ALSEA GYM SEISMIC RETROFIT

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

ALSEA SCHOOL DISTRICT
301 S. 3RD ST.
ALSEA, OR 97324

PROJECT NOTES

1. ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE PROJECT TEAM OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.
2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS. RESPONSIBILITY SHALL INCLUDE BUT NOT LIMITED TO DEMOLITION AND CONSTRUCTION MEANS AND METHODS, TECHNIQUES, SEQUENCING, AND SAFETY REQUIRED TO COMPLETE CONSTRUCTION.
3. BEFORE STARTING A SECTION OF WORK THE CONTRACTOR SHALL CAREFULLY EXAMINE PREPARATORY WORK THAT HAS BEEN EXECUTED. ENSURE THAT WORK AND ADJACENT RELATED WORK WILL FINISH TO PROPER PLANES AND LEVELS.
4. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES.
5. CONTRACTOR IS RESPONSIBLE FOR CHECKING ALL CONTRACT DOCUMENTS, FIELD CONDITIONS, AND DIMENSIONS FOR ACCURACY AND CONFIRMING THAT WORK IS BUILDABLE AS SHOWN BEFORE PROCEEDING WITH THE CONSTRUCTION. IF THERE ARE ANY QUESTIONS, THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING A CLARIFICATION FROM THE PROJECT TEAM BEFORE PROCEEDING WITH THE WORK IN QUESTION OR RELATE WORK.
6. THE CONTRACTOR SHALL NOT SCALE DRAWINGS. WRITTEN DIMENSIONS SHALL ALWAYS GOVERN. CONTRACTOR REQUIRING DIMENSIONS NOT NOTED SHALL ALWAYS CONTACT THE PROJECT TEAM FOR SUCH INFORMATION PRIOR TO PRECEDING WITH WORK RELATED TO THOSE DIMENSIONS
7. THE CONTRACTOR SHALL PROTECT, PATCH, AND REPAIR TO MATCH ANY WALLS, FLOORS, CEILINGS, AND/OR OTHER SURFACES WHICH MAY BE DISTURBED DURING THE INSTALLATION OF MECHANICAL, ELECTRICAL, PLUMBING OR OTHER OWNER WORK.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CUTTING AND PATCHING REQUIRED FOR PROPER INSTALLATION OF MATERIAL AND EQUIPMENT. PROVIDE DEMOLITION AND PATCH/REPAIR IN ALL AREAS (WHETHER SPECIFICALLY SHOWN OR NOT) TO ACCOMMODATE ALL WORK.
9. IF THE CONTRACTOR ENCOUNTERS A CONDITION NOT COVERED IN THE CONSTRUCTION DOCUMENTS, THE CONTRACTOR SHALL NOTIFY AND RESOLVE THE ISSUE WITH THE PROJECT TEAM BEFORE COMMENCING ANY WORK.
10. ALL PERMITS ASSOCIATED WITH THE PROJECT SHALL BE PAID AND OBTAINED BY THE CONTRACTOR.
11. DIMENSIONS ARE TO FACE OF FRAMING UNLESS OTHERWISE NOTED.
12. GENERAL CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR JOB CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF PERSONS AND PROPERTY AND COMPLIANCE WITH OSHA SAFETY STANDARDS.
13. WHEN PORTIONS OF THE WORK ARE PERFORMED BY THE CONTRACTOR ON A DESIGN-BUILD BASIS, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE DESIGN OF SUCH SYSTEMS AND FOR THE SECURING OF ALL ASSOCIATED PERMITS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF ALL DESIGN BUILD SUB CONTRACTORS .
14. CONTRACTOR SHALL AVOID INTERFERENCE AND CONFLICT WITH THE BUILDING'S NORMAL OPERATION. CONTRACTOR TO COMPLY WITH THE BUILDING RULES AND REGULATIONS REGARDING SCHEDULING AND USE OF ELEVATORS AND LOADING DOCKS FOR DELIVERY AND HANDLING OF MATERIALS, EQUIPMENT, AND DEBRIS.
15. ALL KEY NOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE
16. CONTRACTOR SHALL BE RESPONSIBLE FOR ASHREA COMPLIANCE IN SCHOOL, RETAIL, AND OFFICE SPACES

ABBREVIATIONS

A.B.	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
A.C.T.	ACOUSTICAL CEILING TILE
A.F.F.	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ARCH.	ARCHITECT
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BD.	BOARD
BITUM.	BITUMINOUS
BOT./B.O.	BOTTOM/BOTTOM OF
B.O.D.	BASIS OF DESIGN
CER.	CERAMIC
C.G.	CORNER GUARD
C.I.	CONTINUOUS INSULATION
C.J.	CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLR.	CLEAR
CMU	CONCRETE MASONRY UNIT
C.O.	CASED OPENING
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
CORR.	CORRIDOR
CPT.	CARPET
C.T.	CERAMIC TILE
DBA	DEFORMED BAR ANCHOR
D.F.	DRINKING FOUNTAIN
DIA., ⌀	DIAMETER
DR.	DOOR
D.S.	DOWNSPOUT
(E)	EXISTING
ELEC.	ELECTRICAL
EQ.	EQUAL
EXT.	EXTERIOR
F.A.	FIRE ALARM
F.D.	FLOOR DRAIN
FDN.	FOUNDATION
F.O.F	FACE OF FINISH
F.O.S.	FACE OF STUDS
F.S.D.	FIRE SEPARATION DISTANCE
FTG.	FOOTING
Ga.	GAUGE
GALV.	GALVANIZED
GL	GRID LINE
GLB	GLULAM BEAM
G.B.	GRAB BAR
GND.	GROUND
GSM	GALVANISED SHEET METAL
GYP.	GYPSUM
H.B.	HOSE BIBB
H.C.	HOLLOW CORE
H.M.	HOLLOW METAL
HORIZ.	HORIZONTAL
HPC	HIGH PERFORMANCE COATING
HSS	HOLLOW STRUCTURAL STEEL
IBC	INTERNATIONAL BUILDING CODE
IN.	INCH
INT.	INTERIOR
K	KIPS
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
LAM.	LAMINATE
LB.	POUND
L.L.	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LOC.	LOCATION
LONG.	LONGITUDINAL
L.P.	LOW POINT
LVF	LOW VELOCITY FASTENER
MAX.	MAXIMUM
MBMA	METAL BUILDING MANUFACTURERS ASSOCIATION
M.E.	MATCH EXISTING
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MTD.	MOUNTED
MUL.	MULLION
(N)	NEW
N.I.C.	NOT IN CONTRACT
NOM.	NOMINAL
N.T.S.	NOT TO SCALE
OBS.	OBSCURE
o.c.	ON CENTER
O.F.C.I.	OWNER FURNISHED CONTRACTOR INSTALLED
O.F.D.	OVERFLOW DRAIN
O.F.O.I.	OWNER FURNISHED OWNER INSTALLED
OH.	OPPOSITE HAND
OHD	OVERHEAD DOOR
OPP.	OPPOSITE
OWJ	OPEN WEB JOIST
PAF	POWDER ACTUATED FASTENER
P/C.	PRECAST (CONCRETE)
PCF	POUNDS PER CUBIC FOOT
PL	PLATE
P.LAM.	PLASTIC LAMINATE
PLAS.	PLASTER
PLY	PLYWOOD
PJP	PARTIAL JOINT PENETRATION
PSF	POUNDS PER FOOT
PSI	POUNDS PER INCH
P/T	POST-TENSIONED
P.T.	PRESSURE TREATED
R.C.P.	REFLECTED CEILING PLAN
R.D.	ROOF DRAIN
REF.	REFERENCE
REINF.	REINFORCING
REQ'D.	REQUIRED
RL.	RELOCATE
R.O.	ROUGH OPENING
RWD.	REDWOOD
REV.	REVERSED
S.A.M.	SELF-ADHERED MEMBRANE
S.C.	SOLID CORE
SCHED.	SCHEDULE
SIM.	SIMILAR
SLRS	SEISMIC LOAD RESISTING SYSTEM
S.O.G.	SLAB ON GRADE
SPEC.	SPECIFICATION
SQ.	SQUARE
S.S.	STAINLESS STEEL
SSMA	STEEL STUD MANUFACTURERS ASSOCIATION
STD.	STANDARD
STRUCT.	STRUCTURAL
S.T.S.	SELF TAPPING SCREW\
SYM.	SYMMETRICAL
THRU	THROUGH
TYP.	TYPICAL
T & G	TONGUE AND GROOVE
THK.	THICK
TJ	TRUSS JOIST
TRANS.	TRANSVERSE
T.W.	TOP OF WALL
U.N.O.	UNLESS NOTED OTHERWISE
U.T.	ULTRASONIC TESTING
VERT.	VERTICAL
V.I.F.	VERIFY IN FIELD
w/	WITH
w/o	WITHOUT
WF.	WIDE FLANGE
W.P.	WORK POINT

PROJECT NARRATIVE

PROJECT SCOPE:
THE SCOPE OF THIS PROJECT CONSISTS OF THE SEISMIC REHABILITATION OF ALSEA MIDDLE/HIGH SCHOOL GYMNASIUM AND OFFICE ADDITION. THE REHABILITATION WILL BE ACCOMPLISHED BY MEANS OF ADDING LATERAL FORCE RESISTING ELEMENTS WHERE NECESSARY, BY ENSURING A PROPER LOAD PATH TO THE FOUNDATION ELEMENT. IN CONJUNCTION WITH THE LATERAL SYSTEM, THE GYM DIAPHRAGM WILL BE STRENGTHENED AND NEW VERTICAL LATERAL FORCE RESISTING ELEMENTS WILL BE CONSTRUCTED.

SEISMIC REHABILITATION SCOPE:
THE SEISMIC REHABILITATION PORTION OF THE PROJECT IS FUNDED BY A GRANT AWARDED TO ALSEA SCHOOL DISTRICT BY THE INFRASTRUCTURE FINANCE AUTHORITY OREGON BUSINESS DEVELOPMENT DEPARTMENT TO MEET THE BASIC PERFORMANCE OBJECTIVE FOR EXISTING BUILDINGS (BPOE) AS OUTLINED IN THE AMERICAN SOCIETY OF CIVIL ENGINEERS:
"SEISMIC REHABILITATION OF EXISTING BUILDINGS" (ASCE 41-17). THE REHABILITATION OBJECTIVE IS DEFINED AS STRUCTURAL PERFORMANCE LEVEL, IMMEDIATE OCCUPANCY, MEANS THE POST-EARTHQUAKE DAMAGE STATE IN WHICH ONLY VERY LIMITED STRUCTURAL DAMAGE HAS OCCURRED. THE BASIC VERTICAL- AND LATERAL-FORCE-RESISTING SYSTEMS OF THE BUILDING RETAIN NEARLY ALL OF THEIR PRE-EARTHQUAKE STRENGTH AND STIFFNESS. THE RISK OF LIFE THREATENING INJURY AS A RESULT OF STRUCTURAL DAMAGE IS VERY LOW, AND ALTHOUGH SOME MINOR STRUCTURAL REPAIRS MAY BE APPROPRIATE, THESE WOULD GENERALLY NOT BE REQUIRED PRIOR TO REOCCUPANCY."

