BOLTS WITH HORIZONTAL SHORT-SLOTTED HOLES IN PLATE. IF BOLTS ARE TO BE PRETENSIONED, SNUG-TIGHTEN BOLTS PRIOR TO WELDING FLANGES AND PRETENSION AFTER WELDING FLANGES.
 - REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.
 - PROVIDE PREDESIGNED SHEAR CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION LRFD MANUAL WHERE APPLICABLE.
 - MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.
 - IF MOMENT IS SHOWN ON DRAWINGS, PARTIAL PENETRATION WELD TO DEVELOP MOMENT CAN BE USED IN LIEU OF COMPLETE JOINT PENETRATION WELD. FOR THIS CASE, WELD SHALL BE REQUIRED TO DEVELOP THE BEAM FLANGE FORCE COMPUTED AS FOLLOWS: $P_u = (M_u / 0.95D)$, WHERE:
 M_u = DESIGN MOMENT (KIP-FEET)
 D = BEAM DEPTH (INCHES)
 P_u = BEAM FLANGE FORCE (KIPS)

6 TYPICAL MOMENT CONNECTION AT COLUMN FLANGE
NO SCALE

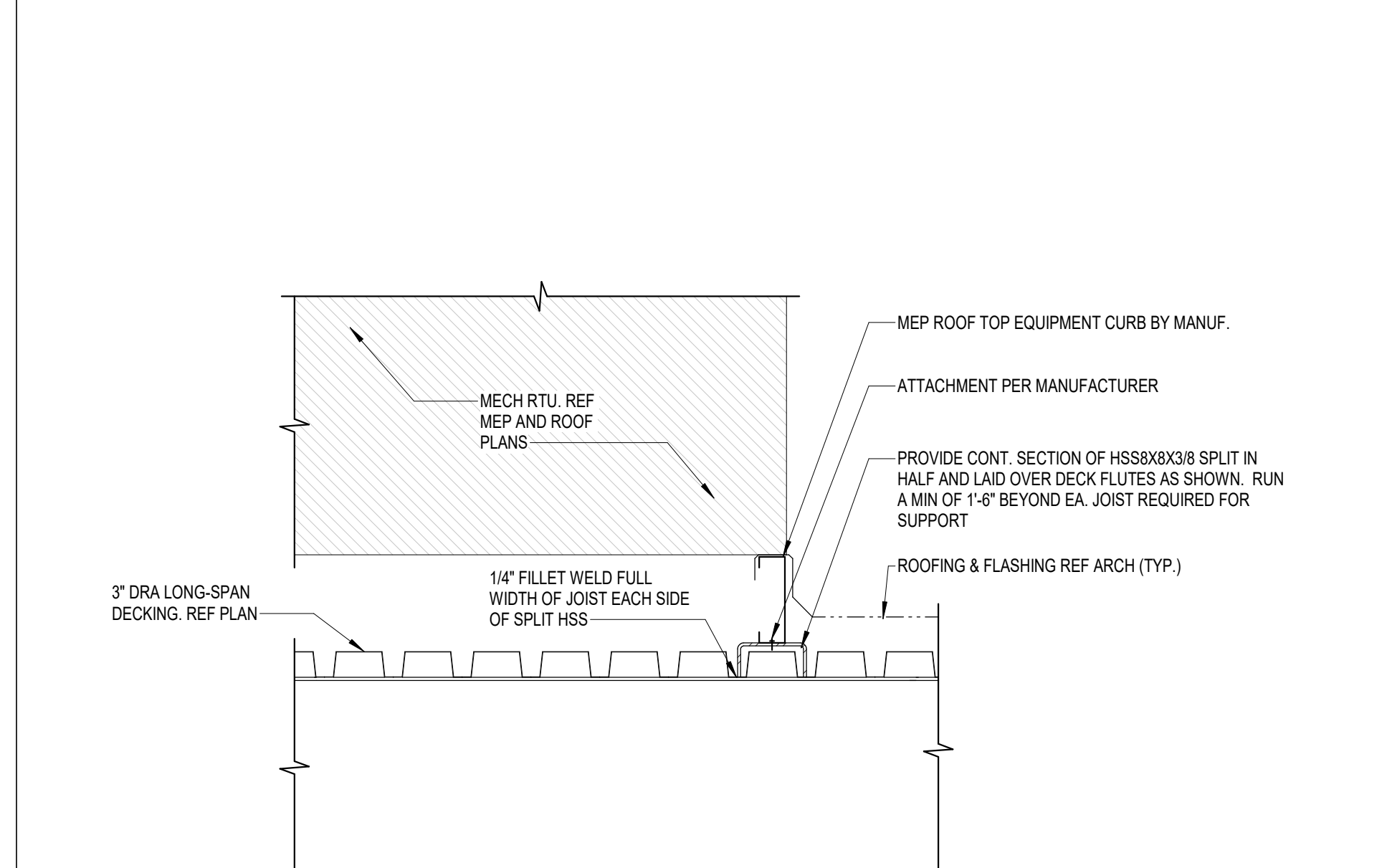




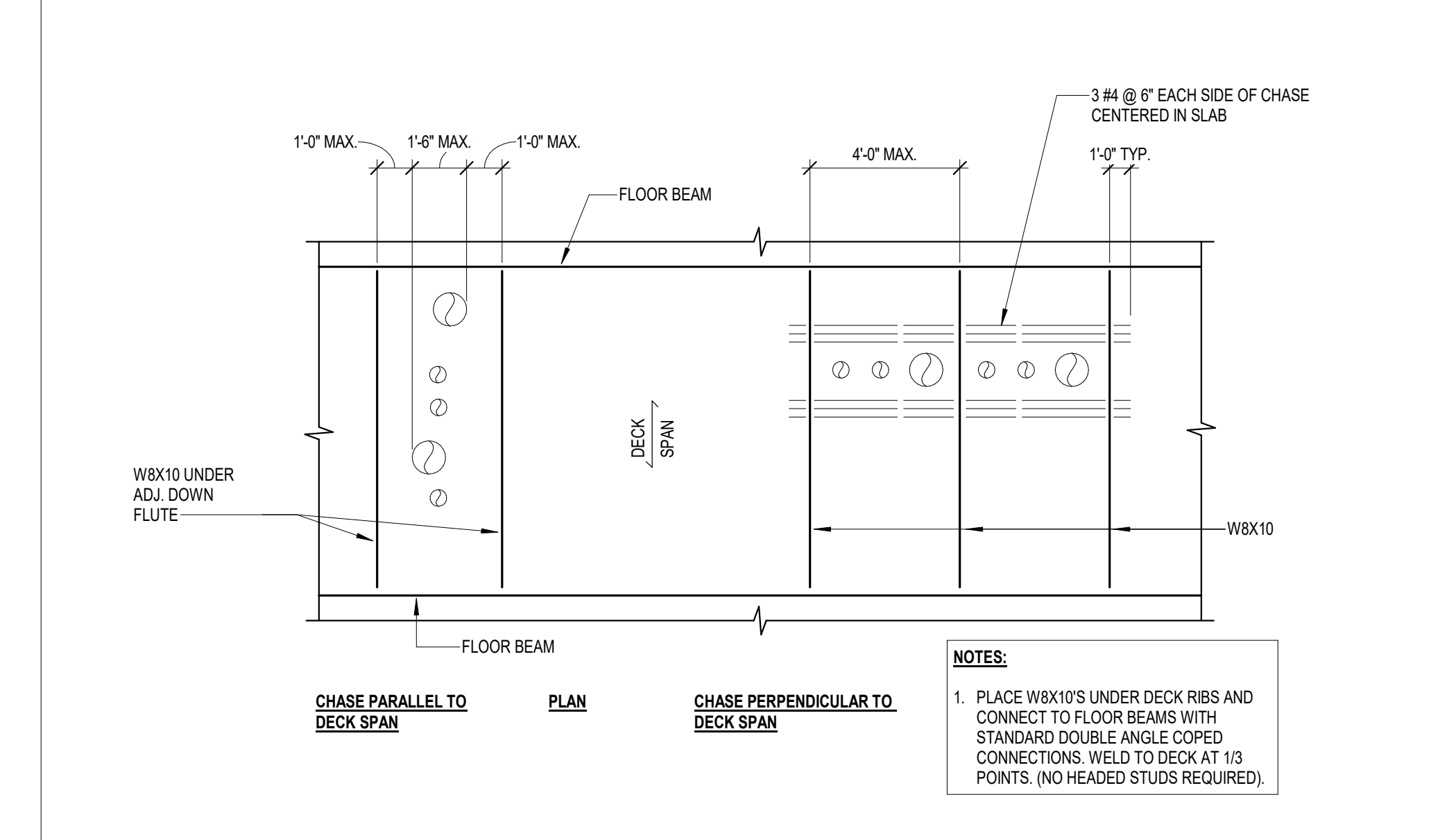
1 TYPICAL BEAM BOTTOM FLANGE BRACE (COMPOSITE CONSTRUCTION)
 NO SCALE



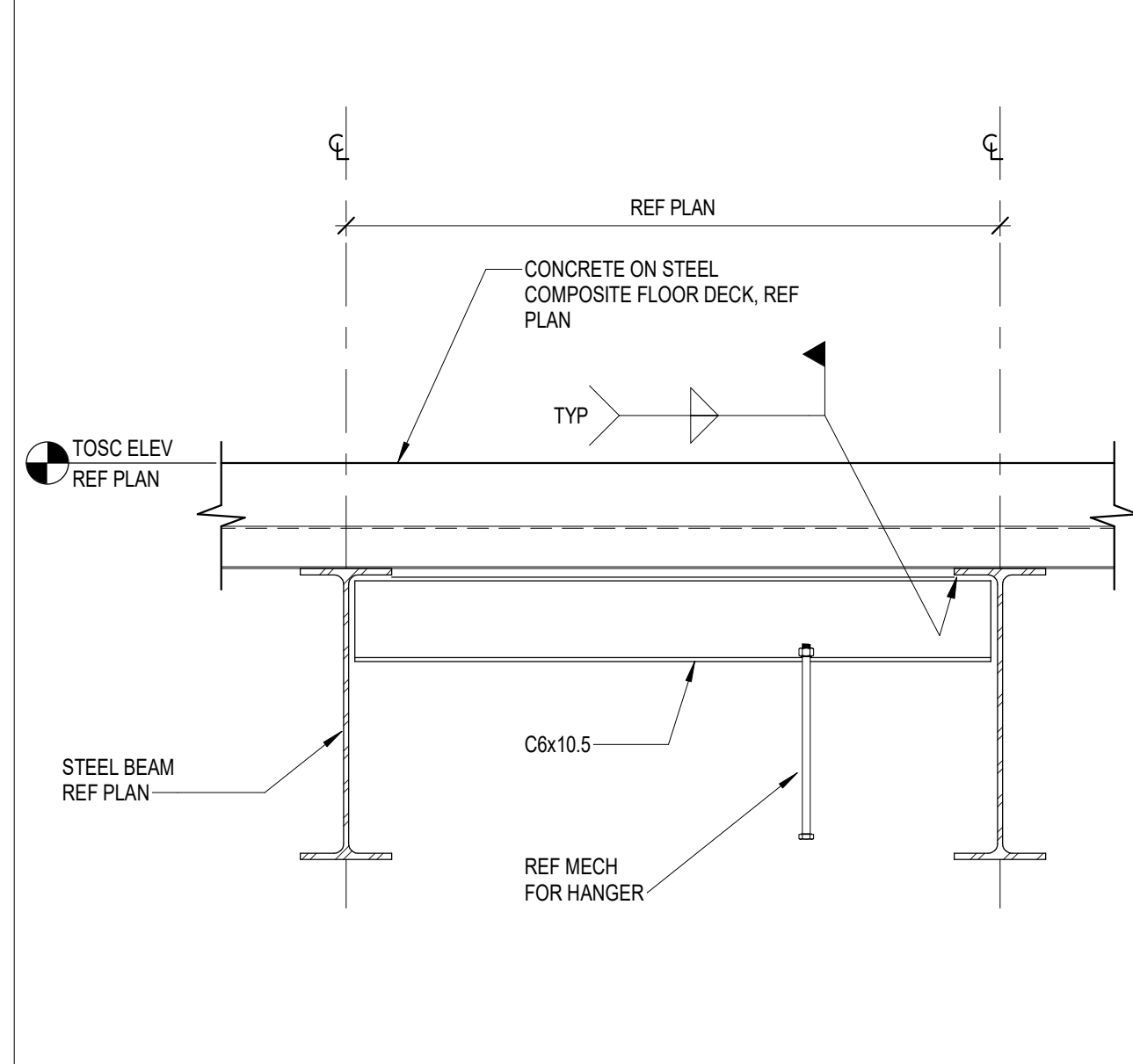
2 TYPICAL BEAM BOTTOM FLANGE BRACE (COMPOSITE CONSTRUCTION)
 1" = 1'-0"



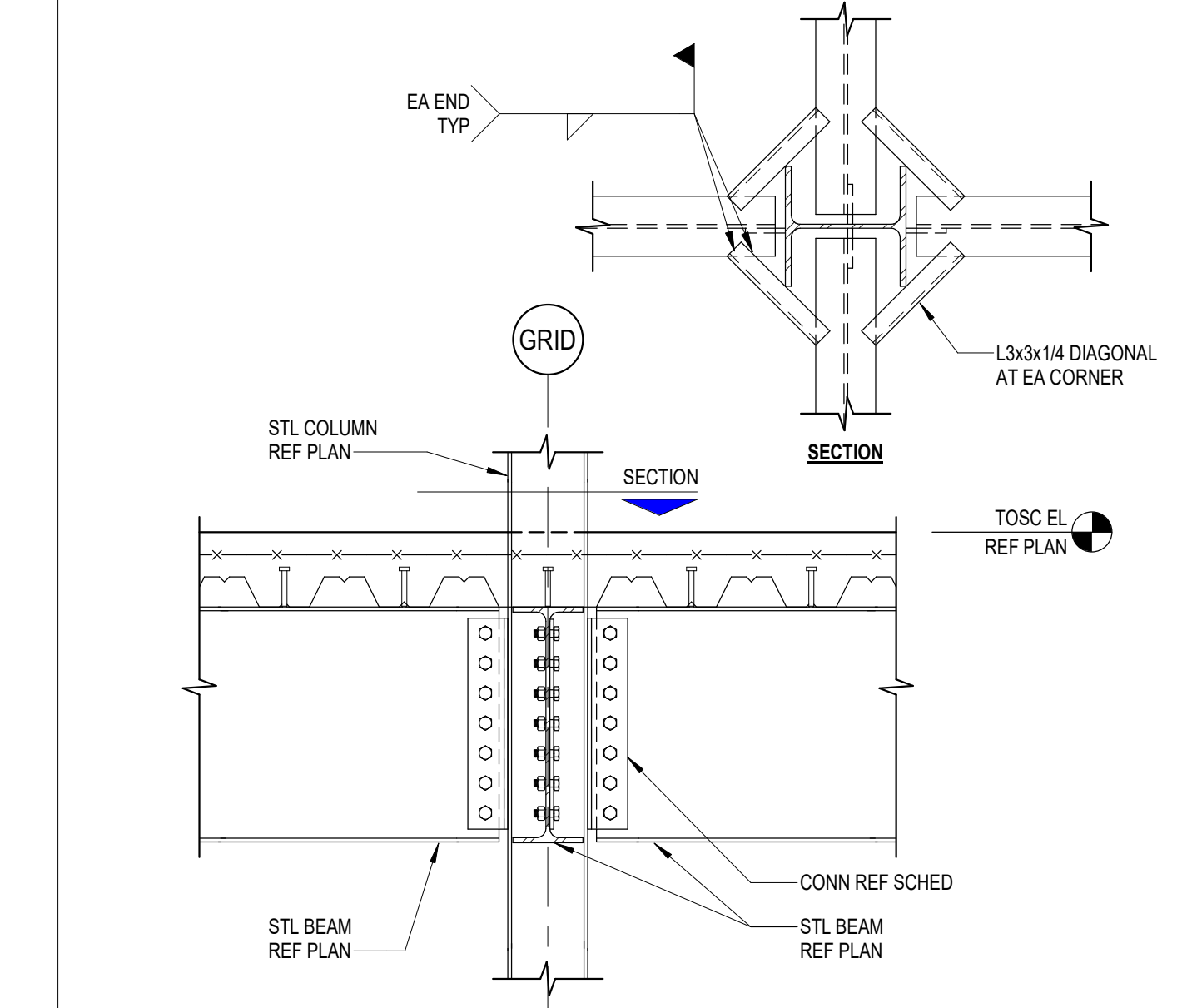
3 TYPICAL RTU SUPPORT
 3/4" = 1'-0"