THE CONTRACTOR IS RESPONSIBLE FOR ALL ASPECTS OF CONSTRUCTION NECESSARY FOR THE COMPLETION OF ALL WORK INCIDENTAL TO THE WORK ILLUSTRATED IN THIS PLAN SET.

PROJECT COMPONENTS:

THE FOLLOWING LIST IS A BRIEF GENERAL DESCRIPTION OF WHAT THIS PROJECT ENTAILS:

1. NEW PLYWOOD SHEATHING OVER (E) SHEATHING.
2. NEW IN-PLANE AND OUT-OF-PLANE CONNECTIONS FROM DIAPHRAGM TO LATERAL FORCE RESISTING ELEMENTS.
3. (E) UNREINFORCED CMU WALLS TO BE BRACED FOR OUT-OF-PLANE SEISMIC FORCES.
4. INFILL OF (E) CONCRETE POST AND BEAM LINES WITH NEW REINFORCED CONCRETE SHEAR WALLS.
5. NEW SHOTCRETE CONCRETE SHEAR WALLS AT STAGE.

INCIDENTAL TO THE SEISMIC REHABILITATION:

1. NEW STANDING SEAM METAL ROOFING, GUTTERS AND DOWNSPOUTS.
2. REMOVAL AND RE-APPLICATION OF FINISHES REQUIRED FOR INSTALLATION OF SEISMIC REHABILITATION STRUCTURAL COMPONENTS.
3. REPAIR OR REPLACEMENT OF EXISTING DAMAGED MATERIALS OR FINISHES.
4. PAINT PORTIONS OF BUILDING EFFECTED BY REHABILITATION.
5. ASBESTOS ABATEMENT MAY BE NECESSARY PER THE ABATEMENT STUDY REPORT.

DESIGN-BUILD NARRATIVE

SUBMITTALS TO BE PROVIDED BY GENERAL CONTRACTOR & M.E.P. SUB-CONTRACTORS TO THE AHJ FOR FINAL APPROVAL OF BUILDING PERMIT.

DESIGN BUILD SCOPE:
*FURTHER MODIFICATION MAY BE REQUIRED. G.C. & SUB-CONTRACTORS TO VERIFY EXISTING CONDITIONS & PROVIDE CODE REQUIRED MEP DESIGN TO MODIFIED AREAS OF WORK.

MECHANICAL:
1. MODIFCATIONS TO THE (E) SYSTEM AS REQUIRED TO ACCOMADATE ALTERATIONS

ELECTRICAL:
1. PROVIDE LIGHTING AS INDICATED..
2. REMOVAL AND REINSTALL (E) DATA PANELS IN STAFF ROOM.

PLUMBING:
1. MODIFCATIONS TO (E) SHOWER HEADS AND CONTROLS IN LOCKER ROOM AS REQUIRED TO ACCOMADATE ALTERATIONS.
2. REMOVAL AND REINSTALL OF (E) FIXTURES IN STAFF RESTROOM.
3. REMOVAL AND REINSTALL OF (E) FIXTURES IN RESTROOMS ON BOTH SIDES OF STAGE.

PROJECT TEAM

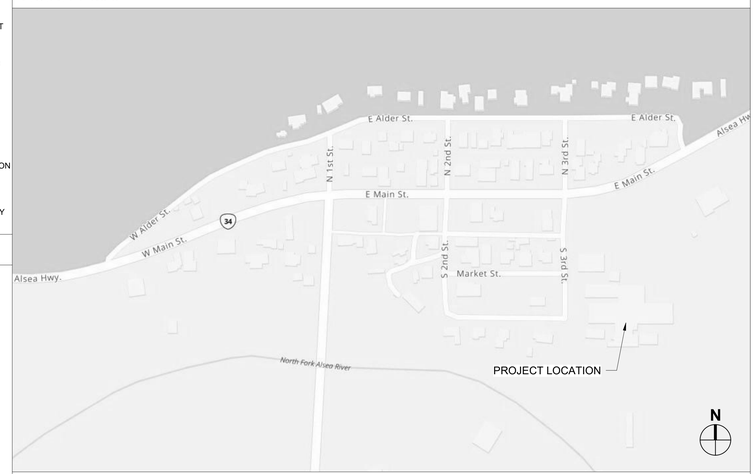
OWNER:
CONTACT: *KRISTA NIERAETH, SUPERINTENDENT*
ALSEA SCHOOL DISTRICT
301 SOUTH 3RD ST.
ALSEA, OR 97324
T 541.487.4305

CONSTRUCTION MANAGEMENT:
CONTACT: *CHRIS GIGGY, OWNER*
INTEGRITY MANAGEMENT SOLUTIONS
18525 VAN HORN RD.
ALSEA, OR 97324
T 541.452.1919

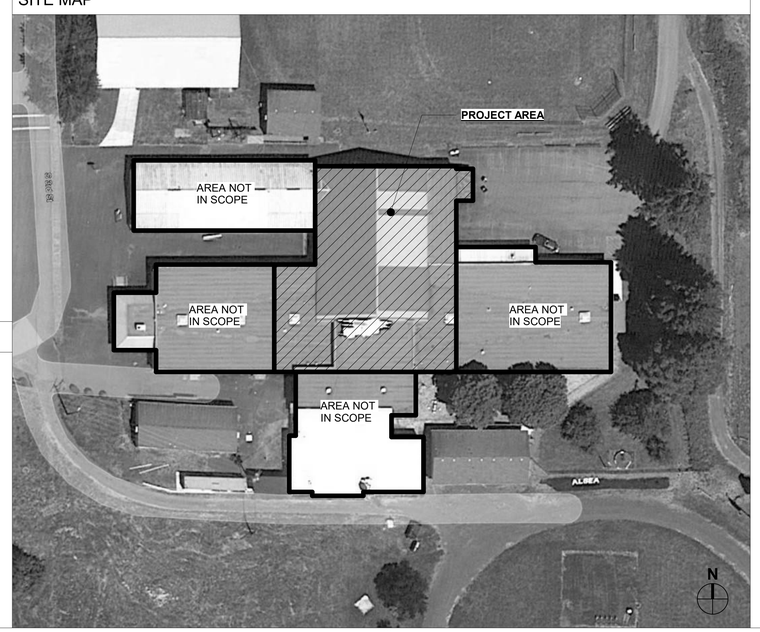
ARCHITECTURE:
CONTACT: *MEAGAN BAKER-WILMES, AIA*
STAMPING REGISTRANT: *MARLENE GILLIS, AIA, ALEP, LEED, AP, CCS*
SODERSTROM ARCHITECTS
1331 NW LOVEJOY ST. #775
PORTLAND, OR 97209
T 503.228.5617

STRUCTURE:
CONTACT: *KRIS TONNING, PE, SE*
STAMPING REGISTRANT: *MATTHEW R. SMITH, PE, SE*
ZCS ENGINEERING & ARCHITECTURE
524 MAIN ST.
OREGON CITY, OR 97045
T 503.659.2205

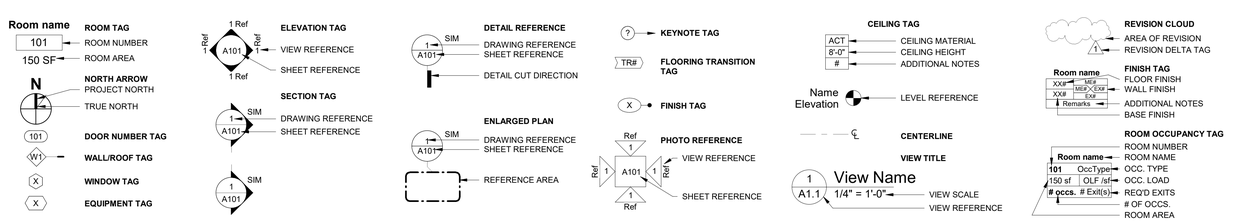
VICINITY MAP



SITE MAP



SYMBOL LEGEND



SHEET INDEX

ZCS
524 Main Street, Suite 2
Oregon City, OR 97045
503.659.2205

ALSEA SCHOOL DISTRICT
301 S. 3RD ST.
ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY- NOT FOR CONSTRUCTION

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

COVER SHEET

G0.00

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

DEMO PLAN LEGEND

- (E) ELEMENT TO REMAIN
- (E) ELEMENT TO BE DEMOLISHED
- (E) WALL TO BE DEMOLISHED
- (E) FLOOR FINISH TO BE DEMOLISHED

DEMO PLAN KEYNOTES

1. REMOVE AND STORE LAVATORY / URINAL FOR REINSTALLATION
2. REMOVE AND STORE PARTITIONS, PARTITION DOORS, AND ACCESSORIES FOR REINSTALLATION
3. REMOVE AND STORE WATER CLOSET FOR REINSTALLATION
4. REMOVE AND STORE LOCKERS FOR REINSTALLATION
5. REMOVE AND STORE GRABRAIL FOR REINSTALLATION
6. DEMOLITION PORTION OF (E) RAMP
7. REMOVE AND STORE SHOWER COMPARTMENT FOR REINSTALLATION
8. DEMO PORTION OF SLAB FOR RELOCATION OF WATER CLOSET
9. CONCRETE COLUMN CUTBACK TO ALLOW FOR SEISMIC JOINT (SEE STRUCT)
10. DEMO URM WALL
11. DEMO CEILING SLAB
12. DEMO CASEWORK
13. DEMO WALL FINISH

DEMO PLAN GENERAL NOTES

A. ALL EXISTING BUILDING COLUMNS, EXTERIOR WALLS AND STRUCTURAL MEMBERS TO REMAIN, U.N.O.

B. OBTAIN DEMOLITION PERMITS AND INCLUDE ALL COSTS OF SAME IN CONTRACT PRICE.

C. CONTRACTOR SHALL KEEP CONSTRUCTION AREA FREE OF DUST AND DEBRIS FOR THE DURATION OF CONSTRUCTION AND SHALL BE RESPONSIBLE FOR DISPOSAL OF ALL DEBRIS.

D. FURNISH ALL LABOR AND MATERIALS/EQUIPMENT TO COMPLETE DEMOLITION AND REMOVAL OF ALL ITEMS AS INDICATED. PATCH AND REPAIR INTERIOR SPACE AS REQUIRED UPON COMPLETION OF DEMOLITION. IF ANY QUESTIONS ARISE AS TO THE REMOVAL OF ANY MATERIAL, CLARIFY THE POINT IN QUESTION WITH PROJECT TEAM BEFORE PROCEEDING.

E. IN PARTITIONS TO BE REMOVED, REMOVE AND CAP ALL OUTLETS, SWITCHES, WIRES, THERMOSTATS, ETC., TO THEIR SOURCE.

F. REMOVE ALL ABANDONED ELECTRICAL CONDUIT, CABLING BACKBOARD AND EQUIPMENT, TYPICAL THROUGHOUT ENTIRE SPACE. AT COMPLETION OF DEMOLITION WORK, THE CONSTRUCTION AREA(S) SHALL BE LEFT IN "BROOM CLEAN" CONDITION. ALL DEBRIS AND MISCELLANEOUS MATERIAL SHALL BE REMOVED.

G. IN ADDITION TO SPECIFIC DEMOLITION SCOPE IDENTIFIED, PERFORM MISCELLANEOUS DEMOLITION AS REQUIRED TO SUPPORT NEW CONSTRUCTION.

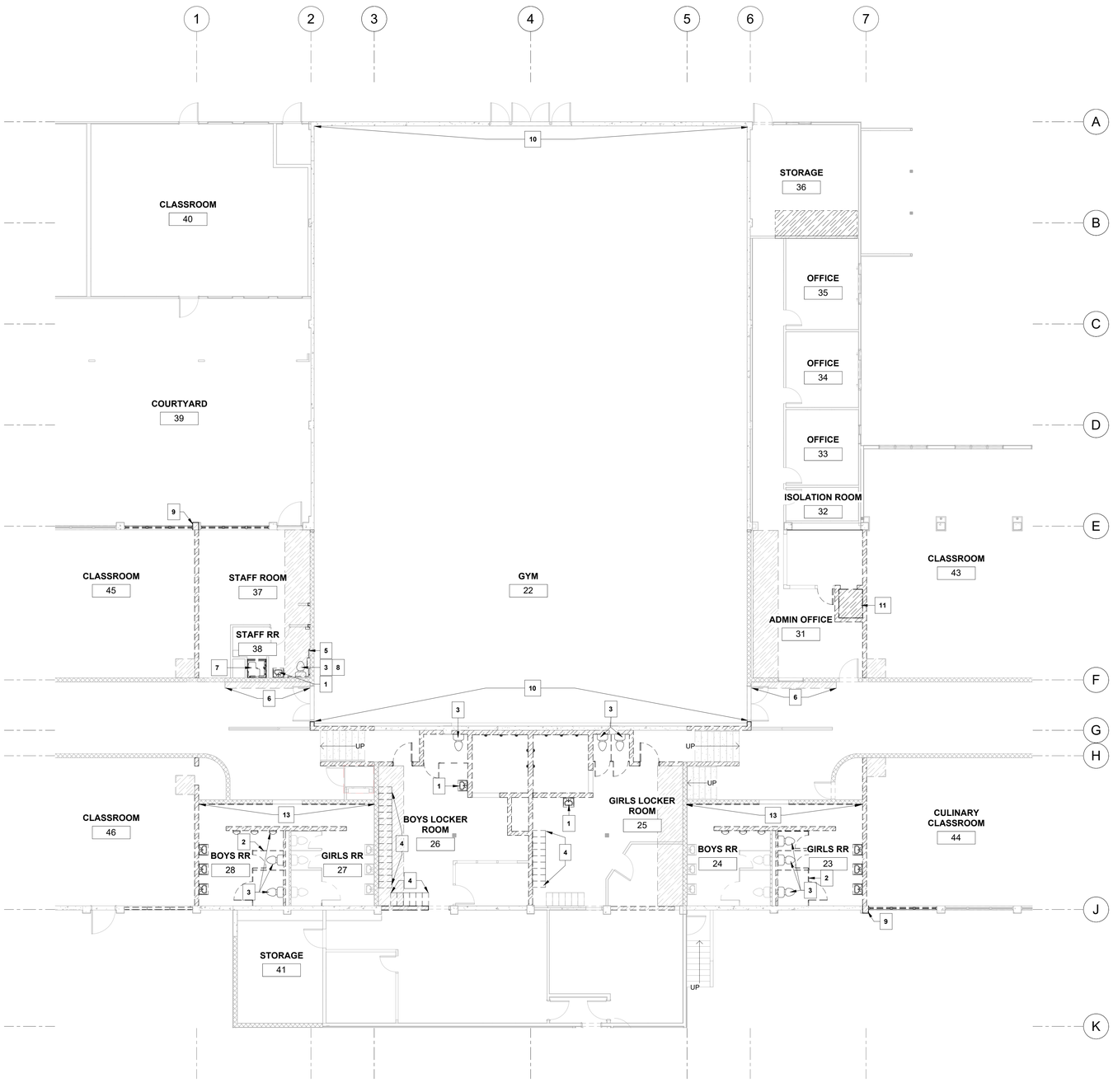
H. NO EXISTING SMOKE DETECTOR, FIRE ALARM BOX OR SIMILAR DEVICE, INCLUDING THE ASSOCIATED WIRING SHALL BE DAMAGED DURING DEMOLITION AND SUBSEQUENT CONSTRUCTION.

I. RELOCATION OF SMOKE DETECTORS, AND FIRE ALARM EQUIPMENT, NECESSITATED BY NEW CONSTRUCTION, SHALL BE ACCOMPLISHED AS A FIRST PRIORITY, AND PER THE PLANS. NO ACTIVE SMOKE DETECTOR SHALL BE PERMANENTLY COVERED OR OTHERWISE REMOVED OR USED FOR OTHER THAN ITS INTENDED PURPOSE. REMOVE TEMPORARY COVERS DAILY.

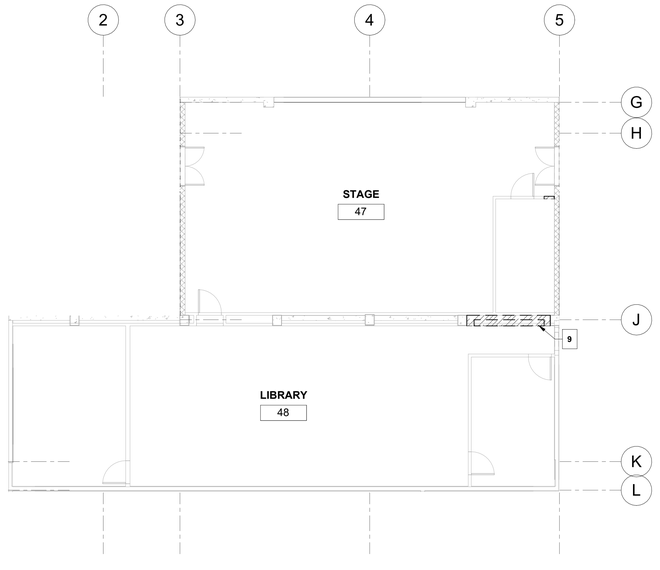
J. REMOVAL OF ANY EQUIPMENT, CABLING SWITCHES, AND CONDUIT PERTAINING TO DATA/COMMUNICATIONS AND TELEPHONE SHALL BE VERIFIED WITH OWNER/TENANT AND PROJECT TEAM.

K. EXISTING ELEMENTS SHOWN ON PLANS IS BASED ON AS-BUILT DRAWINGS AND NON-DESTRUCTIVE SITE OBSERVATION. FIELD VERIFY LOCATIONS, QUANTITIES AND CONFIGURATIONS OF EXISTING ELEMENTS. NOTIFY PROJECT TEAM IF EXISTING CONDITIONS ARE MATERIALLY DIFFERENT THAN WHAT IS SHOWN ON FLOOR PLANS, MARK MATERIAL DIFFERENCES DISCOVERED ON CONTRACTOR RED-LINED AS BUILTS.

L. REMOVE AND SAVE ALL EXISTING EQUIPMENT ON WALLS TO BE DEMOLISHED AND SAVE FOR REINSTALLATION



1 DEMOLITION FLOOR PLAN
AD2.01 1/8" = 1'-0"



2 STAGE DEMO FLOOR PLAN
AD2.01 1/8" = 1'-0"

N

ZCS
524 Main Street, Suite 2
Oregon City, OR 97045
503.659.2205

ALSEA SCHOOL DISTRICT
301 S. 3RD ST.
ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY, NOT FOR CONSTRUCTION-

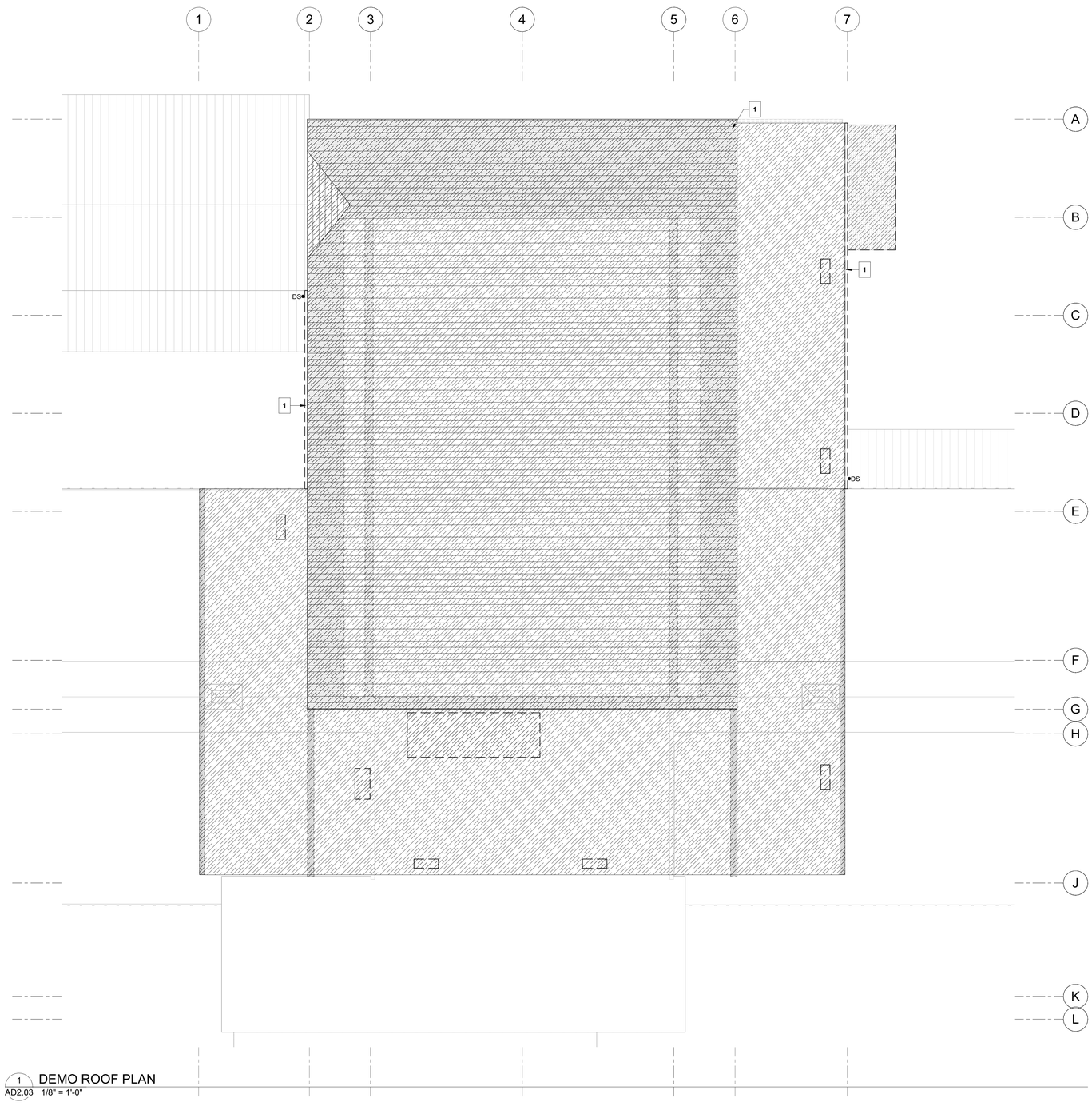
△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	JLS
CHECKED:	MBW
DATE:	12-03-2024