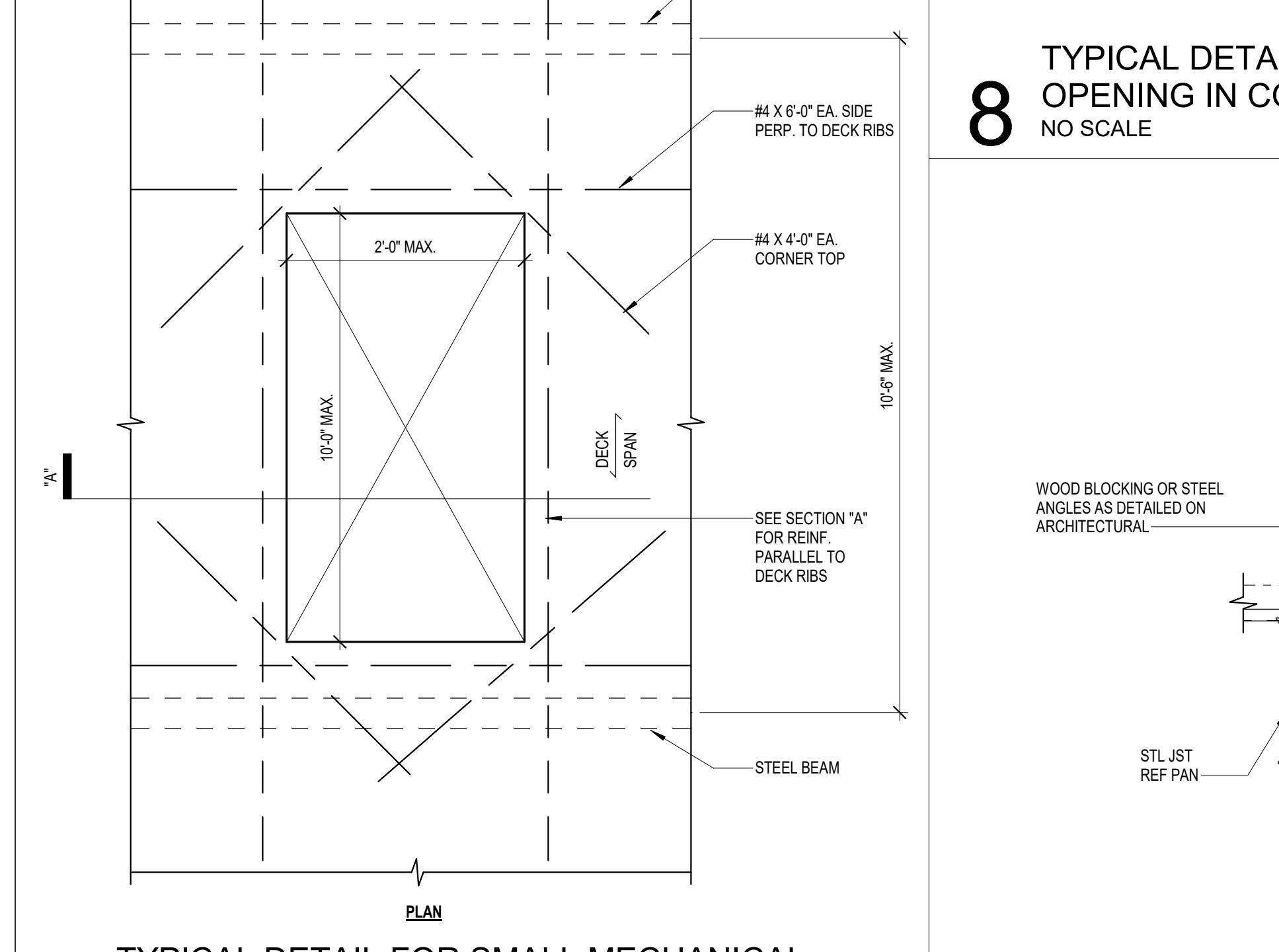
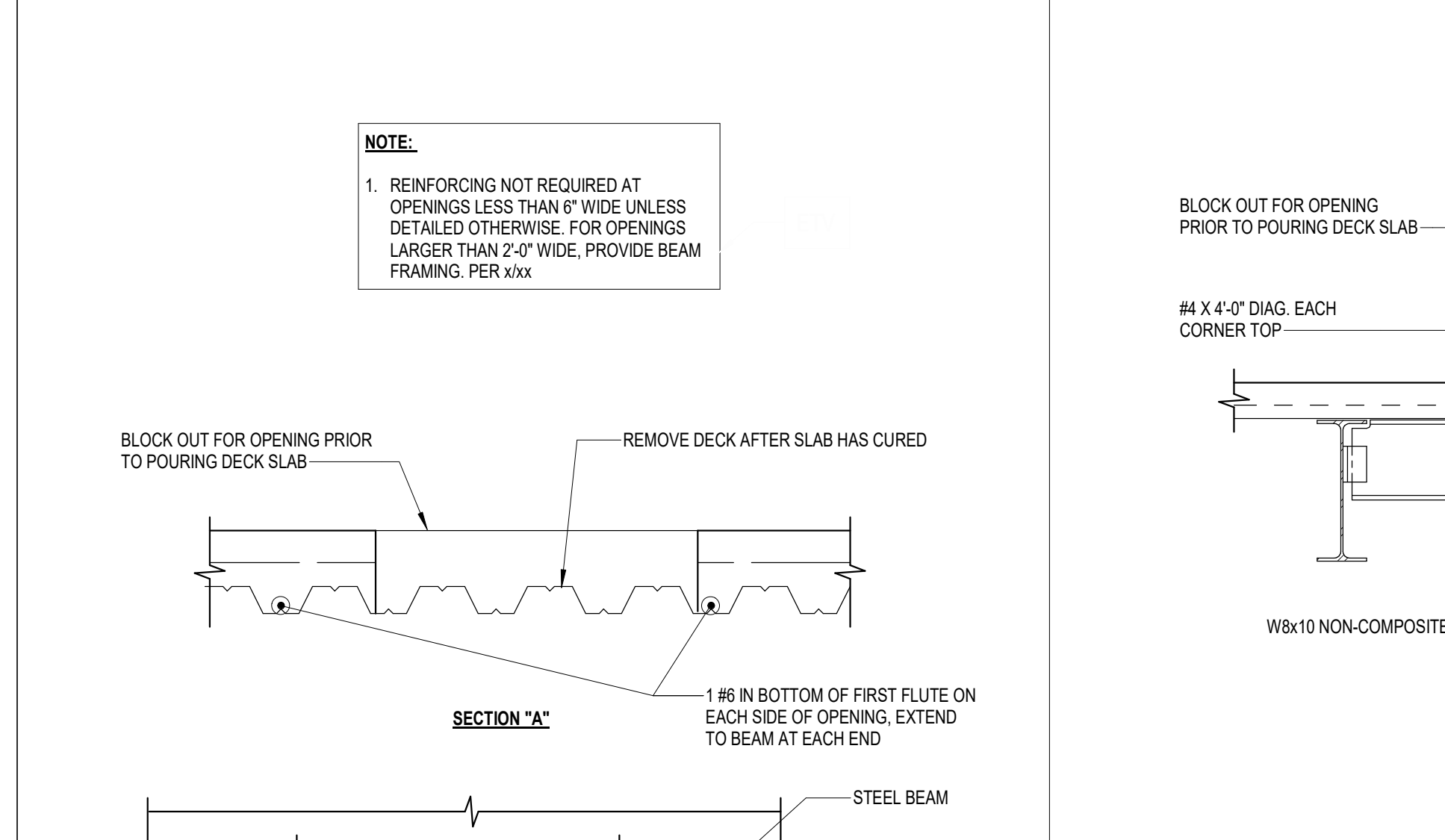
4 TYPICAL DETAIL FOR FLOOR STRENGTHENING AT PLUMBING CHASE
 NO SCALE



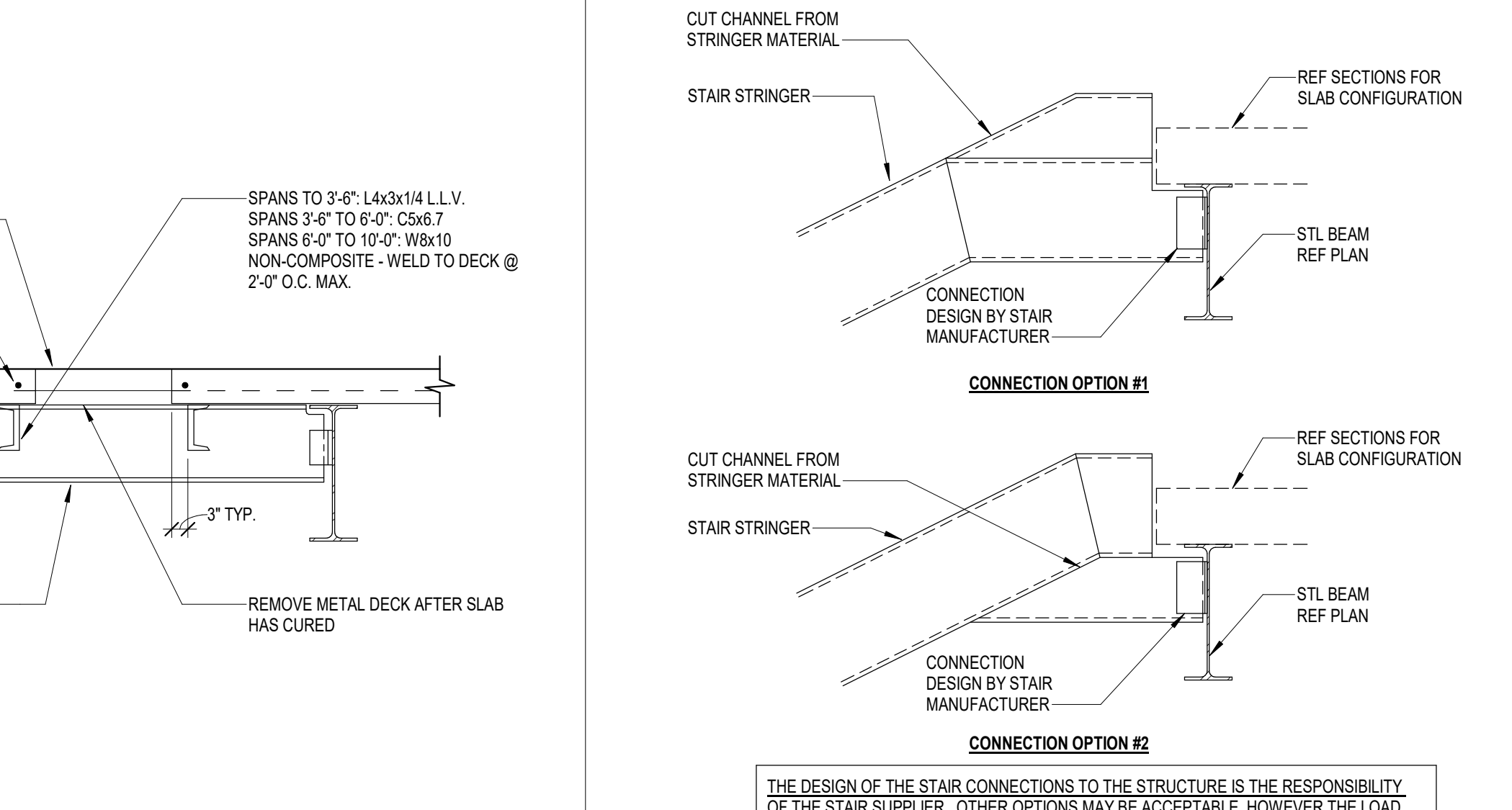
5 TYPICAL MECHANICAL HANGER AT FLOOR
 NO SCALE



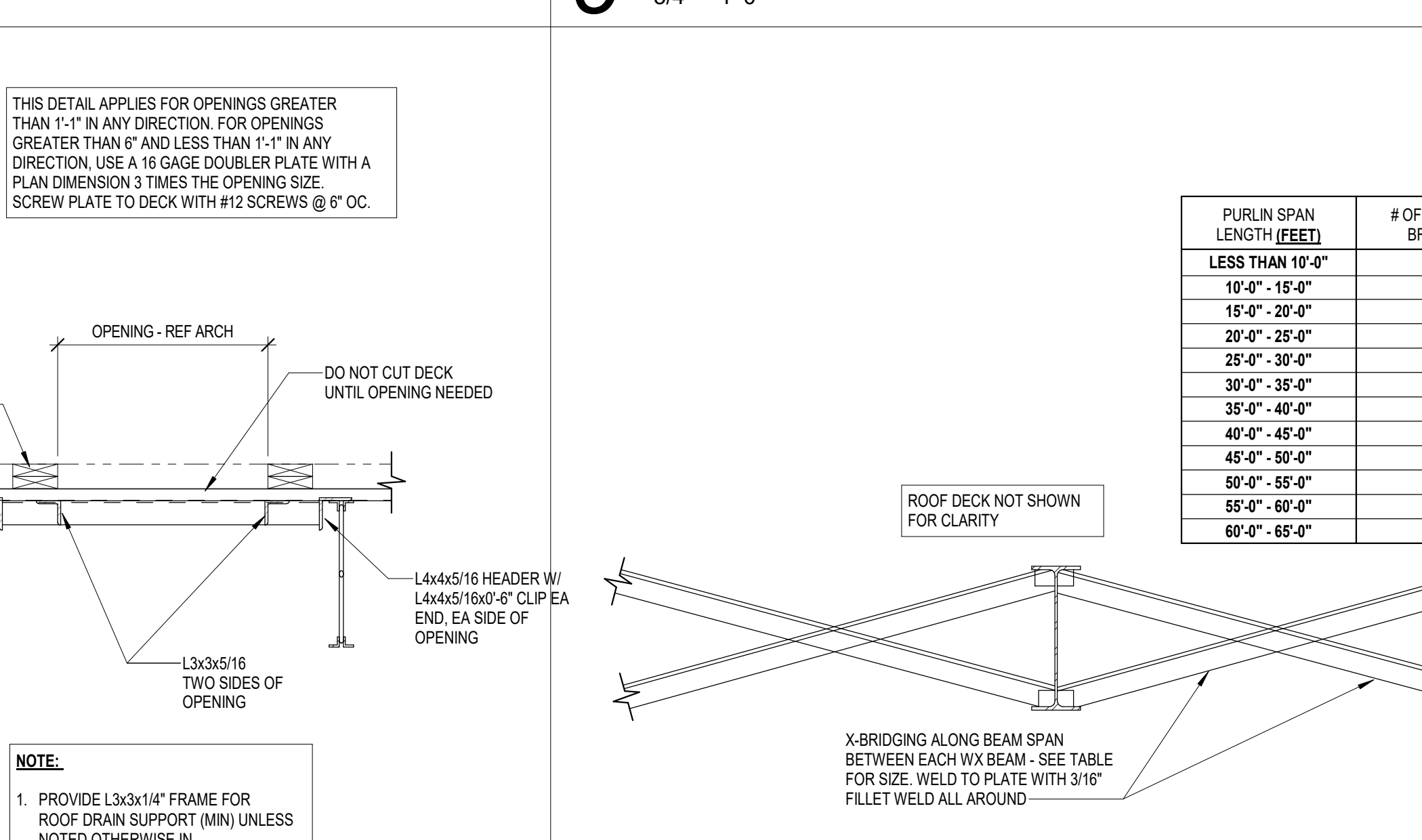
6 TYPICAL DECK SUPPORT AT COLUMN DETAIL
 3/4" = 1'-0"