DEMO FLOOR PLAN

AD2.01

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



1 DEMO ROOF PLAN
AD2.03 1/8" = 1'-0"

DEMO ROOF GENERAL NOTES

A. NOTIFY ALL APPLICABLE REGULATORY AGENCIES 48 HOURS PRIOR TO BEGINNING WORK

B. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION AND MAINTENANCE OF ALL TEMPORARY ROOF ACCESS SYSTEMS. ALL SYSTEMS MUST COMPLY WITH OSHA.

C. COORDINATE STAGING AND MATERIALS STORAGE AREA WITH DISTRICT PERSONNEL.

D. THE PROPER DISPOSAL OF ALL DEMOLITION MATERIALS AND DEBRIS WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL MAKE EFFORTS TO RECYCLE AS MUCH DEMOLITION MATERIALS AS POSSIBLE.

E. ROOF REPLACEMENT SHALL BE APPROPRIATELY STAGED IN SEQUENCE TO PREVENT THE INTRUSION OF MOISTURE INTO ANY PORTION OF THE BUILDING.

F. NO PORTION OF THE ROOF SHALL BE LEFT UNPROTECTED AGAINST THE ELEMENTS BETWEEN CONTRACTOR SHIFTS.

G. CONTRACTORS SHOW IMMEDIATELY NOTIFY THE PROJECT TEAM UPON THE DISCOVERY OF ANY WATER INTRUSION RELATED DAMAGE UNDER THE EXISTING ROOF SYSTEM.

H. EXISTING ELEMENTS SHOWN ON PLANS BASED ON AS-BUILT DRAWINGS AND NON-DESTRUCTIVE SITE OBSERVATION. FIELD VERIFY LOCATIONS, QUANTITIES AND CONFIGURATIONS OF EXITING ELEMENTS. NOTIFY PROJECT TEAM IF EXISTING CONDITIONS ARE MATERIALLY DIFFERENT THAN WHAT IS SHOWN ON PLANS. MARK MATERIAL DIFFERENCES DISCOVERED ON CONTRACTOR RED-LINED AS BUILTS.

I. DEMO AND SAVE EXISTING EQUIPMENT FOR REINSTALL WHERE ROOF REPLACEMENT IS TO OCCUR

DEMO ROOF LEGEND

- (E) METAL ROOFING TO BE DEMOLISHED TO (E) SHEATHING
- (E) BUR TO BE DEMOLISHED DOWN TO (E) SHEATHING
- (E) SHEATHING TO BE DEMOLISHED
- (E) ELEMENT TO BE DEMOLISHED
- STEP IN ROOF
- ROOF SLOPE
- DS — DOWNSPOUT LOCATION
- (E) EQUIPMENT

DEMO ROOF PLAN KEYNOTES

1. DEMO (E) GUTTERS, AND DOWNSPOUTS

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ALSEA SCHOOL DISTRICT
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ALSEA, OR 97324

ALSEA GYM
SEISMIC RETROFIT

PRELIMINARY - NOT FOR CONSTRUCTION
EXPIRES: 06-30-25

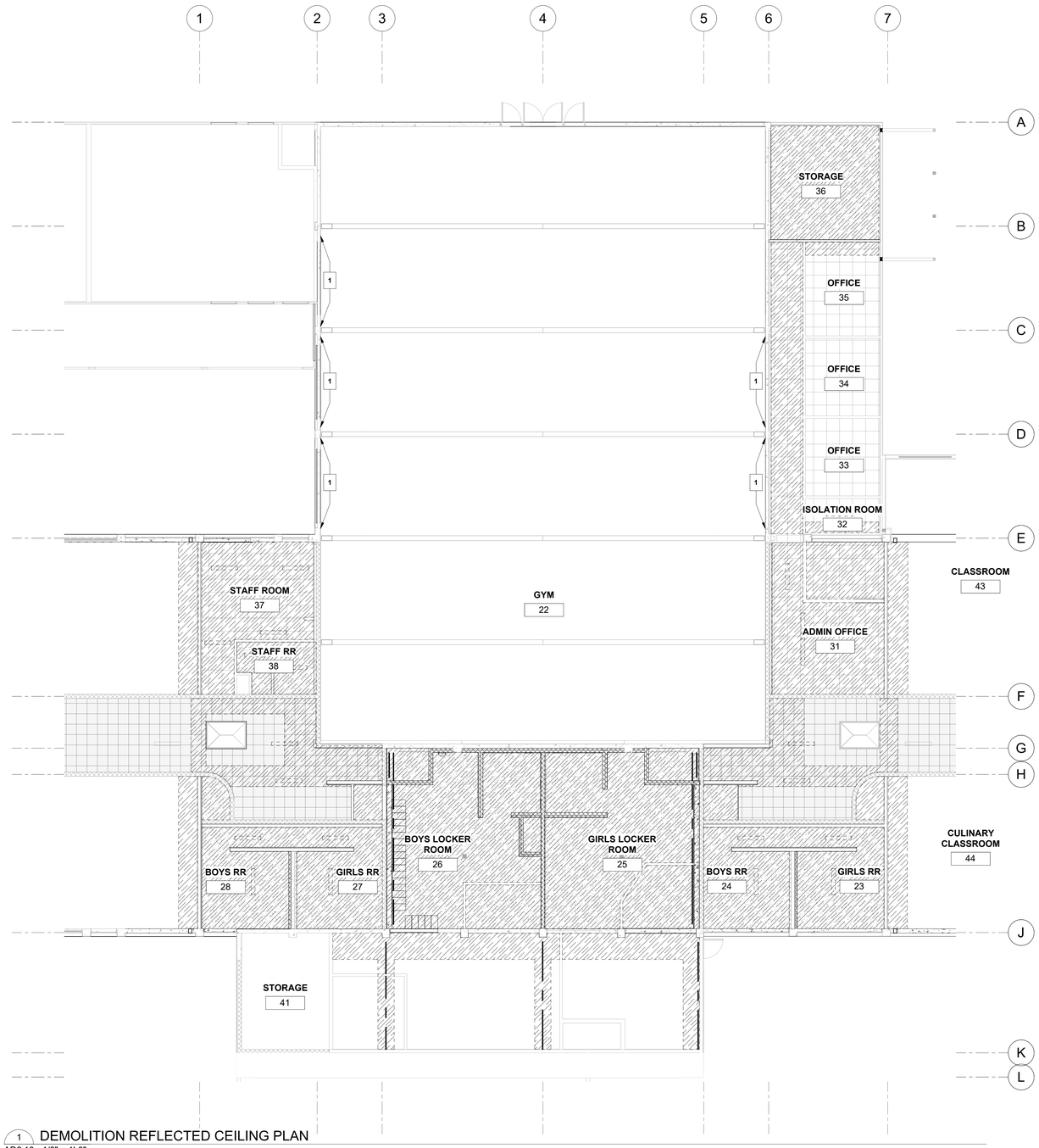
△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	JLS
CHECKED:	MBW
DATE:	12-03-2024

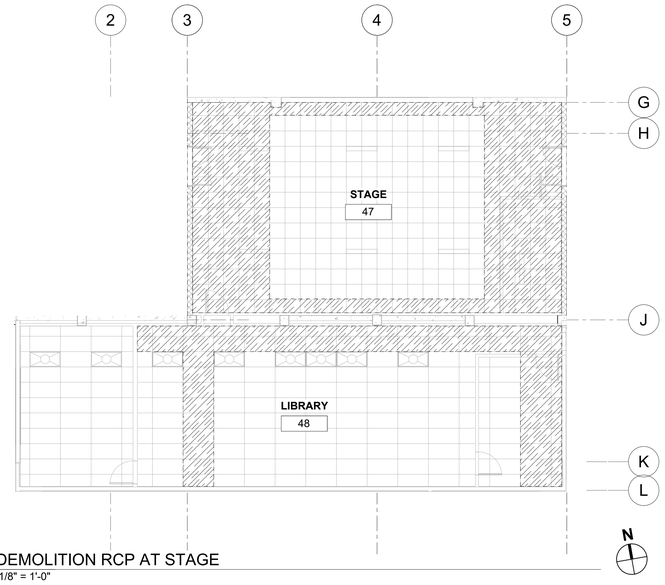
DEMO ROOF PLAN

AD2.03

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



1 DEMOLITION REFLECTED CEILING PLAN
AD6.10 1/8" = 1'-0"



2 DEMOLITION RCP AT STAGE
AD6.10 1/8" = 1'-0"

DEMO RCP LEGEND

- DEMO (E) GYP CEILING
- DEMO (E) 24" x 24" ACT CEILING
- DEMO (E) 24" x 48" ACT CEILING
- (E) SKYLIGHT
- (E) LIGHTING

DEMO RCP KEYNOTES

1. DEMO URM WALL

DEMO PLAN GENERAL NOTES

A. ALL EXISTING BUILDING COLUMNS, EXTERIOR WALLS AND STRUCTURAL MEMBERS TO REMAIN, U.N.O.

B. OBTAIN DEMOLITION PERMITS AND INCLUDE ALL COSTS OF SAME IN CONTRACT PRICE.

C. CONTRACTOR SHALL KEEP CONSTRUCTION AREA FREE OF DUST AND DEBRIS FOR THE DURATION OF CONSTRUCTION AND SHALL BE RESPONSIBLE FOR DISPOSAL OF ALL DEBRIS.

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E. IN PARTITIONS TO BE REMOVED, REMOVE AND CAP ALL OUTLETS, SWITCHES, WIRES, THERMOSTATS, ETC., TO THEIR SOURCE.

F. REMOVE ALL ABANDONED ELECTRICAL CONDUIT, CABLING BACKBOARD AND EQUIPMENT, TYPICAL THROUGHOUT ENTIRE SPACE. AT COMPLETION OF DEMOLITION WORK, THE CONSTRUCTION AREA(S) SHALL BE LEFT IN "BROOM CLEAN' CONDITION. ALL DEBRIS AND MISCELLANEOUS MATERIAL SHALL BE REMOVED.

G. IN ADDITION TO SPECIFIC DEMOLITION SCOPE IDENTIFIED, PERFORM MISCELLANEOUS DEMOLITION AS REQUIRED TO SUPPORT NEW CONSTRUCTION.

H. NO EXISTING SMOKE DETECTOR, FIRE ALARM BOX OR SIMILAR DEVICE, INCLUDING THE ASSOCIATED WIRING SHALL BE DAMAGED DURING DEMOLITION AND SUBSEQUENT CONSTRUCTION.

I. RELOCATION OF SMOKE DETECTORS, AND FIRE ALARM EQUIPMENT, NECESSITATED BY NEW CONSTRUCTION, SHALL BE ACCOMPLISHED AS A FIRST PRIORITY, AND PER THE PLANS. NO ACTIVE SMOKE DETECTOR SHALL BE PERMANENTLY COVERED OR OTHERWISE REMOVED OR USED FOR OTHER THAN ITS INTENDED PURPOSE. REMOVE TEMPORARY COVERS DAILY.

J. REMOVAL OF ANY EQUIPMENT, CABLING SWITCHES, AND CONDUIT PERTAINING TO DATA/COMMUNICATIONS AND TELEPHONE SHALL BE VERIFIED WITH OWNER/TENANT AND PROJECT TEAM.

L. EXISTING ELEMENTS SHOWN ON PLANS IS BASED ON AS-BUILT DRAWINGS AND NON-DESTRUCTIVE SITE OBSERVATION. FIELD VERIFY LOCATIONS, QUANTITIES AND CONFIGURATIONS OF EXISTING ELEMENTS. NOTIFY PROJECT TEAM IF EXISTING CONDITIONS ARE MATERIALLY DIFFERENT THAN WHAT IS SHOWN ON FLOOR PLANS. MARK MATERIAL DIFFERENCES DISCOVERED ON CONTRACTOR RED-LINED AS BUILTS.

K. DEMOLISH ACT CEILINGS TO NEXT CLOSEST CEILING GRID

L. EXISTING LIGHT FIXTURES TO BE REMOVED AND RETAINED FOR RE-INSTALLATION AS NEEDED TO ACCOMADATE WORK

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ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY-
NOT FOR CONSTRUCTION

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	JLS
CHECKED:	MBW
DATE:	12-03-2024

DEMOLITION REFLECTED CEILING PLAN

AD6.10

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

GENERAL NOTES

A. VERIFY ALL DIMENSIONS AND NOTIFY PROJECT TEAM IF DISCREPANCIES OCCUR.

B. G.C. SHALL COORDINATE ALL INTERIOR AND EXTERIOR FINISHES w/ ARCH. PRIOR TO CONSTRUCTION.

C. PAINT NEW GYP BD TO MATCH EXISTING ADJACENT, U.N.O.

D. G.C. TO PROVIDE FIRE BLOCKING AS REQUIRED PER CODE.

E. G.C. SHALL PROVIDE ALL APPROPRIATE BACKING AS REQUIRED FOR ACCESSORIES AND OTHER MISCELLANEOUS ITEMS.

F. ALL DIMENSION LINES TO THE FACE OF FRAMING, U.N.O.

WALL LEGEND

NEW WALL

NEW FLOOR TO MATCH EXISTING

FLOOR PLAN KEYNOTES

1. REMOVE AND REINSTALL LAVATORY
2. REMOVE AND REINSTALL PARTITIONS
3. REMOVE AND REINSTALL WATER CLOSET
4. REMOVE AND REINSTALL LOCKERS
5. REMOVE AND REINSTALL SHOWER HEADS
6. NEW GYP BD AT NEW SHEAR WALLS
7. NEW TILE AT NEW SHOWER WALLS
8. INFILL WALL AND FINISH TO MATCH ADJACENT
9. NEW FRP AND RESILIENT WALL BASE TO MATCH EXISTING ADJACENT
10. TILE PATCH / REPAIR CONC FLOOR
11. FRP AT NEW WALL IN RESTROOM
12. NEW RESILIENT FLOOR TILE TO MATCH ADJACENT
13. REBUILD PORTION OF RAMP TO MATCH ADJACENT
14. REMOVE AND REINSTALL GRABRAIL
15. REMOVE AND REINSTALL SHOWER COMPARTMENT
16. NEW URM WALL
17. NEW CARPET TILE TO MATCH ADJACENT

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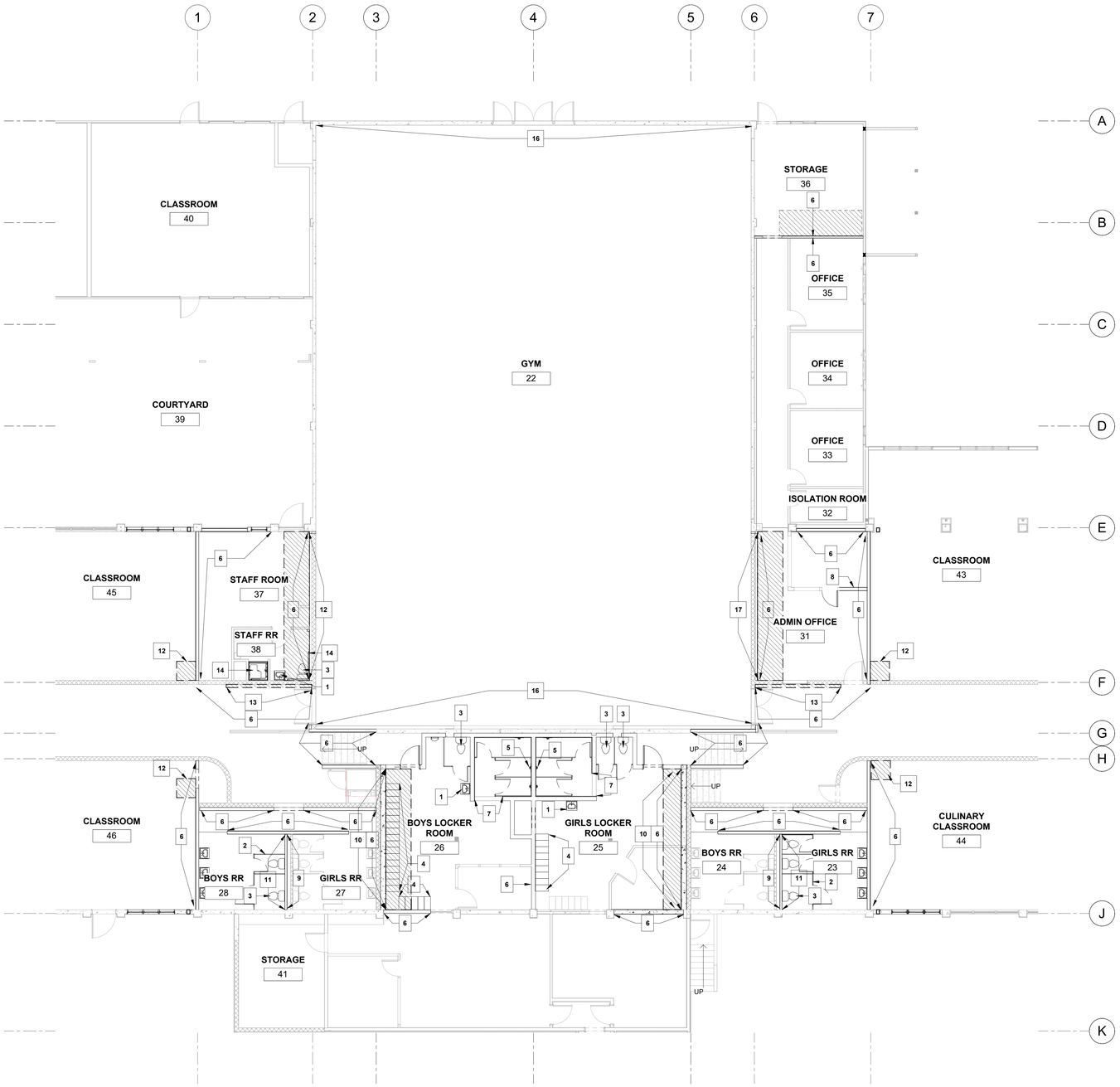
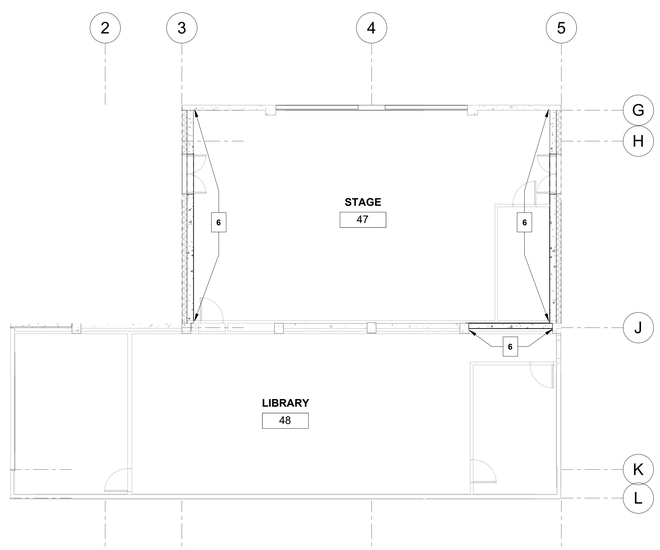


1 A2.01 MAIN FLOOR PLAN
1/8" = 1'-0"



2 A2.01 STAGE FLOOR PLAN
1/8" = 1'-0"

N

-PRELIMINARY-
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△	DESCRIPTION	DATE:

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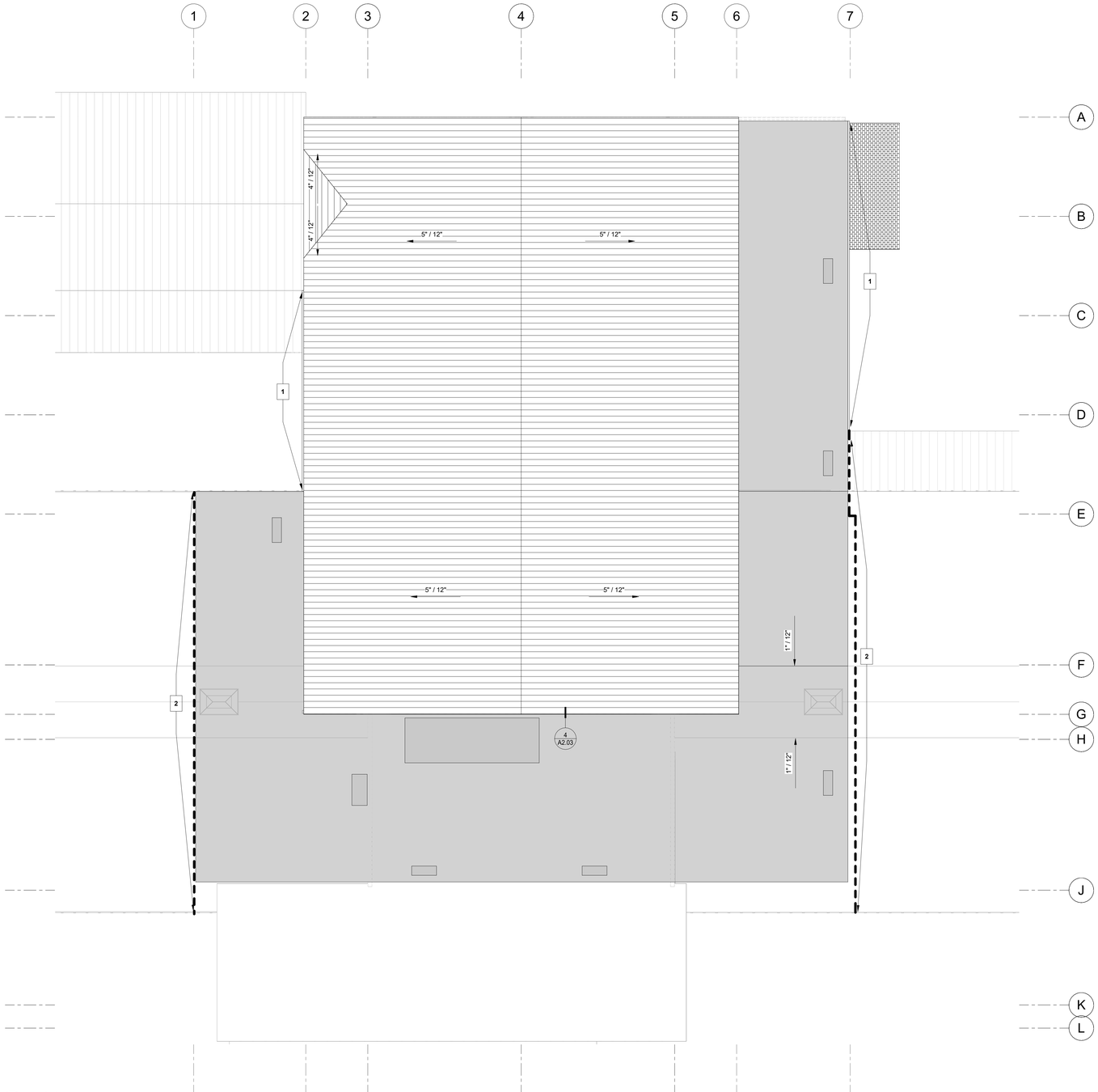
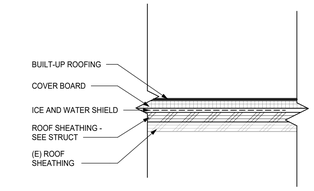
FLOOR PLAN

A2.01

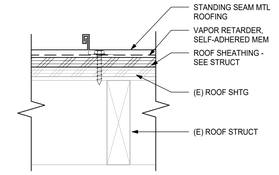
100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



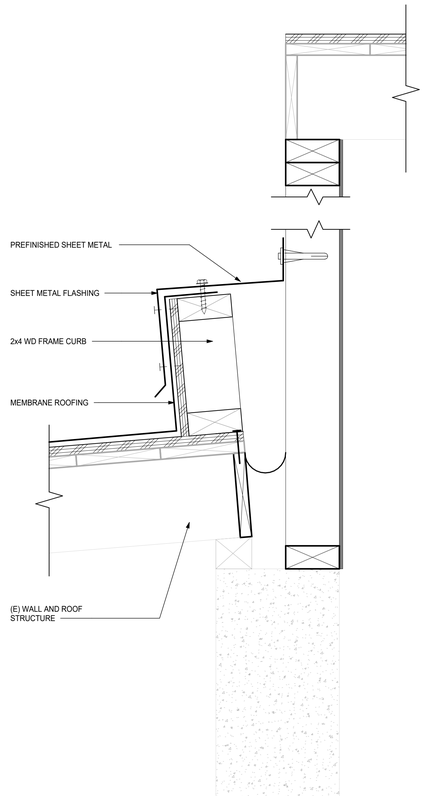
1 A2.03 ROOF PLAN
1/8" = 1'-0"



2 A2.03 ROOF ASSEMBLY - A
3" = 1'-0"



3 A2.03 ROOF ASSEMBLY - TYPE B
3" = 1'-0"



4 A2.03 JOINT COVER DETAIL
3" = 1'-0"

ROOF PLAN GENERAL NOTES

A. ALL WORK AND MATERIALS SHALL CONFORM TO ALL APPLICABLE STATE AND LOCAL REGULATIONS, STANDARDS AND MFR. SPECIFICATIONS AND THE 2022 OSSC. CONTACT PROJECT TEAM FOR DIRECTIVE IN THE EVENT OF CONFLICTING STANDARDS AND SPECS.

B. VERIFY ALL DIMENSIONS, ELEVATIONS AND LOCATIONS PRIOR TO CONSTRUCTION. NOTIFY PROJECT TEAM OF ANY DISCREPANCIES. DIMENSIONS ON THIS PLAN ARE NOT SUITABLE FOR MATERIAL ORDERING USE. CONTRACTOR MUST FIELD VERIFY ALL DIMENSIONS PRIOR TO BIDDING AND ORDERING.

C. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION, AND MAINTENANCE OF ALL TEMPORARY ROOF ACCESS SYSTEMS. ALL SYSTEMS MUST COMPLY WITH OSHA.

D. COORDINATE STAGING AND MATERIALS STORAGE AREA WITH ANY APPLICABLE PARTIES.

E. SECURITY OF STORED MATERIAL IS THE RESPONSIBILITY OF THE CONTRACTOR.

F. NO PORTION OF THE ROOF SHALL BE LEFT UNPROTECTED AGAINST THE ELEMENTS BETWEEN CONTRACTOR SHIFTS.

G. REINSTALL EXISTING ROOFTOP EQUIPMENT

H. SEE PLAN SET AND/OR SPECIFICATIONS FOR MORE INFORMATION.

ROOF SYMBOLS

ROOF SLOPE

EXISTING EQUIPMENT

ROOF TYPE LEGEND

NEW STANDING SEAM METAL ROOF

NEW BUILT UP ROOF

NEW ASPHALT SHINGLE ROOF

ROOF PLAN KEYNOTES

1. PRE-FINISHED GUTTER AND DOWNSPOUTS w/ CLEANOUTS, MATCH EXISTING DOWNSPOUT LOCATIONS AND COLOR
2. SEISMIC JOINT

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PRELIMINARY - NOT FOR CONSTRUCTION

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DRAWN:	JLS
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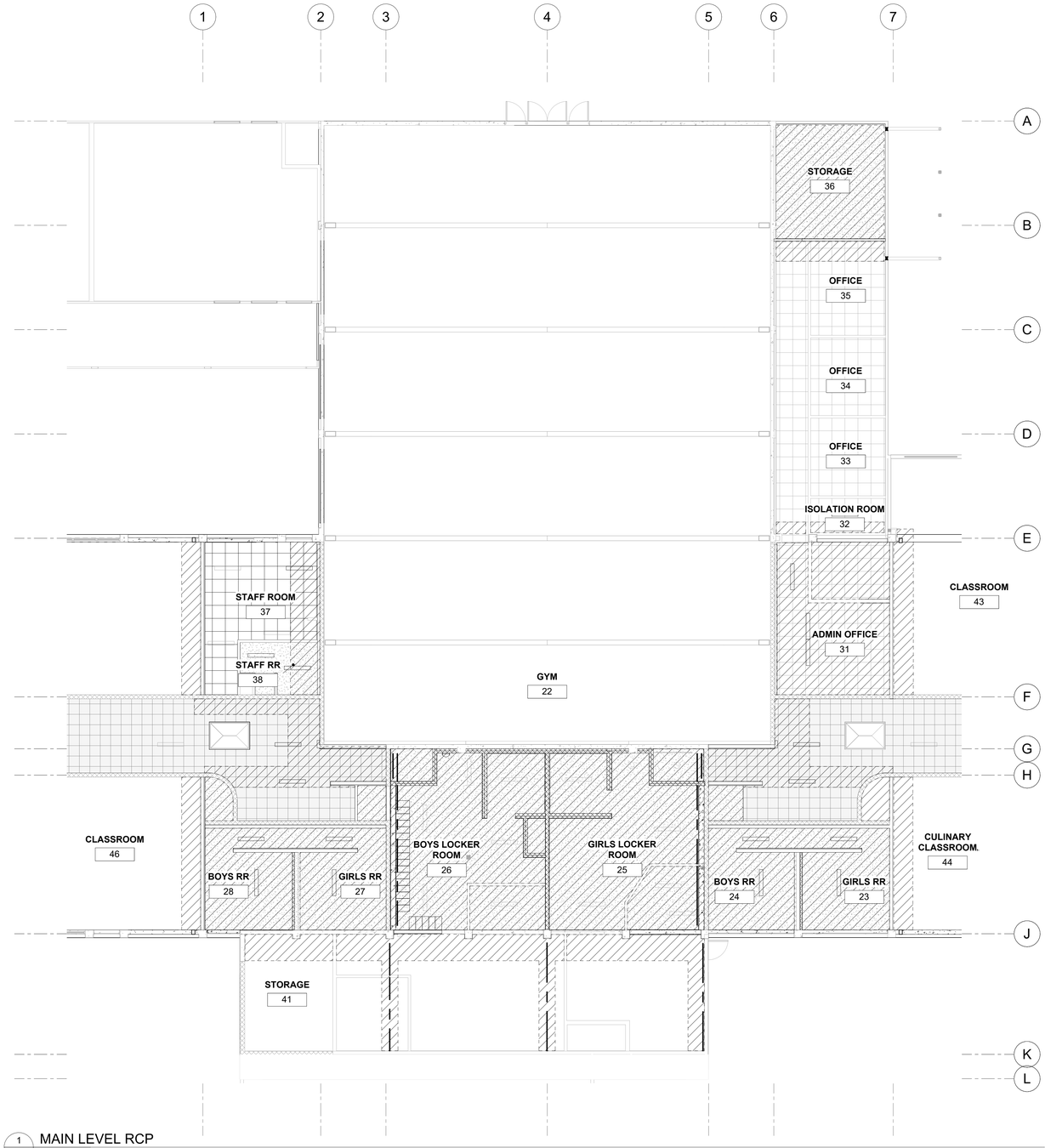
DATE:	12-03-2024