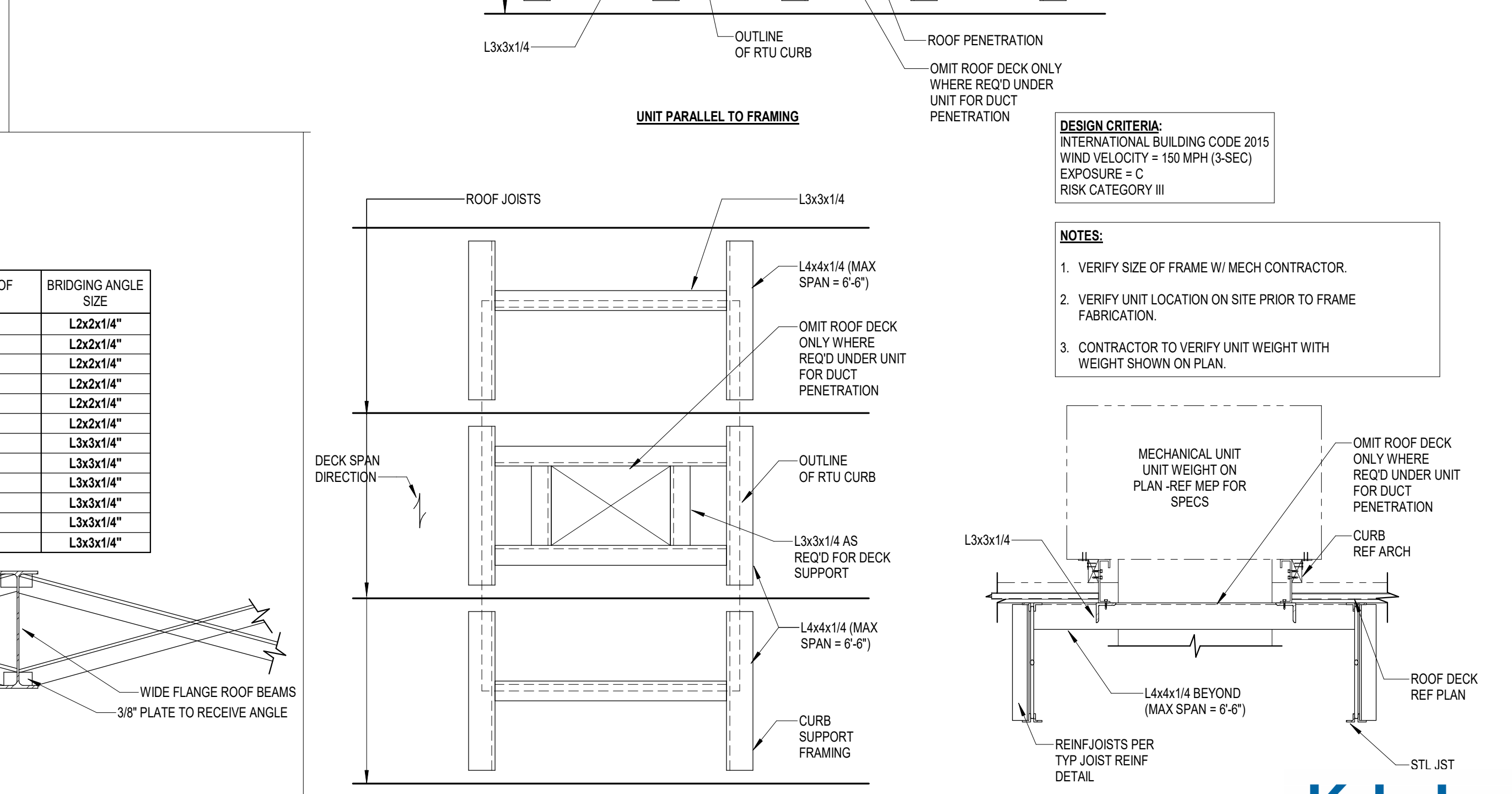
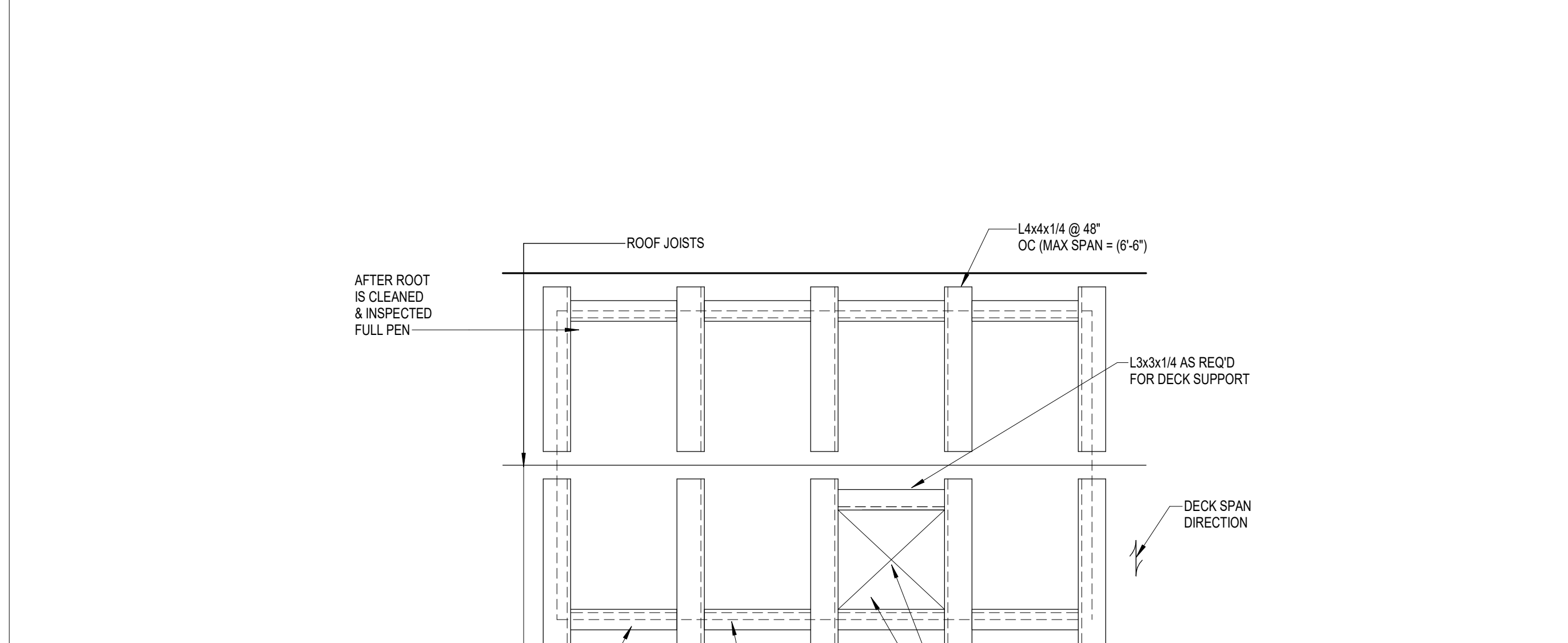
7 TYPICAL DETAIL FOR SMALL MECHANICAL OPENING IN COMPOSITE SLAB
 NO SCALE



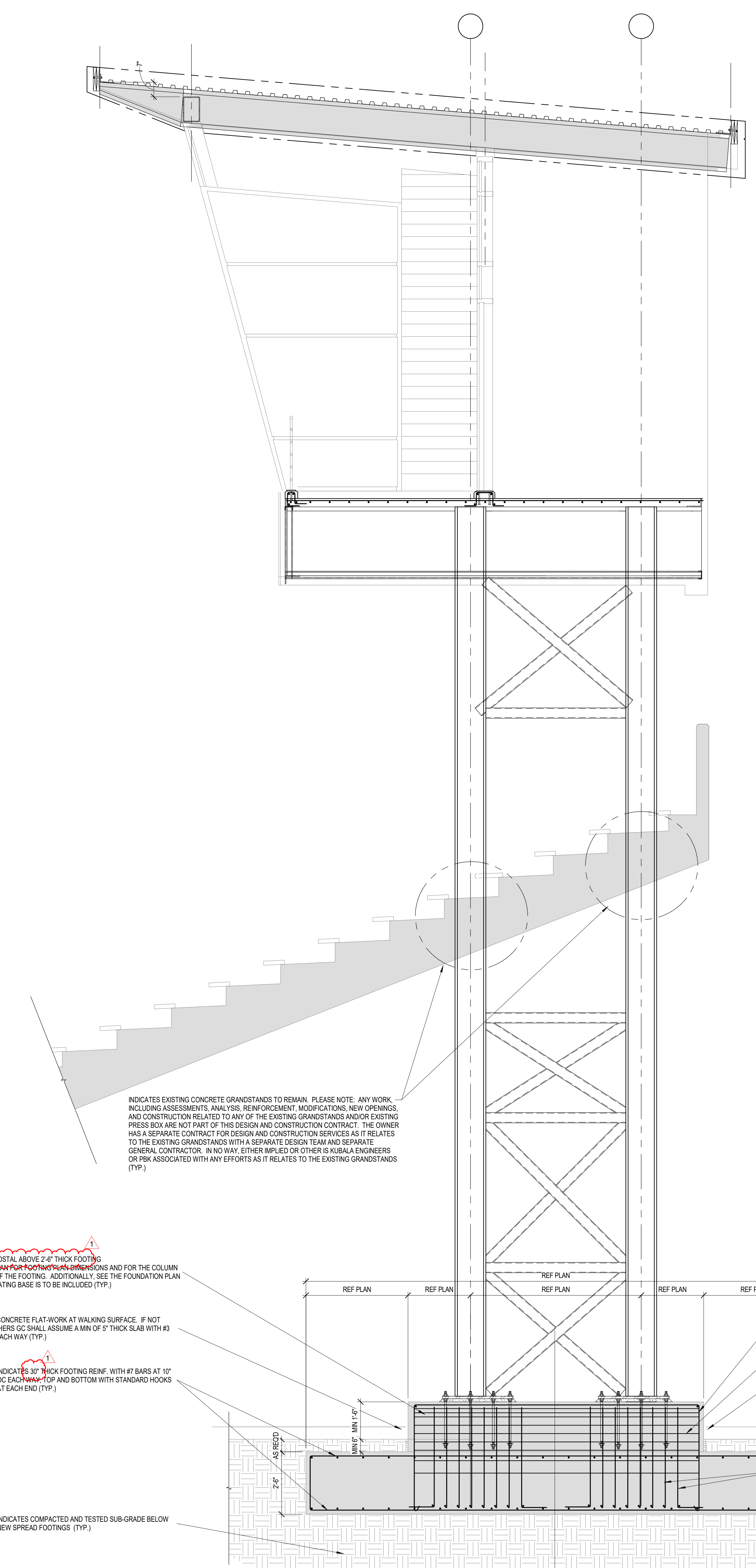
9 TYPICAL CONNECTION OPTIONS FOR STAIR STRINGER TO STRUCTURAL STEEL
 3/4" = 1'-0"



11 TYPICAL UPLIFT BRIDGING DETAIL
 3/4" = 1'-0"



12 TYPICAL SUPPORT AT MECHANICAL UNIT DETAIL
 NO SCALE



INDICATES EXISTING CONCRETE GRANDSTANDS TO REMAIN. PLEASE NOTE: ANY WORK, INCLUDING ASSESSMENTS, ANALYSIS, REINFORCEMENT, MODIFICATIONS, NEW OPENINGS, AND CONSTRUCTION RELATED TO ANY OF THE EXISTING GRANDSTANDS AND/OR EXISTING PRESS BOX ARE NOT PART OF THIS DESIGN AND CONSTRUCTION CONTRACT. THE OWNER HAS A SEPARATE CONTRACT FOR DESIGN AND CONSTRUCTION SERVICES AS IT RELATES TO THE EXISTING GRANDSTANDS WITH A SEPARATE DESIGN TEAM AND SEPARATE GENERAL CONTRACTOR. IN NO WAY, EITHER IMPLIED OR OTHER IS KUBALA ENGINEERS OR PBK ASSOCIATED WITH ANY EFFORTS AS IT RELATES TO THE EXISTING GRANDSTANDS (TYP.)

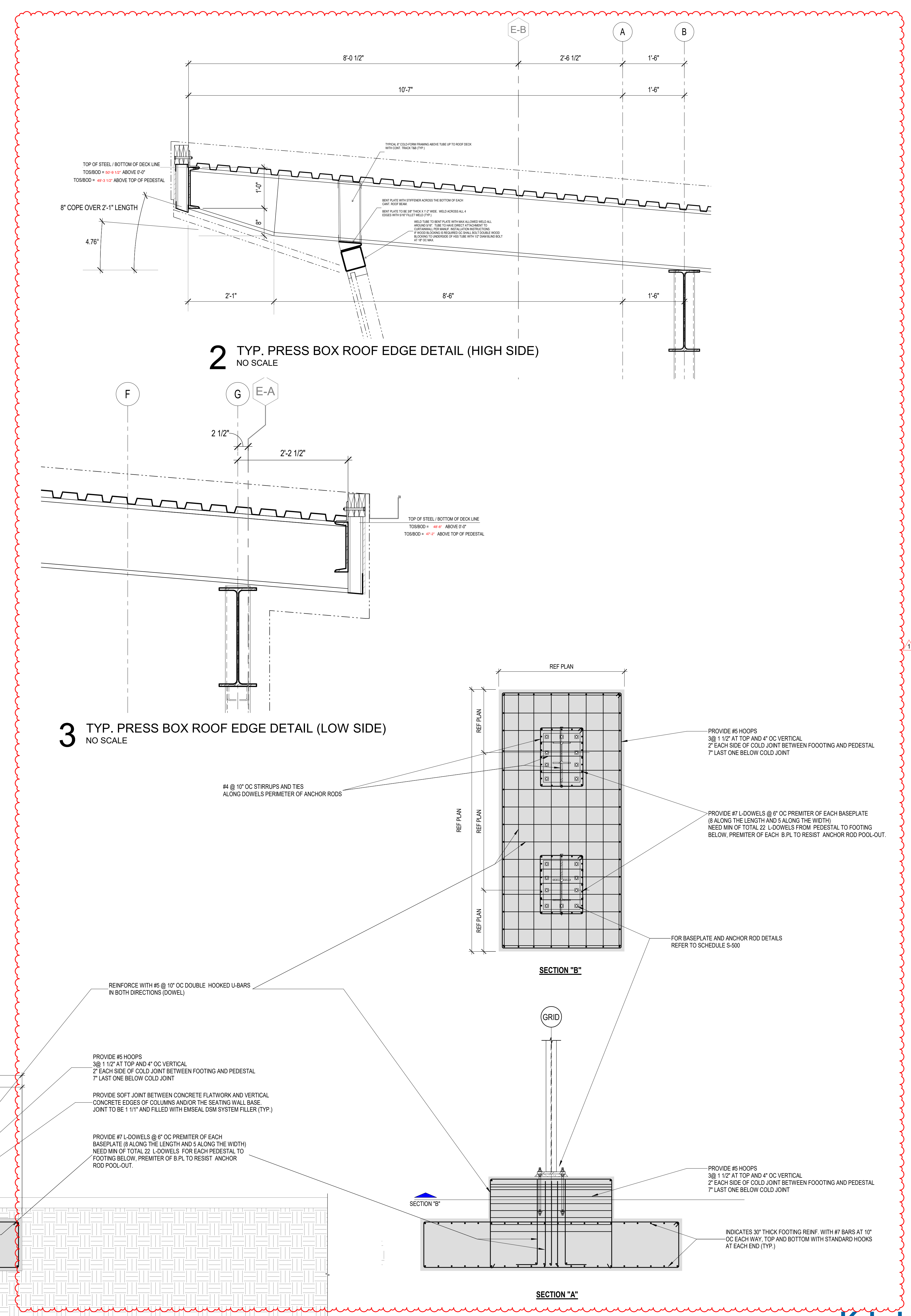
INDICATES MIN 2" PEDISTAL ABOVE 2'-0" THICK FOOTING FOR FOUNDATION FOR COLUMN OR BEAMS. INDICATES LOCATION ON TOP OF THE FOOTING. ADDITIONALLY, SEE THE FOUNDATION PLAN FOR WHERE THE SEATING BASE IS TO BE INCLUDED (TYP.)

INDICATES NEW CONCRETE FLAT-WORK AT WALKING SURFACE. IF NOT SPECIFIED BY OTHERS GC SHALL ASSUME A MIN OF 5" THICK SLAB WITH #3 BARS AT 12" OC EACH WAY (TYP.)

INDICATES 30" THICK FOOTING REINF. WITH #7 BARS AT 10" OC EACH WAY, TOP AND BOTTOM WITH STANDARD HOOKS AT EACH END (TYP.)

INDICATES COMPACTED AND TESTED SUB-GRADE BELOW NEW SPREAD FOOTINGS (TYP.)

1 PRESS BOX SECTION
1 : 27



2 TYP. PRESS BOX ROOF EDGE DETAIL (HIGH SIDE)
NO SCALE

3 TYP. PRESS BOX ROOF EDGE DETAIL (LOW SIDE)
NO SCALE

PROVIDE #5 HOOPS 3/8" 1 1/2" AT TOP AND 4" OC VERTICAL 2" EACH SIDE OF COLD JOINT BETWEEN FOOTING AND PEDESTAL 7" LAST ONE BELOW COLD JOINT

PROVIDE #7 L-DOWELS @ 8" OC PERIMETER OF EACH BASEPLATE (8 ALONG THE LENGTH AND 5 ALONG THE WIDTH) NEED MIN OF TOTAL 22 L-DOWELS FROM PEDESTAL TO FOOTING BELOW, PERIMETER OF EACH B.P.L. TO RESIST ANCHOR ROD POOL-OUT.

FOR BASEPLATE AND ANCHOR ROD DETAILS REFER TO SCHEDULE S-500

REINFORCE WITH #5 @ 10" OC DOUBLE HOOKED L-BARS IN BOTH DIRECTIONS (DOWEL)

PROVIDE #5 HOOPS 3/8" 1 1/2" AT TOP AND 4" OC VERTICAL 2" EACH SIDE OF COLD JOINT BETWEEN FOOTING AND PEDESTAL 7" LAST ONE BELOW COLD JOINT

PROVIDE SOFT JOINT BETWEEN CONCRETE FLATWORK AND VERTICAL CONCRETE EDGES OF COLUMNS AND/OR THE SEATING WALL BASE. JOINT TO BE 1-1/2" AND FILLED WITH EMBEAL DSI SYSTEM FILLER (TYP.)

PROVIDE #7 L-DOWELS @ 8" OC PERIMETER OF EACH BASEPLATE (8 ALONG THE LENGTH AND 5 ALONG THE WIDTH) NEED MIN OF TOTAL 22 L-DOWELS FOR EACH PEDESTAL TO FOOTING BELOW, PERIMETER OF B.P.L. TO RESIST ANCHOR ROD POOL-OUT.

PROVIDE #5 HOOPS 3/8" 1 1/2" AT TOP AND 4" OC VERTICAL 2" EACH SIDE OF COLD JOINT BETWEEN FOOTING AND PEDESTAL 7" LAST ONE BELOW COLD JOINT

INDICATES 30" THICK FOOTING REINF. WITH #7 BARS AT 10" OC EACH WAY, TOP AND BOTTOM WITH STANDARD HOOKS AT EACH END (TYP.)

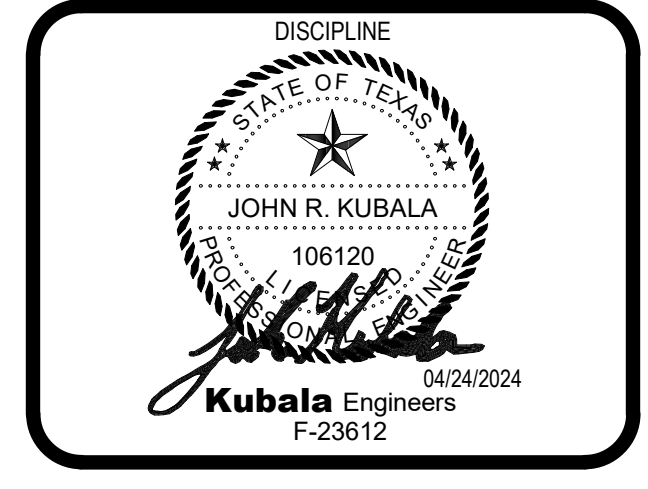
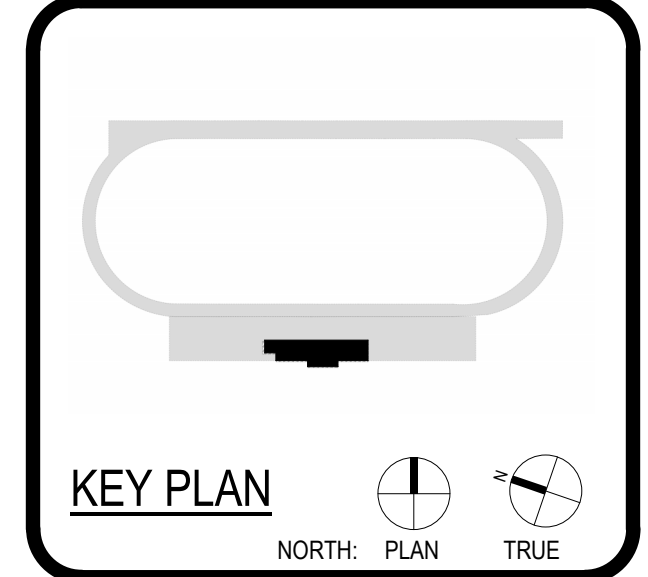


ARCHITECT
HOUSTON
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713-961-4571 F
TX Firm: BR 1698

STRUCTURAL
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1113 98th Street
Houston, TX 77036
LEAF ENGINEERS
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1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL

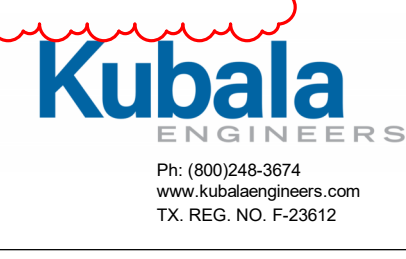


CLIENT GALVESTON ISD		
DATE 01/31/2024	PROJECT NUMBER 230063	
DRAWING HISTORY		
No.	Description	Date
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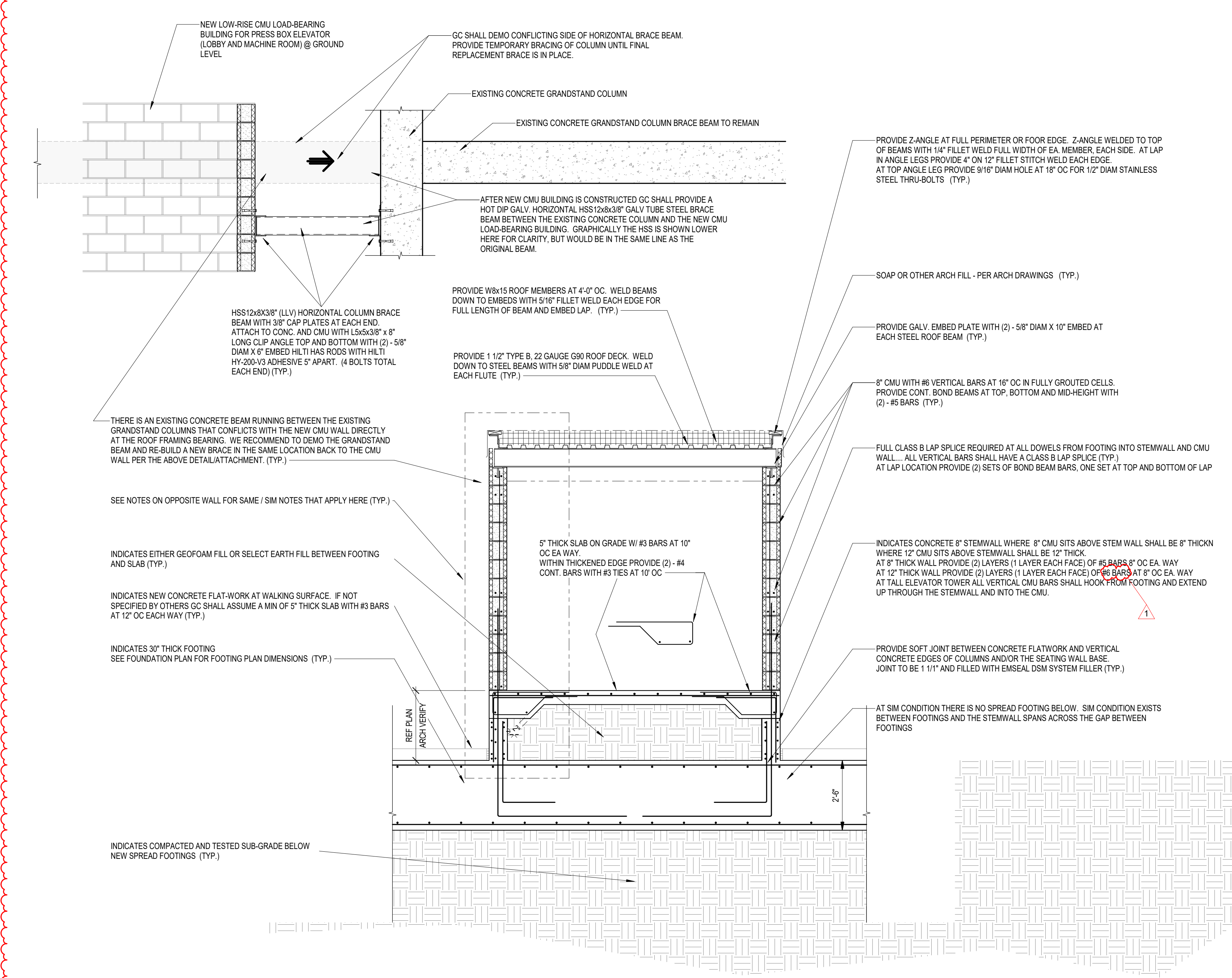
ISSUE FOR PROPOSAL
BUILDING NUMBER