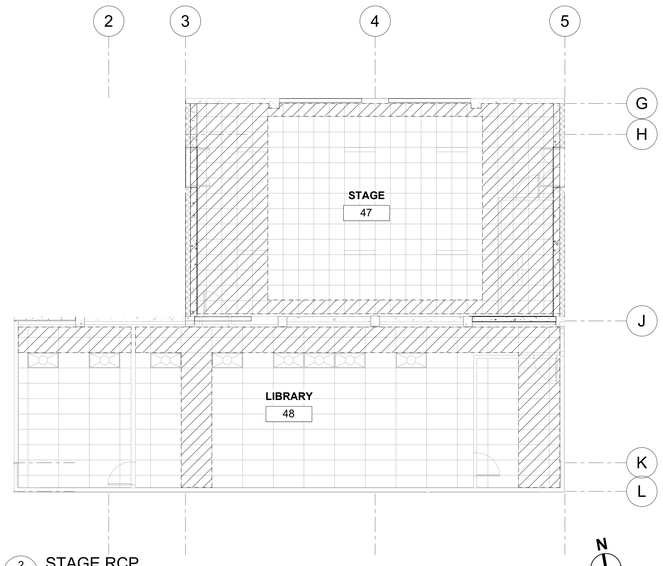
ROOF PLAN

A2.03

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



1 A6.01 MAIN LEVEL RCP
1/8" = 1'-0"



2 A6.01 STAGE RCP
1/8" = 1'-0"

N

RCP GENERAL NOTES

A. PROVIDE WALL BACKING FOR REINFORCEMENT AS REQUIRED.

B. PROVIDE SOLID BLOCKING FOR ALL 'J' BOXES SUSPENDED LIGHT AND CEILING FAN FIXTURES, TELEVISION SUPPORT, ARTIFACT SHELVES AND ANY OTHER CEILING MOUNTED EQUIPMENT.

C. THE MEANS OF EGRESS ILLUMINATION LEVEL SHALL BE NOT LESS THAN 1 FOOTCANDLE AT THE WALKING SURFACE. ALONG EXIT ACCESS STAIRWAYS, EXIT STAIRWAYS AND AT THEIR REQUIRED LANDINGS, THE ILLUMINATION LEVEL SHALL NOT BE LESS THAN 10 FOOTCANDLES AT THE WALKING SURFACE WHEN THE STAIRWAY IS IN USE.

D. ALL EXPOSED CONDUITS AND 'J' BOXES SHALL BE PAINTED TO MATCH THE ADJACENT FINISH U.N.O.

E. ALL ROOMS THAT ARE TO RECEIVE A CEILING PATCH SHALL HAVE ALL OF HARD LID PAINTED TO MATCH EXISTING ADJACENT U.N.O.

F. REINSTALL ALL (E) LIGHTING FIXTURES REMOVED DURING CEILING DEMO

CEILING LEGEND :

NEW GYP CEILING TO MATCH EXISTING

NEW 24" x 24" ACT CEILING TO MATCH EXISTING

NEW 24" x 48" ACT CEILING TO MATCH EXISTING

RCP KEYNOTES

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ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY-
NOT FOR CONSTRUCTION
EXPIRES: 06-30-25

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	JLS
CHECKED:	MBW
DATE:	12-03-2024

REFLECTED CEILING PLAN

A6.01

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

PROJECT STRUCTURAL NOTES (Alsea, Benton County, Oregon, 97324)

GENERAL INFORMATION:

1. STRUCTURAL DRAWINGS ARE A PORTION OF THE CONTRACT DOCUMENTS AND ARE INTENDED TO BE USED WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THESE DRAWINGS INTO THEIR SHOP DRAWINGS AND WORK. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.
2. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
3. ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.
4. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS. RESPONSIBILITY SHALL INCLUDE BUT NOT LIMITED TO DEMOLITION AND CONSTRUCTION MEANS AND METHODS, TECHNIQUES, SEQUENCING, AND SAFETY REQUIRED TO COMPLETE CONSTRUCTION.
5. UNLESS OTHERWISE NOTED, MATERIAL AND DESIGN SPECIFICATIONS CITED HEREIN SHALL BE THOSE CONFORMING WITH THE VERSION OF THE APPLICABLE SPECIFICATIONS OR CODE MOST RECENTLY ADOPTED BY THE PERMITTING AUTHORITY. THESE STRUCTURAL NOTES ARE TO BE USED AS A SUPPLEMENT TO THE SPECIFICATIONS.
6. THIS STRUCTURE AND ALL OF ITS PARTS MUST BE ADEQUATELY BRACED AGAINST WIND, LATERAL EARTH AND SEISMIC FORCES UNTIL THE PERMANENT LATERAL-FORCE RESISTING SYSTEMS HAVE BEEN CONSTRUCTED AND ALL ATTACHMENTS AND CONNECTIONS NECESSARY FOR THE STABILITY OF THE STRUCTURE AND ITS PARTS HAVE BEEN MADE.
7. ALL FEATURES OF CONSTRUCTION NOT FULLY SHOWN SHALL BE OF THE SAME TYPE AND CHARACTER AS SHOWN FOR SIMILAR CONDITIONS, SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.
8. ALL PRODUCTS AND MATERIALS USED BY THE CONTRACTOR SHALL BE APPLIED, PLACED, ERECTED OR INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
9. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.

CODE REQUIREMENT:

1. CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE AND ASCE 41-17.

DESIGN CRITERIA:

1. DESIGN IS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADING AND ALLOWABLE LOAD IS USED FOR DESIGN:

A. LIVE LOADS:
- ROOF — 20 PSF
- GYM — 100 PSF

B. GROUND SNOW LOAD: — 33 PSF
- EXPOSURE FACTOR — 1.0
- SNOW IMPORTANCE FACTOR — 1.0
- THERMAL FACTOR — 1.0
- FLAT ROOF SNOW LOAD — 29 PSF

C. WIND LOAD:
- BASIC WIND SPEED (3-SECOND GUST) — 107 MPH
- WIND EXPOSURE — C
- WIND IMPORTANCE FACTOR — 1.0
- BUILDING CATEGORY — IV
- INTERNAL PRESSURE COEFFICIENT — 0.18
- TOPOGRAPHIC FACTOR — 1.0

D. EARTHQUAKE DESIGN DATA:
- RISK CATEGORY — IV
- BSE-1E
- S_S — 0.180g
- S_1 — 0.070g
- S_{XS} — 0.230g
- BSE-2E
- S_S — 0.710g
- S_1 — 0.370g
- S_{XS} — 0.870g
- SITE CLASS — C
- SEISMIC DESIGN CATEGORY — D
- ANALYSIS PROCEDURE — LINEAR STATIC PROCEDURE

BASIC SEISMIC-FORCE RESISTING SYSTEM:
- CONCRETE SHEAR WALLS
- WOOD SHEAR WALLS

E. GEOTECHNICAL CRITERIA:
- DESIGN BASED ON REPORT BY: — FOUNDATION ENGINEERING DATED SEPTEMBER 10,2024
- ULTIMATE SOIL PRESSURE: — 7,500 PSF

STRUCTURAL OBSERVATION:

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATION BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SER TO PERFORM THESE OBSERVATIONS.

STRUCTURAL OBSERVATION	
ITEM	**COMMENTS**
PRIOR TO FIRST CONCRETE POUR	AFTER REBAR PLACEMENT
PRIOR TO BEGINNING SHOTCRETE OPERATIONS	AFTER REBAR PLACEMENT
DURING INITIAL STEEL ERECTION	--
DURING INITIAL WOOD FRAMING CONSTRUCTION	--
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	--

A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.

SPECIAL INSPECTIONS AND TESTING:

1. SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEETS S0.20. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS:

1. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING:
   A. CONCRETE MIX DESIGNS, CONCRETE AND MASONRY REINFORCEMENT (INCLUDING MILL TEST REPORTS), STRUCTURAL STEEL (INCLUDING MILL TEST REPORTS)
   B. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE OF THE STRUCTURAL ENGINEER OF RECORD.
   C. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS, INCLUDING: GLUE-LAMINATED MEMBERS, SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE INCLUDED FOR CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC WITH THE FOLLOWING:
      - EARTHQUAKE AND WIND LOADS AS NOTED IN DESIGN CRITERIA
   D. THE CONTRACTOR SHALL COORDINATE SEISMIC RESTRAINTS OF MECHANICAL, PLUMBING, AND ELECTRICAL EQUIPMENT, MACHINERY, AND ASSOCIATED PIPING WITH THE STRUCTURE. ANY CONNECTIONS TO THE STRUCTURE SHALL CONFORM TO OSSC AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION.

DIVISION 03 - CONCRETE

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD 28 DAY CYLINDER TESTS PER ASTM C39, AND SHALL BE AS FOLLOWS:

ABSOLUTE WATER-CEMENT RATIO BY WEIGHT			
f'c (PSI)	NON AIR-ENTRAINED	AIR-ENTRAINED	USE
4,000	0.50	N/A	INTERIOR SLABS ON GRADE
4,000	0.45	N/A	WALL FOOTINGS
5,000	0.45	N/A	SHEAR WALLS AS NOTED

2. VERIFY WATER/CEMENT RATIO WITH FLOOR COVERING MANUFACTURER FOR CONCRETE FLOORS WITH MOISTURE SENSITIVE FLOOR COVERINGS, AND VERIFY COORDINATE WITH PROJECT SPECIFICATIONS.
3. MINIMUM CEMENT CONTENT PER CUBIC YARD SHALL BE AS FOLLOWS:
   - f'c=4,000 psi: 550 lbs.
4. FLY ASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F, MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.
5. THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS, ALONG WITH TEST DATA COMPLIANT WITH OSSC SECTION 1905, A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.
6. A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494, USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, SHALL BE INCORPORATED IN CONCRETE DESIGN MIXES. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494, TYPE F OR G, MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 8". AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260 SHALL BE USED IN CONCRETE MIXES FOR EXTERIOR HORIZONTAL SURFACES EXPOSED TO WEATHER. THE AMOUNT OF ENTRAINED AIR SHALL BE 5% +/- 1% BY VOLUME.