PRESS BOX BUILDING SECTIONS

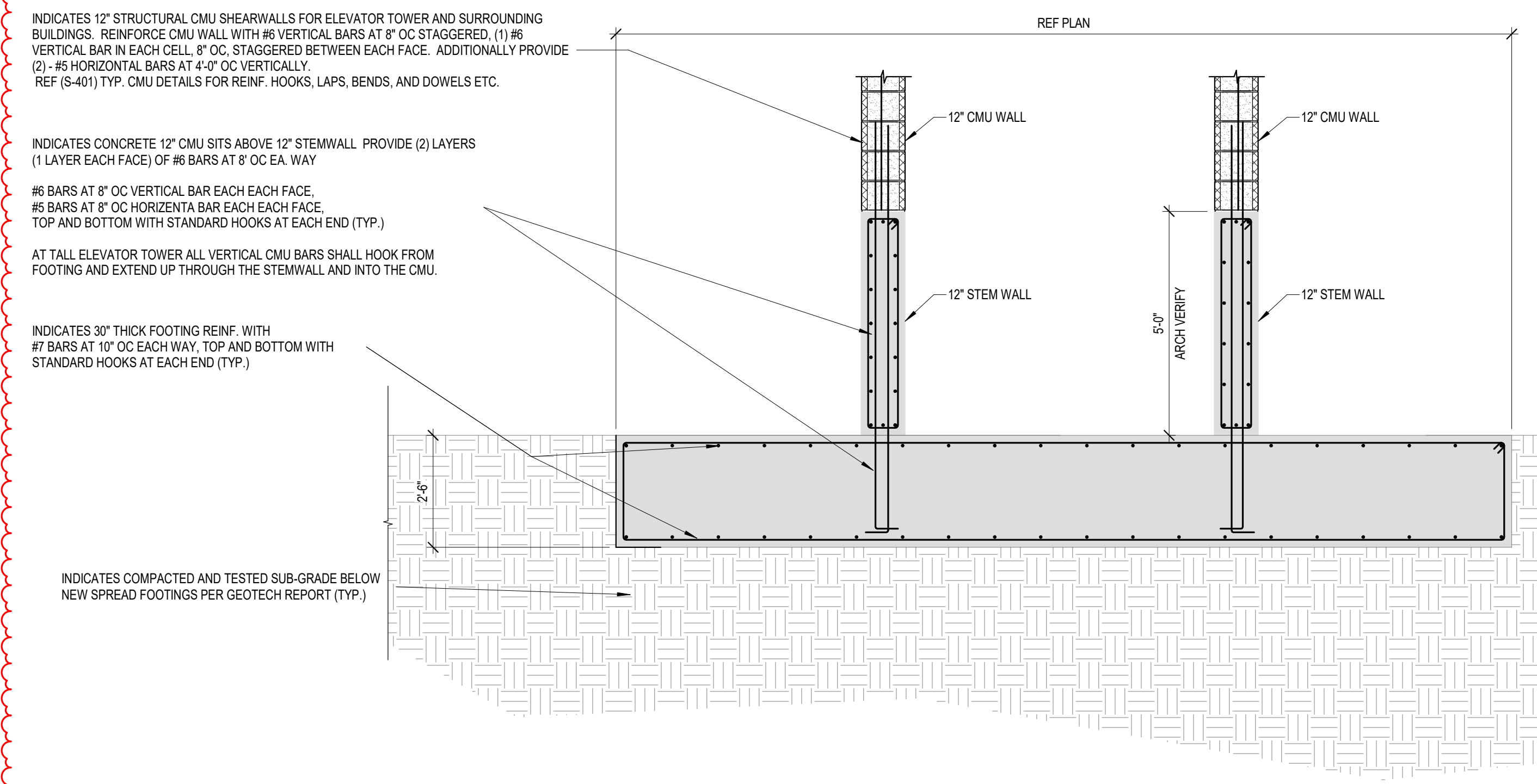
S-525



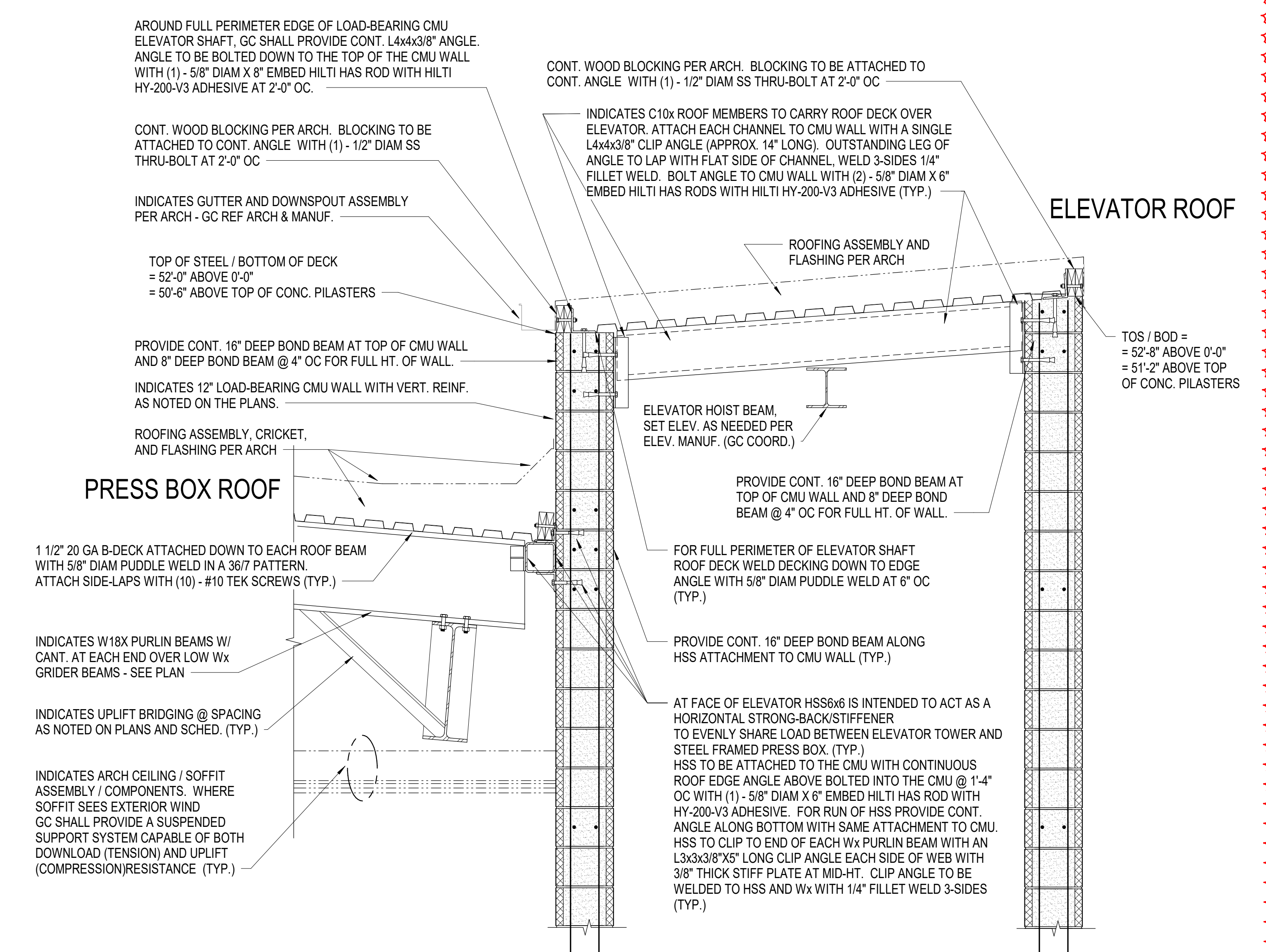
FOR BLUEBEAM LABELING: OOR
 \$528
 PRESS BOX BUILDING SECTIONS



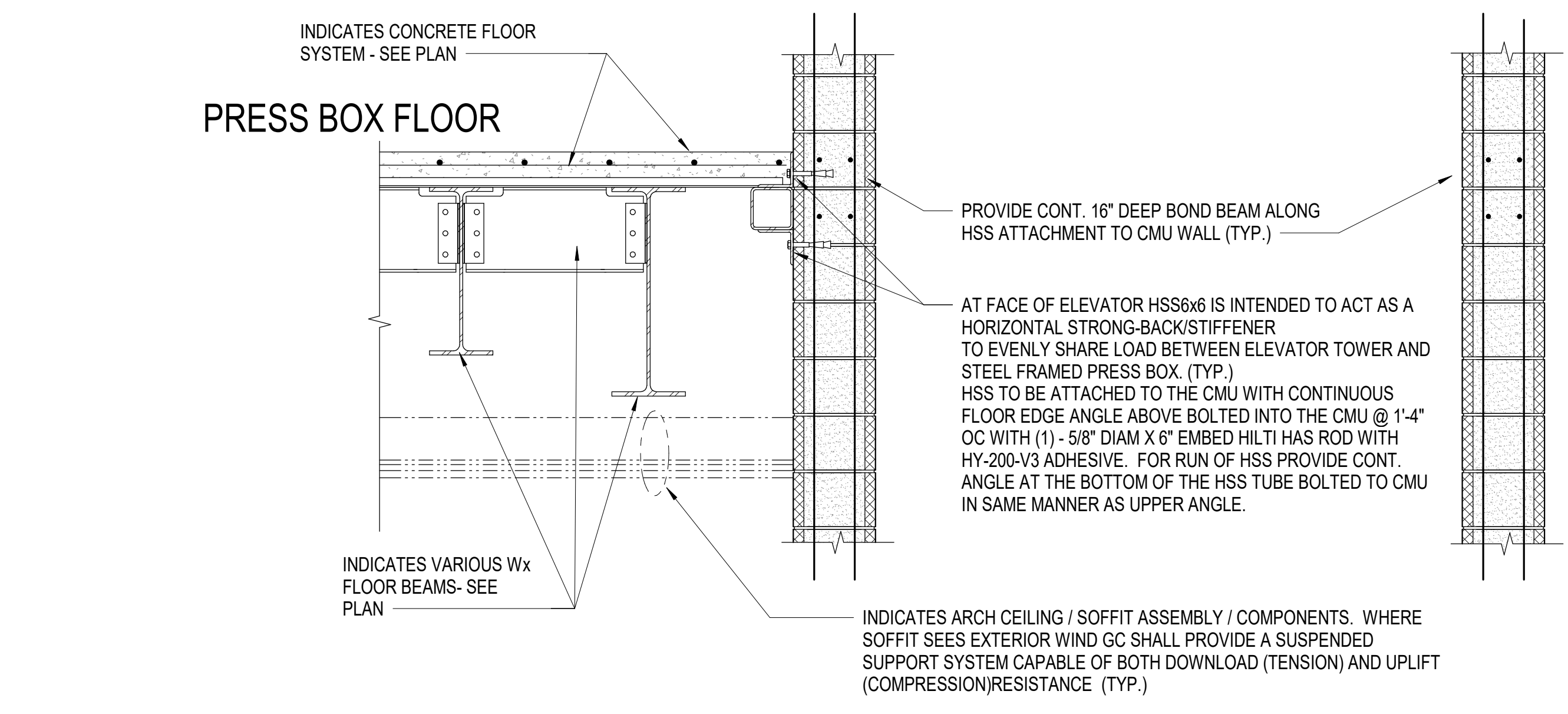
1 SECTION AT FIRE RISER ROOM AND ELEVATOR LOBBY
 1 : 28



2 ELEVATOR SHAFT AND PIT
 1 : 28



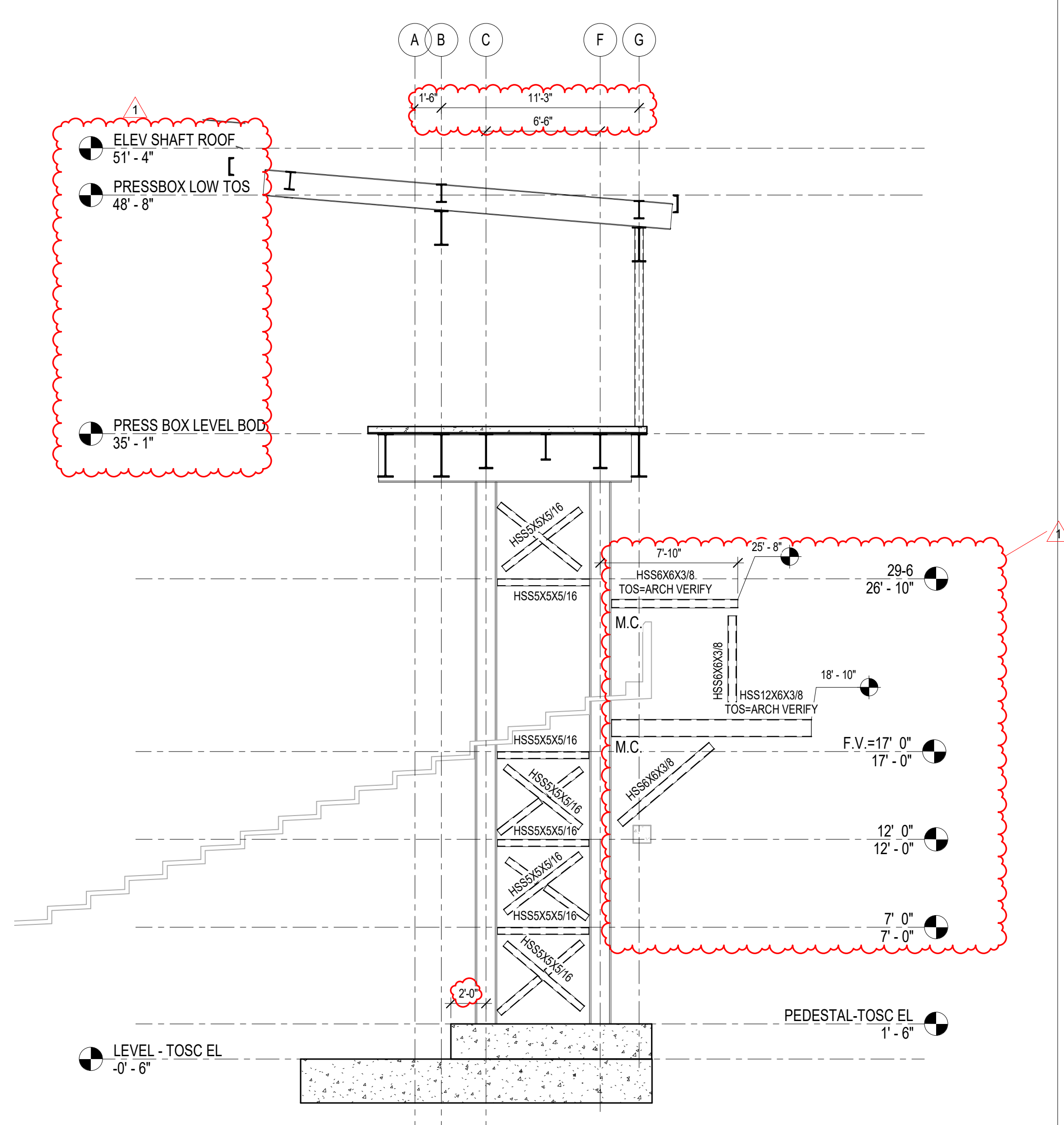
SECTION VIEW @ ELEVATOR ROOF & LOW PRESS BOX ROOF



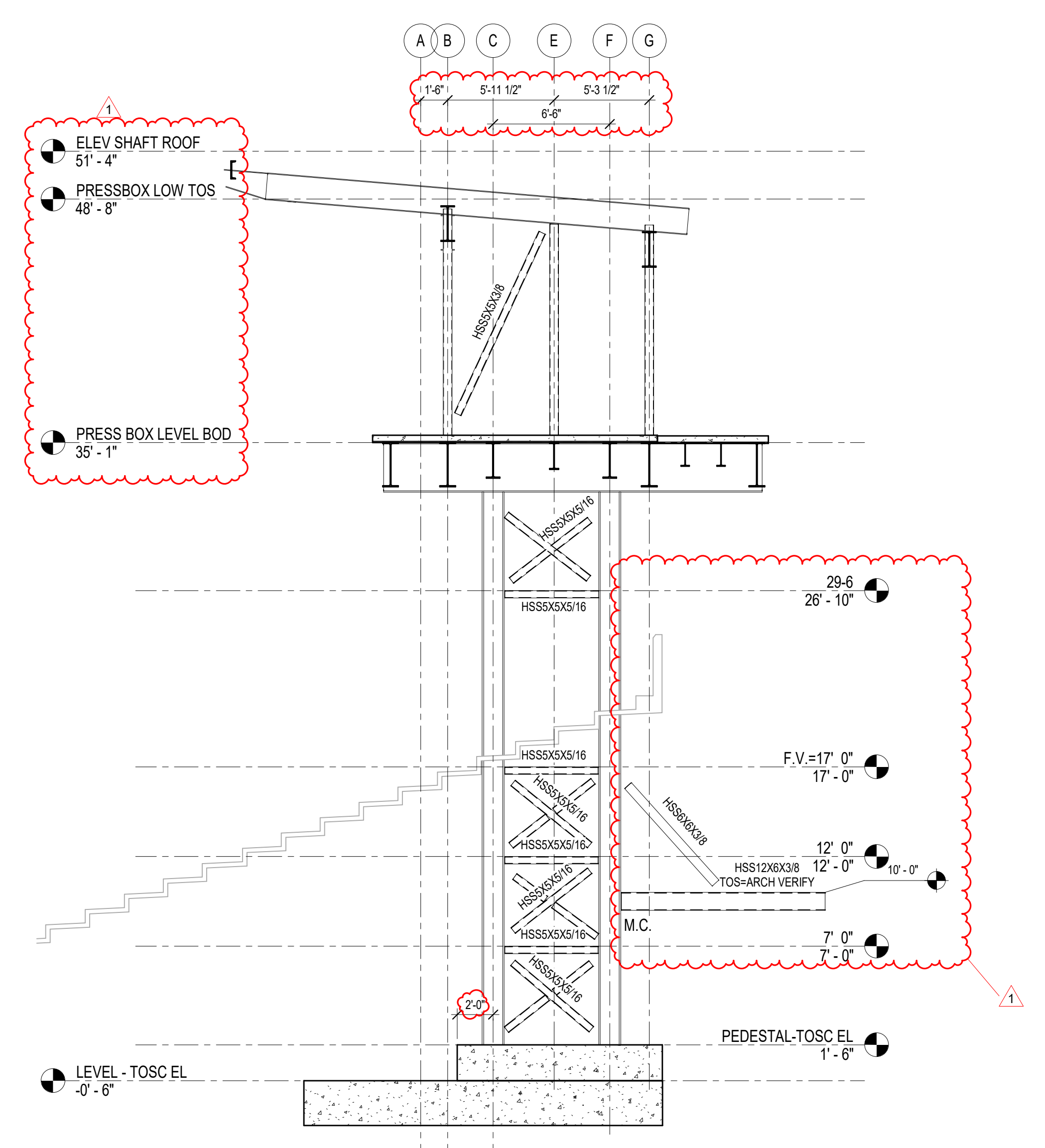
SECTION VIEW @ ELEVATOR @ 2ND FLOOR

3 TYPICAL WINDOW OR WALL HEAD BRACING AT BEAMS AT JOISTS
 NO SCALE

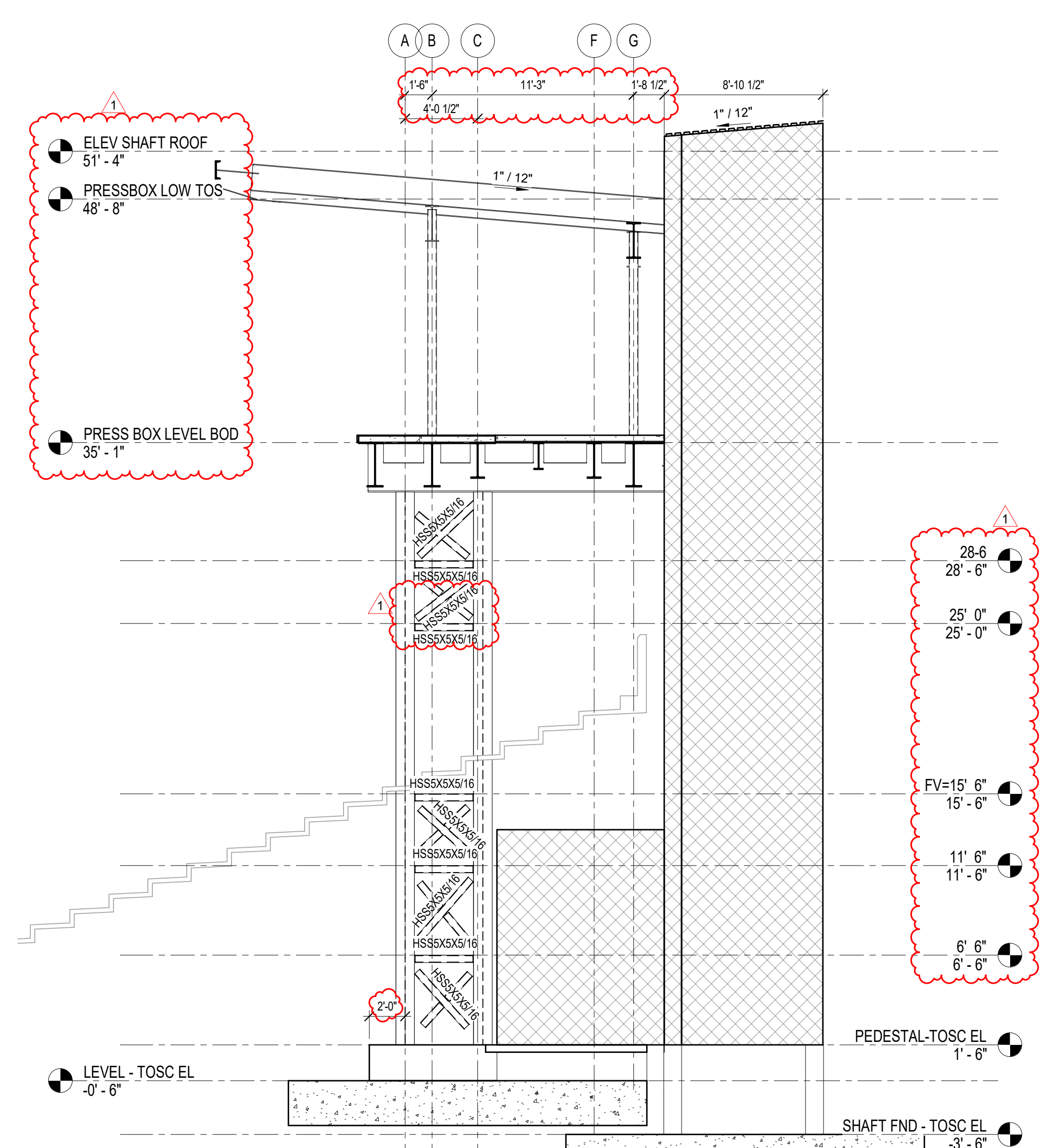
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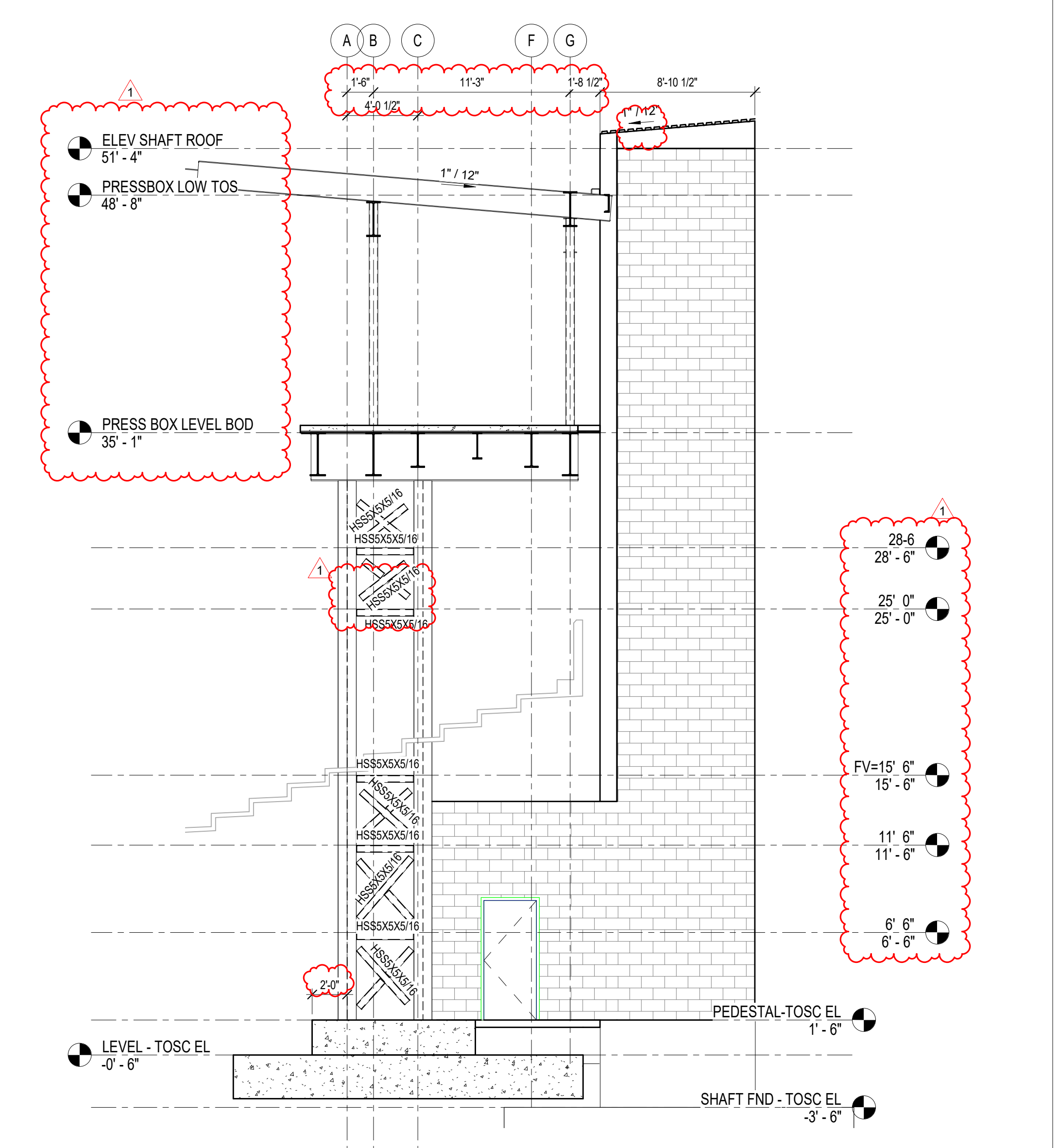
1 Elevation GL-12
3/16" = 1'-0"



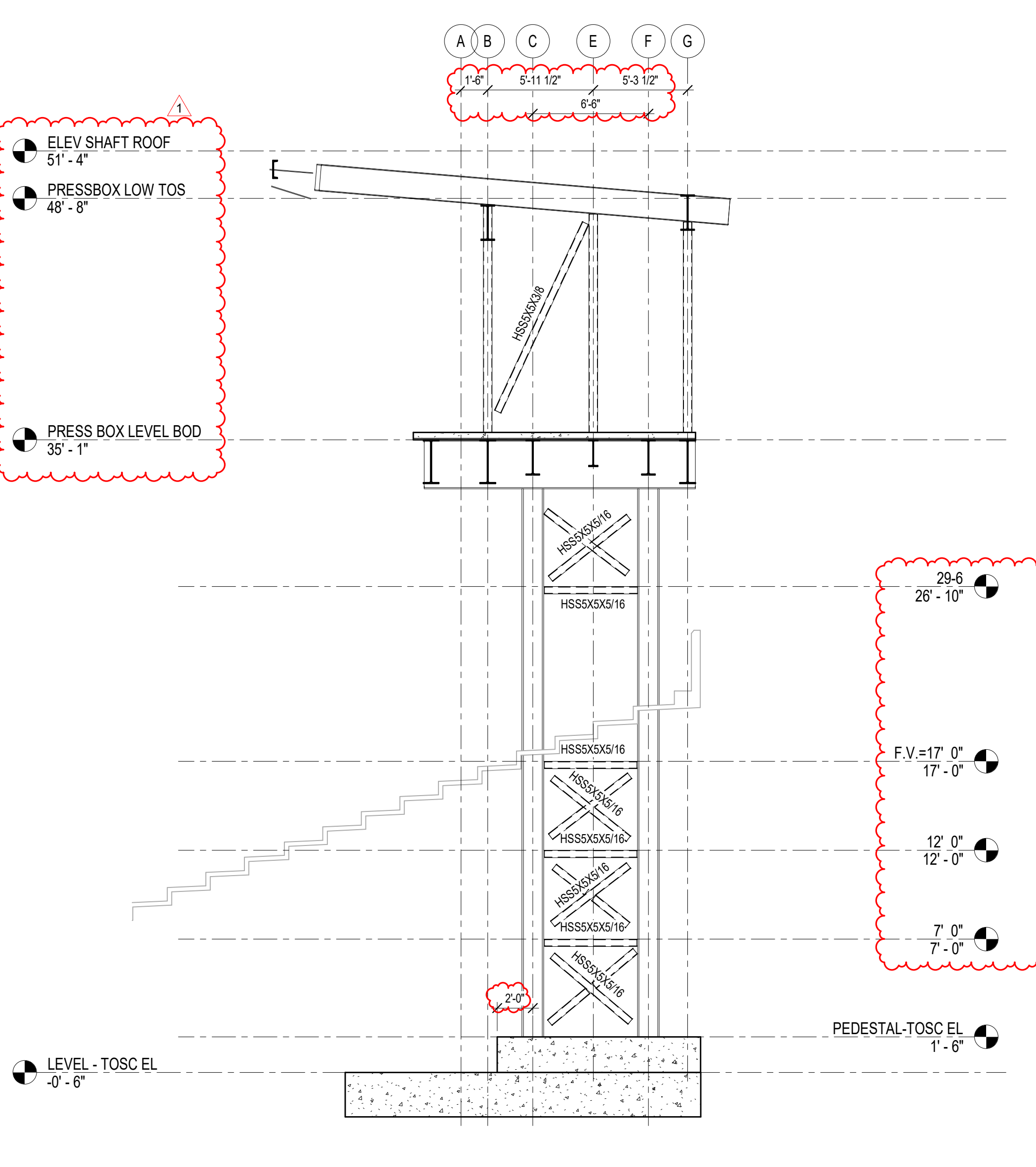
2 Elevation GL-11
3/16" = 1'-0"



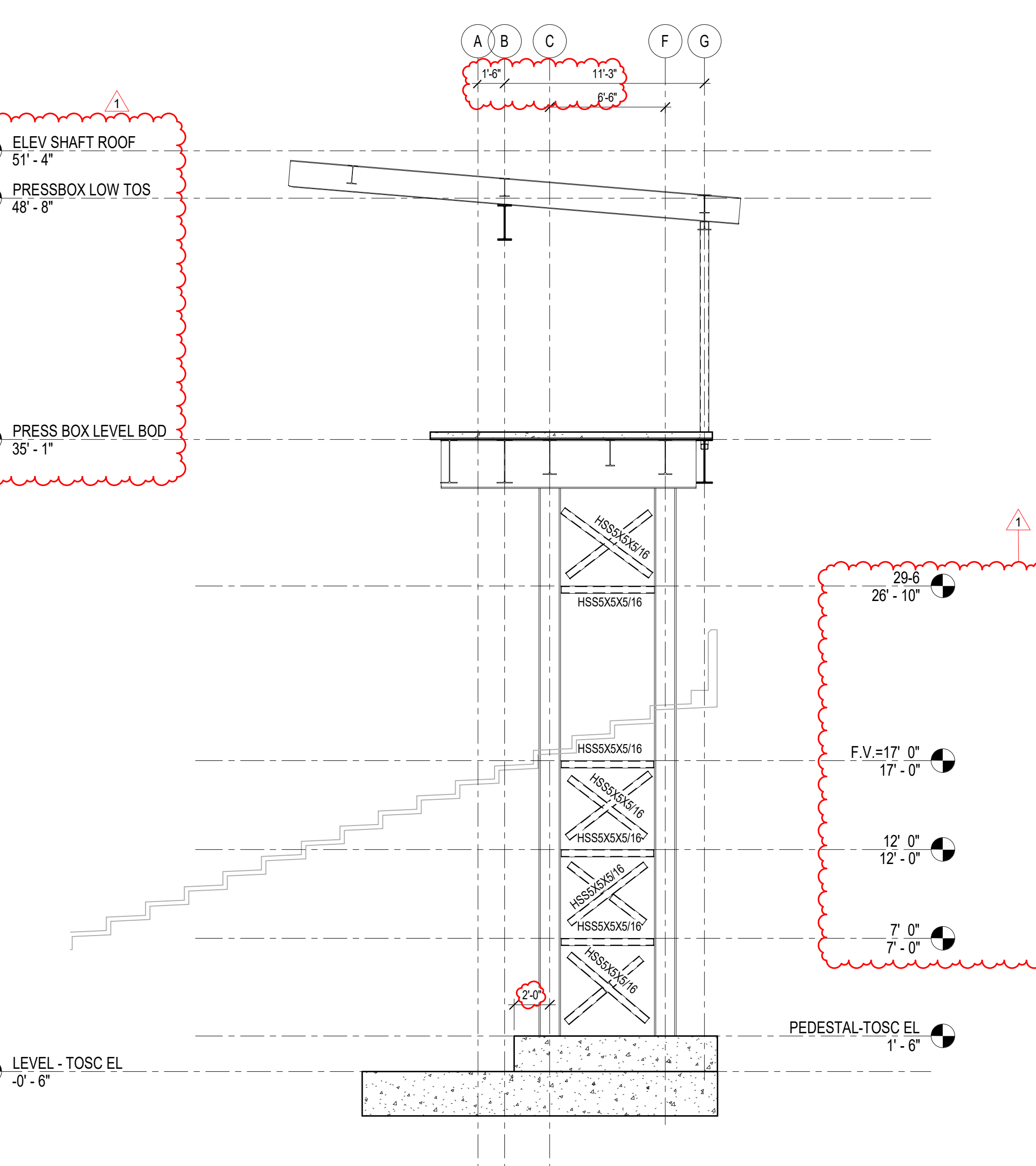
3 Elevation GL-10
3/16" = 1'-0"



4 Elevation GL-7
3/16" = 1'-0"



5 Elevation GL-5
3/16" = 1'-0"



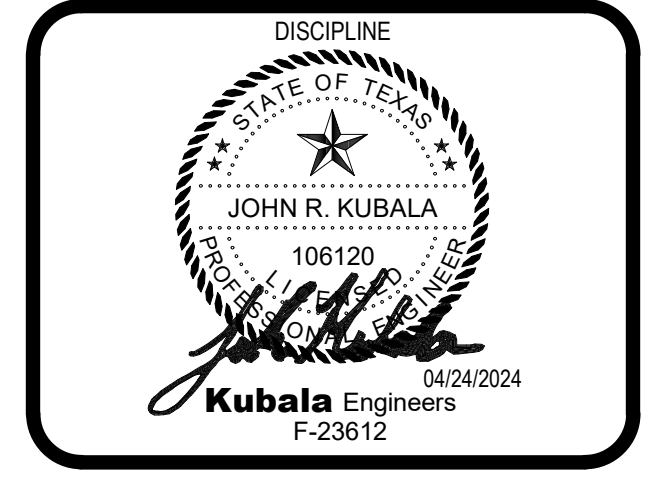
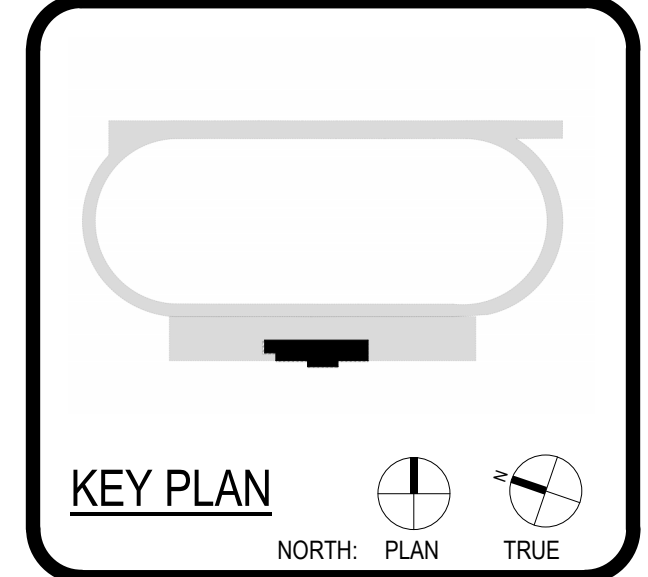
6 Elevation GL-4
3/16" = 1'-0"



ARCHITECT
HOUSTON
11 Greenway Plaza, 22nd Floor
Houston, TX 77046
713-965-0088 P
713-961-4571 F
TX Firm: 88-1688
REGISTERED ARCHITECTS
11/13/2009
LEAF ENGINEERS
1112-000-3303

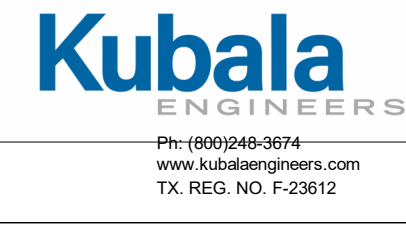
GALVESTON ISD
KERMIT COURVILLE STADIUM
NEW PRESS BOX

1429 27TH ST.
GALVESTON, TX, 77550
ISSUE FOR PROPOSAL



CLIENT GALVESTON ISD		
DATE 01/31/2024	PROJECT NUMBER 230063	
DRAWING HISTORY		
No.	Description	Date
1	AS1#1	04/24/2024

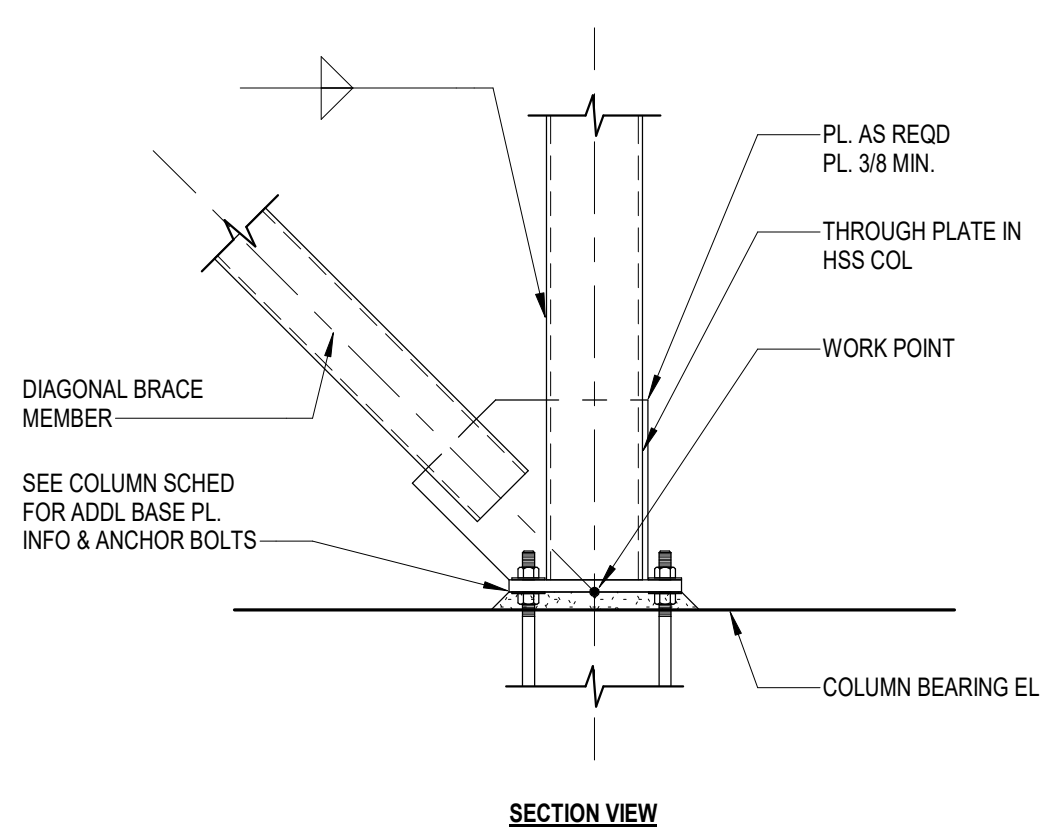
ISSUE FOR PROPOSAL
BUILDING NUMBER
TYPICAL WIND BRACING ELEVATIONS



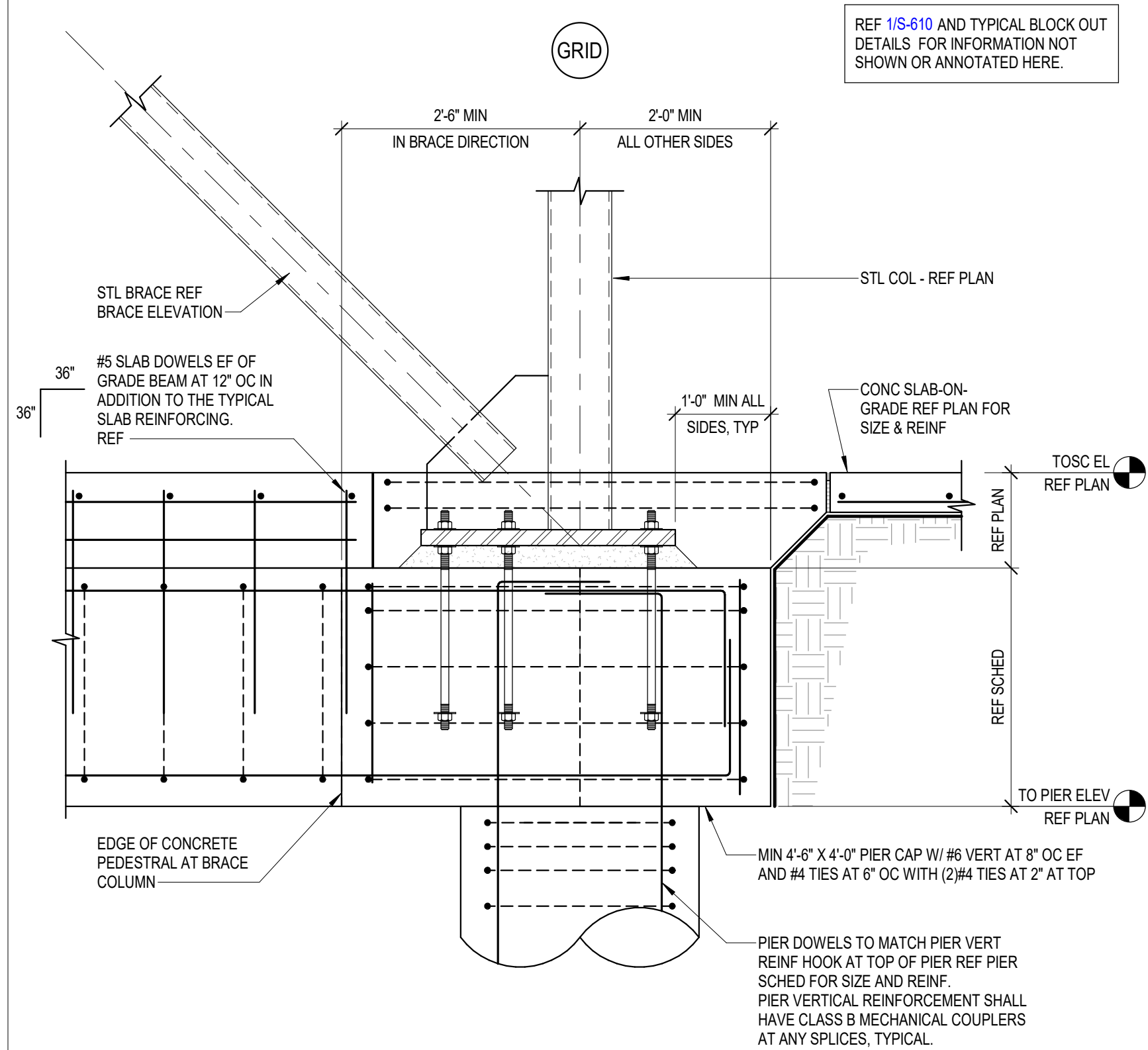
S-600

VERTICAL BRACE NOTES:

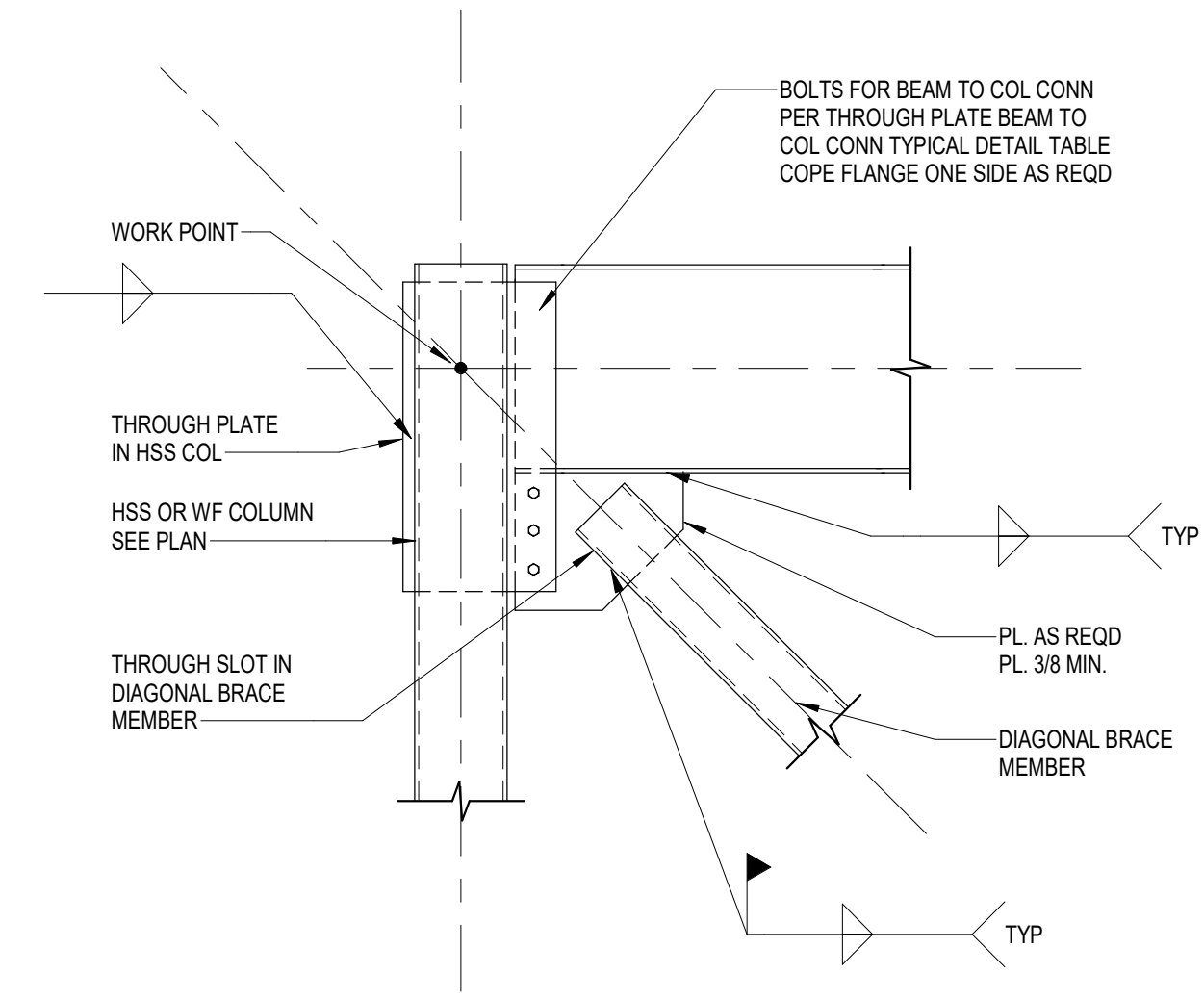
1. VERTICAL BRACE CONNECTIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS WORKING FOR THE FABRICATOR. SUBMIT CALCULATIONS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. MEMBER CONNECTIONS TO BE DESIGNED FOR FORCES (KIPS) SHOWN. FORCES INDICATED ARE UNFACTORED ACCORDING TO ALLOWABLE STRESS DESIGN METHOD (ASD). WHERE FORCES ARE NOT GIVEN, DESIGN FOR THE TENSILE CAPACITY OF THE DIAGONAL MEMBERS.
3. CONNECTIONS SHALL BE DESIGNED AND DETAILED TO TRANSFER FORCES BETWEEN MEMBERS WITHOUT ECCENTRICITY.
4. CONNECTIONS SHALL BE DESIGNED AND DETAILED FOR BOTH TENSION AND COMPRESSION PER FORCES PROVIDED.
5. FILLET WELD CONNECTIONS SHALL COMPLY WITH REQUIREMENTS NOTED IN AWS D1.1



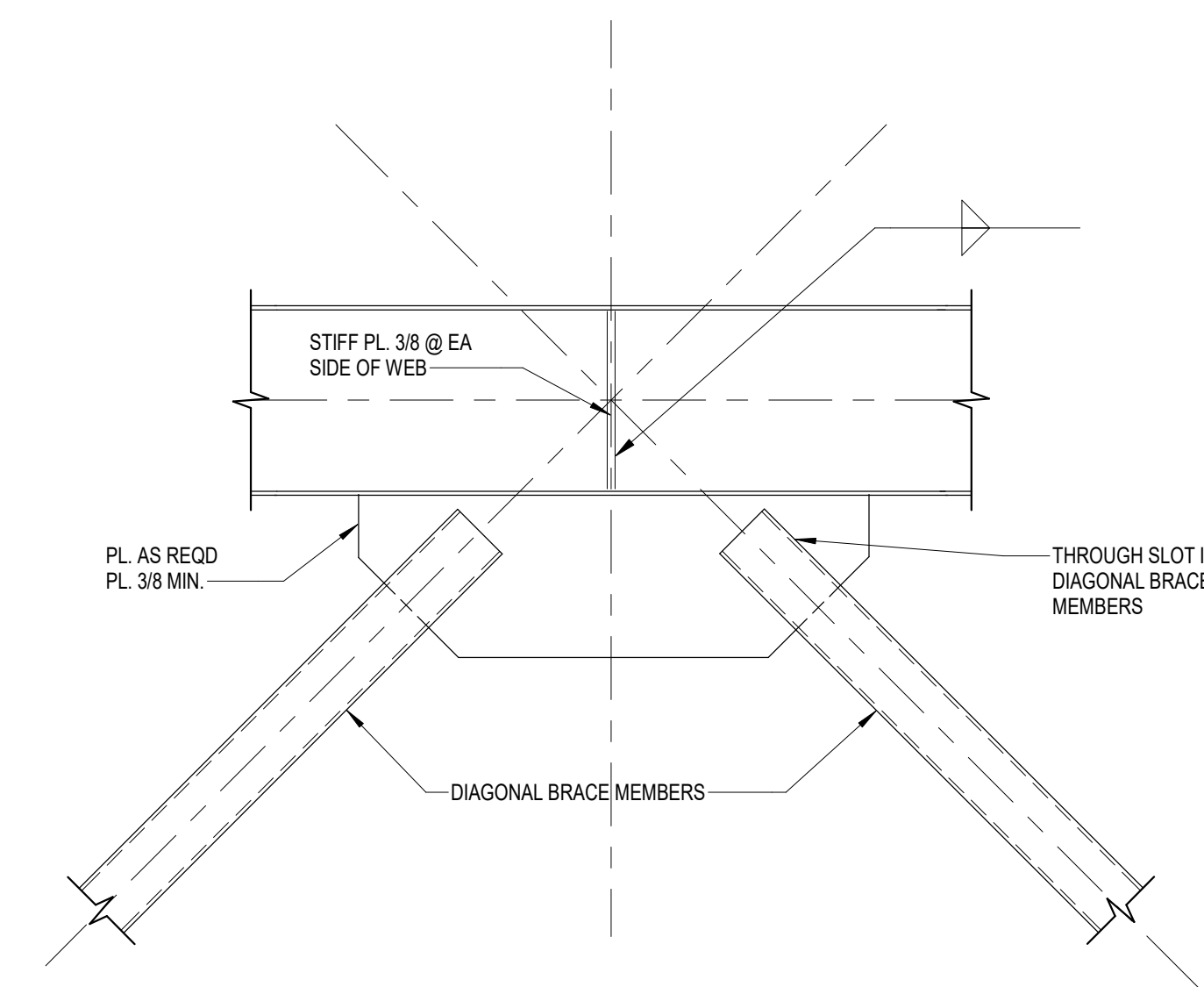
1 TYPICAL DETAIL OF VERTICAL BRACE CONNECTION
NO SCALE



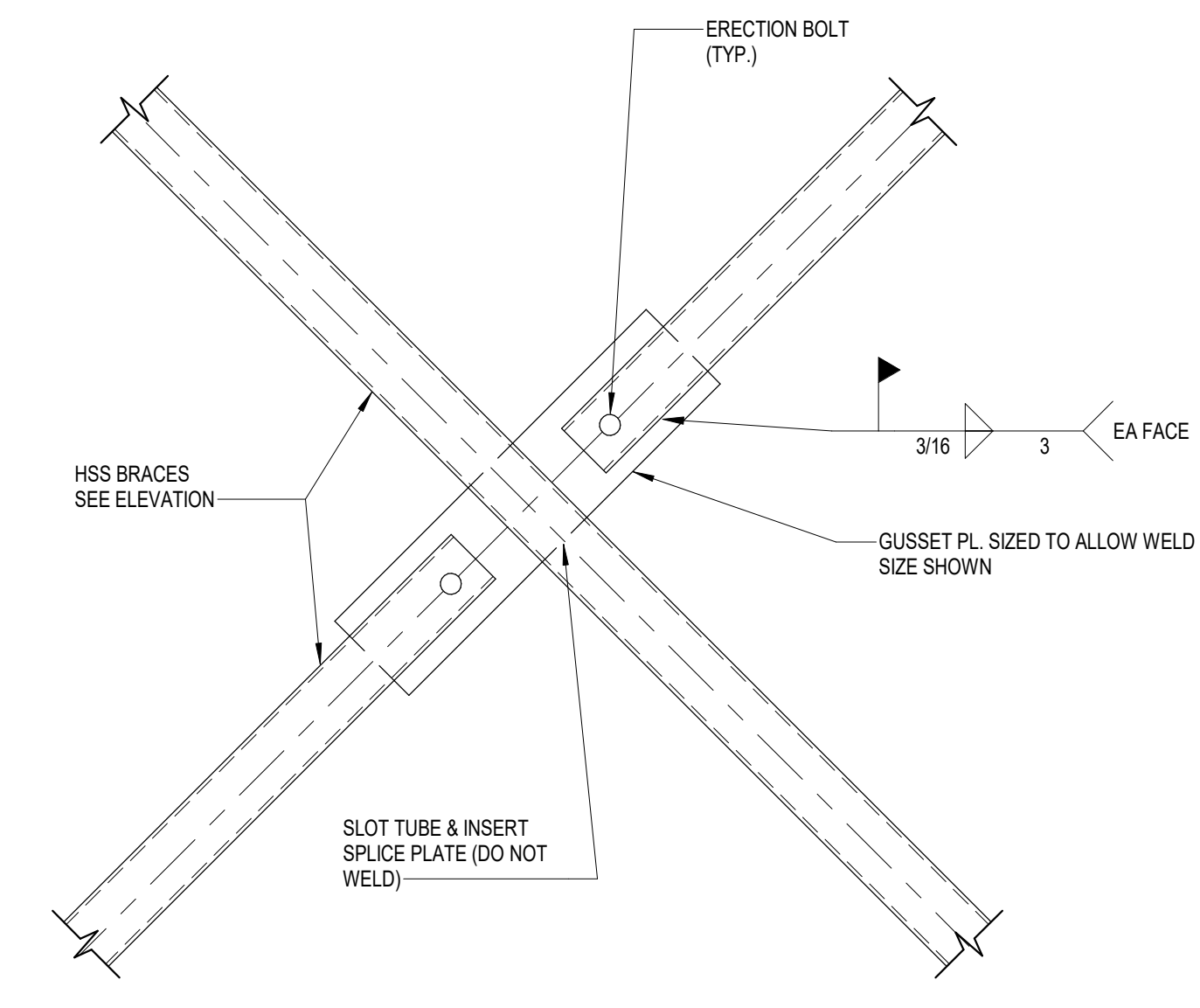
2 TYPICAL DETAIL OF VERTICAL BRACE CONNECTION
NO SCALE



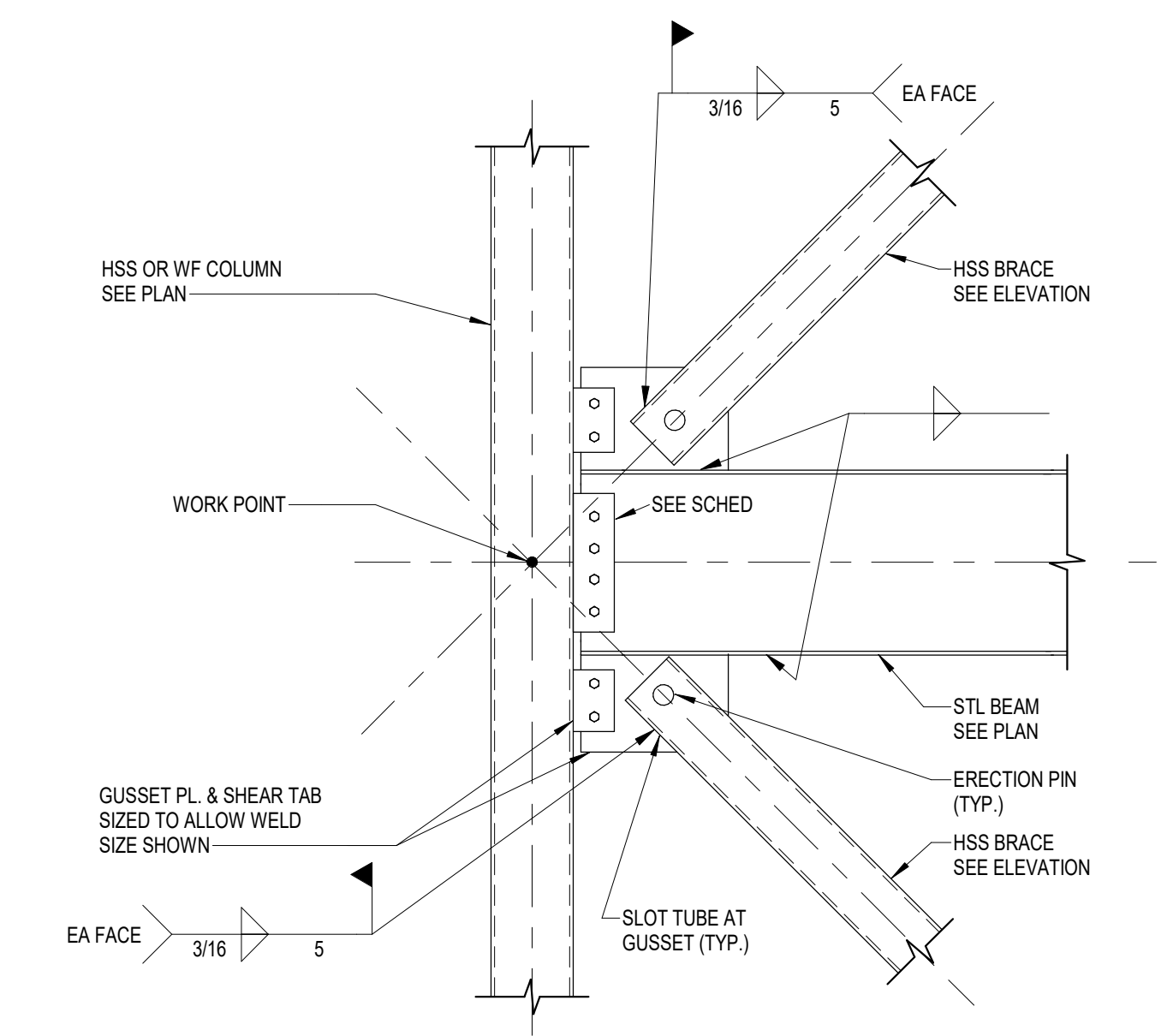
3 TYPICAL DETAIL OF VERTICAL BRACE CONNECTION
NO SCALE



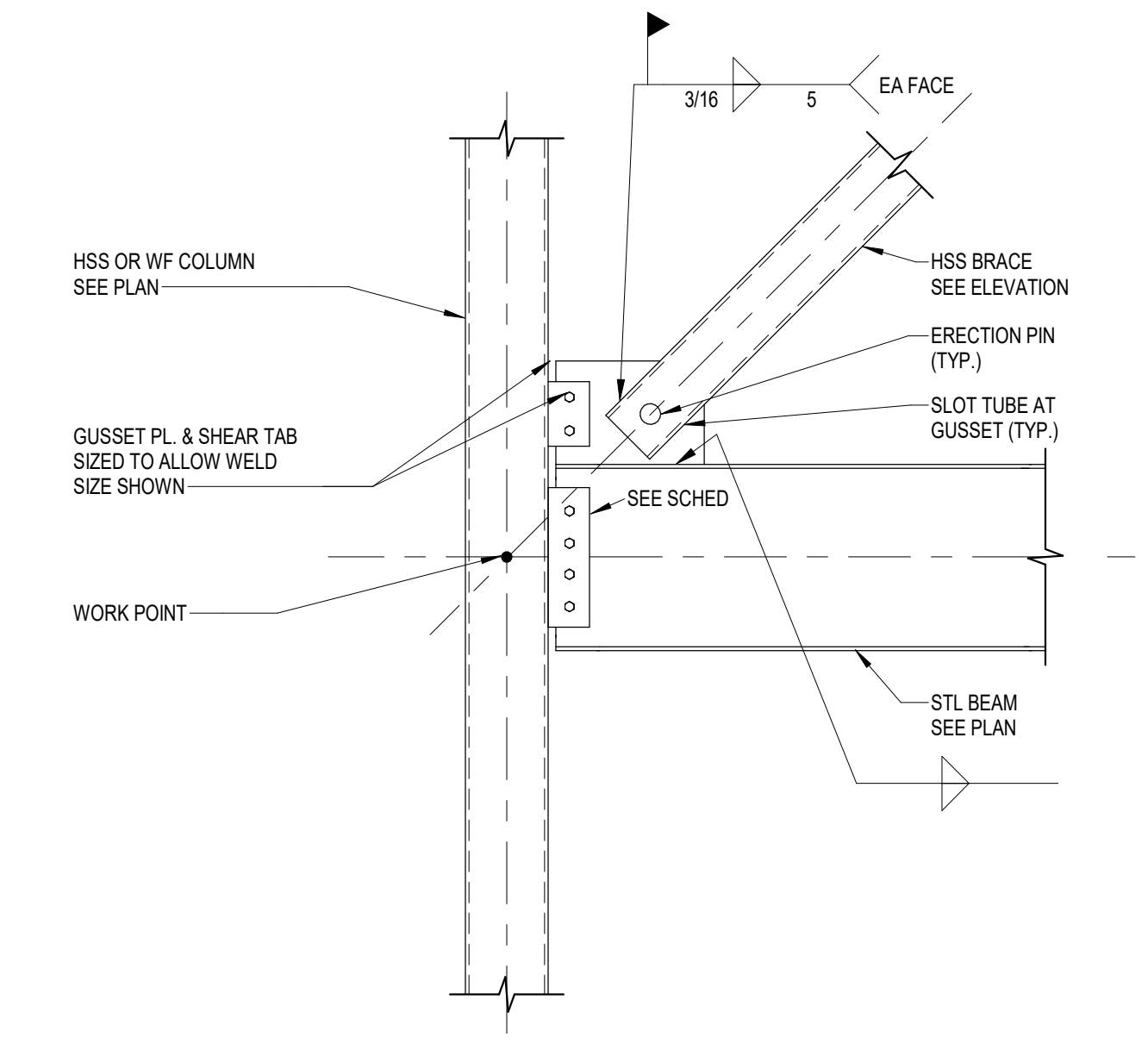
4 TYPICAL DETAIL OF VERTICAL BRACE CONNECTION
NO SCALE



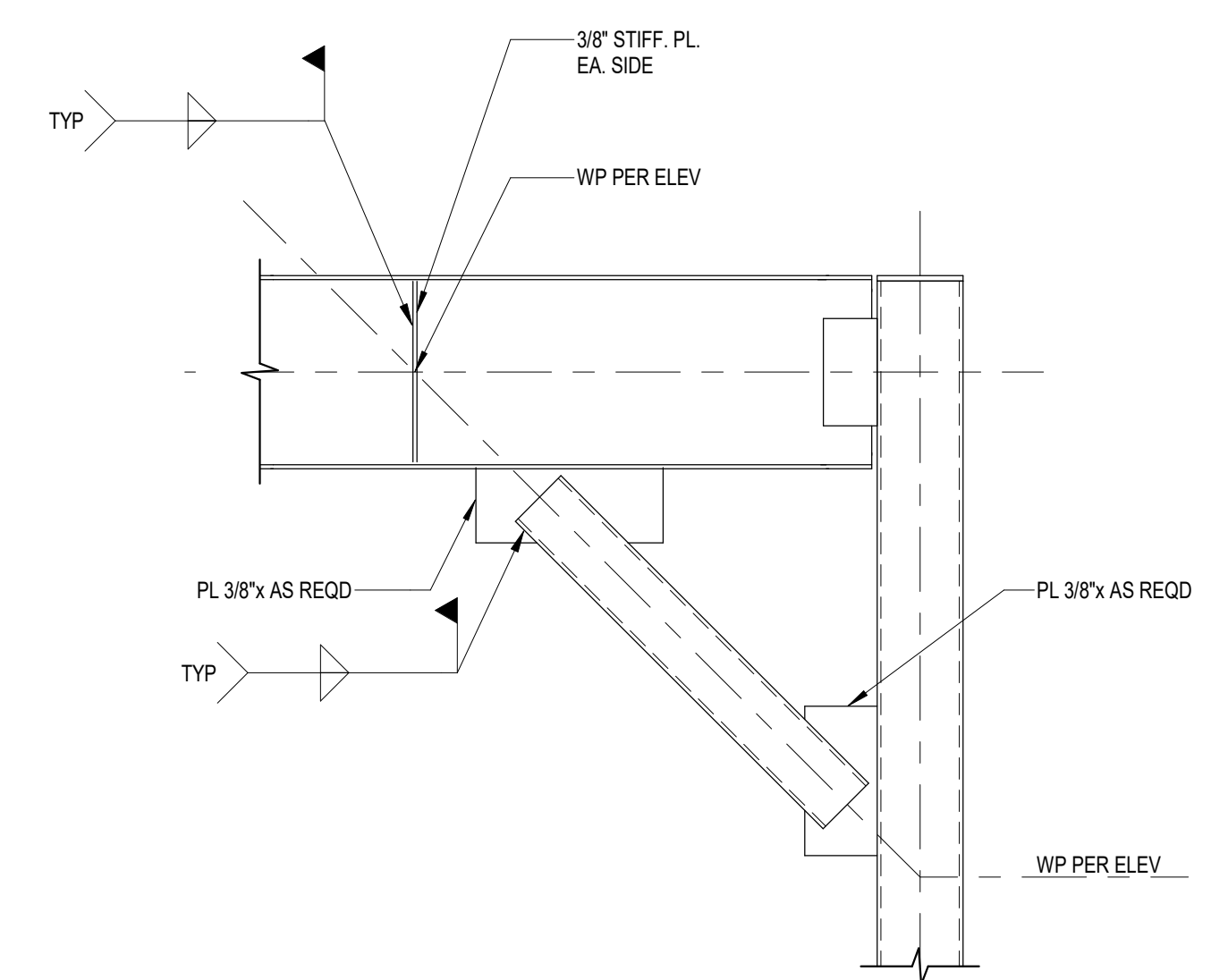
5 TYPICAL DETAIL OF TUBE X-BRACE
NO SCALE



6 TYPICAL DETAIL OF TUBE K-BRACE AT WIDE FLANGE BEAM
NO SCALE



7 TYPICAL DETAIL OF TUBE K-BRACE AT WIDE FLANGE BEAM
NO SCALE

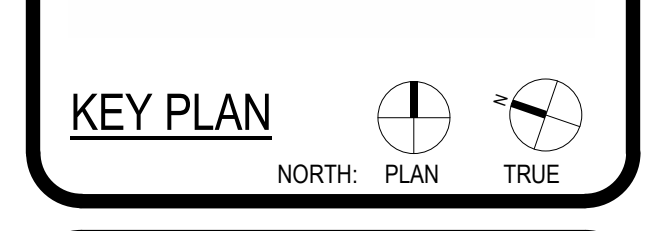
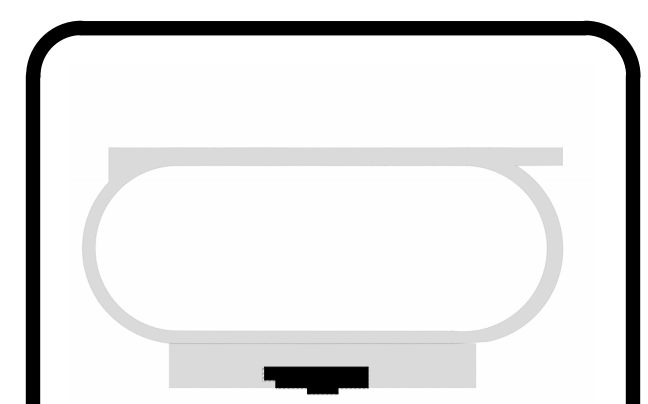


8 TYPICAL DETAIL OF VERTICAL BRACE CONNECTION
NO SCALE



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HOUSTON
11 Greenway Plaza, 22nd Floor
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713-961-4571 F
TX Firm: BR 1608
PBK Architects, Inc.
P.E. ENGINEERS
113-000-9933
LEAF ENGINEERS
113-000-3305

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GALVESTON ISD
DATE
01/31/2024
PROJECT NUMBER
230063
DRAWING HISTORY
No. Description Date
1 ASI #1 04/24/2024

ISSUE FOR PROPOSAL
BUILDING NUMBER

TYPICAL WIND BRACING DETAILS

S-610