CONCRETE CAST IN PLACE:

1. CONCRETE SHALL HAVE A MAXIMUM SLUMP OF 4" WITHOUT THE USE OF ADMIXTURES AS NOTED.
2. A MINIMUM OF THREE (3) CONCRETE TEST CYLINDERS SHALL BE PROVIDED FOR EACH ONE HUNDRED (100) CU. YARDS, OR EACH DAY OF POUR, FOR EACH CONCRETE STRENGTH. CYLINDERS SHALL BE TESTED AS FOLLOWS:
   A. ONE (1) AT SEVEN (7) DAYS, AND
   B. TWO (2) AT TWENTY-EIGHT (28) DAYS
3. CONCRETE CYLINDER SAMPLING AND TESTING SHALL CONFORM WITH ASTM SPECIFICATIONS. ACCEPTANCE OF CONCRETE SHALL BE GOVERNED BY THE PROVISIONS OF ACI 318-19 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". TWO (2) SETS OF MIX DESIGNS, WITH COMPLETE STATISTICAL BACKUP, SHALL BE SUBMITTED FOR REVIEW.
4. CONCRETE MATERIALS, FORM WORK, MIXING, PLACING AND CURING SHALL CONFORM WITH THE SPECIFICATIONS CONTAINED IN THE ACI "MANUAL OF CONCRETE PRACTICE".
5. AT AREAS OF DEPRESSIONS FOR SLABS AND BEAMS, PROVIDE MINIMUM THICKNESS OF DEPTH AS FOR ADJACENT AREAS, UNLESS NOTED OTHERWISE.
6. CONCRETE SLABS SHALL BE INSTALLED WITH CONSTRUCTION JOINTS NOT SPACED FARTHER THAN 12'-6" APART AND SHALL BE DIVIDED INTO APPROXIMATELY SQUARE PANELS. PANEL DIMENSION RATIOS SHALL NOT EXCEED 1.5:1.
7. **ALL SAW CUT CONTROL JOINTS SHALL BE CUT WITHIN 4 TO 12 HOURS AFTER CONCRETE PLACEMENT. SAW CUT SHALL BE 1.5" DEEP.**
8. CONCRETE SHALL NOT BE PLACED ON FROZEN GROUND.
9. BOND NEW CONCRETE TO EXISTING CONCRETE WITH "WELD-CRETE", AS MANUFACTURED BY LARSON PRODUCTS CORPORATION, OR APPROVED. AS A MINIMUM, EXISTING CONCRETE SURFACES SHALL BE ROUGHENED BY CHIPPING TO A MINIMUM 1/4" AMPLITUDE TO EXPOSE COARSE AGGREGATE. PREPARATION AND APPLICATION IS TO BE IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
10. ALL EXPOSED CORNERS SHALL HAVE 3/4" CHAMFER, UNLESS NOTES OTHERWISE.

CONCRETE REINFORCING STEEL:

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. FOR DEFORMED BARS AND ASTM A185 FOR SMOOTH WELDED WIRE FABRIC (WWF), UNLESS OTHERWISE NOTED. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE.
2. BARS IN SLABS SHALL BE SUPPORTED ON WELL CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STRANDED PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAINED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE, MSP-1 REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315. LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULE, EXCEPT AS NOTED. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON BAR-GRIP SPLICES OR APPROVED WITH A CURRENT ICC APPROVAL REPORT.

TYPICAL LAP SPLICE LENGTH SCHEDULE								
BAR SIZE	3,000 psi		4,000 psi		5,000 psi		6,000 psi	
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	22	32	19	28	17	25	16	23
#4	29	43	25	37	22	33	20	31
#5	36	54	31	47	28	42	25	38
#6	43	64	37	56	33	50	31	46
#7	63	94	54	81	49	73	44	66
#8	72	107	62	93	55	83	51	76

NOTES:
A. DIMENSIONS ARE IN INCHES.
B. CASES 1 AND 2 ARE DEFINED AS FOLLOWS: (db = BAR DIAMETER)
   a. BEAMS OR COLUMNS:
      - CASE 1: COVER ≥ db AND c-c SPACING ≥ 2db
      - CASE 2: COVER < db OR c-c SPACING < 2db
   b. ALL OTHERS:
      - CASE 1: COVER ≥ db AND c-c SPACING ≥ 3db
      - CASE 2: COVER < db OR c-c SPACING < 3db
C. FOR TOP BARS, MULTIPLY LAP LENGTH ABOVE BY 1.3. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.

3. REINFORCEMENT SHALL BE SECURED IN FORMS WITH TIES AND ANCHORAGE TO PREVENT DISPLACEMENT. ALL TIE WIRE SHALL BE MIN. #16 ANNEALED STEEL.
4. ALL REINFORCING STEEL SHALL BE TIED 100% ALONG ALL PERIMETER EDGES AND 50%FIELD.
5. REINFORCING (MINIMUM UNLESS NOTED OTHERWISE ON PLANS)
   A. PLACE TWO (2) NO. 4 CONTINUOUS AT BOTTOM, TOP AND AT DISCONTINUOUS ENDS OF ALL FOUNDATIONS.
   B. PLACE BARS AT CORNERS AND INTERSECTIONS FOR WALLS AND FOUNDATIONS EQUAL IN SIZE AND NUMBER TO HORIZONTAL REINFORCING WITH LEGS THAT SATISFY THE REQUIRED LAP SPLICE LENGTH PER SCHEDULE ABOVE.
   C. PLACE TWO (2) NO. 4x OPENING DIMENSIONS PLUS 4'-0" EACH SIDE OF ALL OPENINGS AND TWO (2) NO. 4x4'-0" DIAGONAL BARS AT EACH CORNER OF ALL SLAB OPENINGS GREATER THAN 1'-6" IN DIMENSION.
6. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI DETAILING MANUAL 315.
   A. ALL REINFORCING STEEL SHALL BE ACCURATELY AND SECURELY PLACED.
   B. REINFORCING SHALL NOT BE BENT OR DISPLACED FOR THE CONVENIENCE OF OTHER TRADES, UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
   C. SPLAY REINFORCING STEEL AROUND OPENINGS WITH 1" IN 10" SPLAY, UNLESS NOTED OTHERWISE.
   D. MINIMUM COVER FROM CONCRETE SURFACES TO REINFORCING STEEL SHALL BE:
      - 3" TO BOTTOM OF FOOTING
      - 2" TO EARTH FACE OF WALL
      - 3/4" TO INSIDE FACE OF WALL
      - 1 1/2" TO MAIN STEEL BEAMS AND COLUMNS
      - 3/4" SLAB TO TOP AND BOTTOM SURFACES, CENTER OF SLAB ON GRADE
7. REINFORCEMENT BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT, UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
8. REINFORCEMENT COUPLERS SHALL BE LENTON, FOX-HOWLETT OR APPROVED, CAPABLE OF DEVELOPING ONE HUNDRED TWENTY-FIVE PERCENT (125%) OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCEMENT.

CONCRETE ACCESSORIES:

1. EXPANSION BOLTS SHALL BE HILTI KWIK TZ, DEWALT POWER-STUD+SD2, OR APPROVED WITH EQUIVALENT ICC ALLOWABLE TENSION AND SHEAR VALUES. EXPANSION BOLTS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
2. EPOXY ADHESIVE SHALL BE HILTI HIT-RE 500 V3, SIMPSON SET-3G, DEWALT PURE110+ EPOXY, DEWALT AC200+ ACRYLIC, OR APPROVED WITH EQUIVALENT ICC ALLOWABLE TENSION AND SHEAR VALUES. EPOXY ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION.
3. PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED, GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE NOTED. NO LOADS OR WELDS SHALL BE PLACED ON EMBEDDED PLATES OR ANGLES FOR A MINIMUM OF 7 DAYS AFTER CASTING.
4. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI, OR AN APPROVED ALTERNATE WHEN SUBMITTED AND APPROVED BY THE EOR (ACI 318-19 D.9.2.2)/(ACI 318-19 17.8.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
5. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI 318-19 17.2.2).
6. USE SIMPSON ARC (ADHESIVE RETAINING CAPS) WHERE SIMPSON EPOXY IS SPECIFIED.

FOUNDATIONS:

1. ALL FOUNDATION ELEMENTS SHALL BE PLACED OVER FIRM, UNDISTURBED NATIVE SOIL OR ON APPROVED STRUCTURAL FILL. THE EXISTING SITE SHALL BE CLEARED AND GRUBBED OF ALL ORGANIC AND/OR EXPANSIVE MATERIAL PRIOR TO STRUCTURAL FILL IMPORT. REFER TO THE PROJECT GEOTECHNICAL REPORT FOR SUBGRADE REQUIREMENTS.
2. ALL PERIMETER FOUNDATION ELEMENTS SHALL BE PLACED A MINIMUM OF 12" BELOW LOWEST ADJACENT EXTERIOR GRADE.
3. THE GENERAL CONTRACTOR SHALL COORDINATE ALL SLAB PENETRATIONS WITH PLUMBING, ELECTRICAL AND MECHANICAL REQUIREMENTS.
4. THE GENERAL CONTRACTOR SHALL REVIEW THE PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL RECOMMENDATIONS FOR STRUCTURAL FILL, PLACEMENT OF STRUCTURAL FILL, AND COMPACTION REQUIREMENTS.

NON-SHRINK GROUT:

1. GROUT SHALL BE NON-SHRINKABLE GROUT CONFORMING WITH ASTM C 1107 AND C.R.D. - 621, CORPS OF ENGINEERS "SPECIFICATIONS FOR NON-SHRINK GROUT". GROUT SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT TWENTY-EIGHT (28) DAYS OF 5000 psi. PRE-GROUTING OF BASE PLATES WILL NOT BE PERMITTED.

DIVISION 05 - METALS

STRUCTURAL STEEL AND MISCELLANEOUS IRON:

STRUCTURAL STEEL SHALL BE:

STRUCTURAL STEEL	
ASTM A36	CHANNELS, PLATES, AND ANGLES, U.N.O.
ASTM A500, GRADE B (Fy = 46 KSI)	HOLLOW STRUCTURAL SECTIONS (TUBES)

1. DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE", WITH EXCEPTIONS NOTED IN SPECIFICATIONS.
2. DRAWINGS ARE DIMENSIONED FOR LAYOUT AND NOT DIMENSIONED PER AISC STANDARDS. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO COORDINATE BETWEEN ALL DRAWINGS AND DEVELOP SHOP DRAWINGS WITH DETAIL AND DIMENSIONING PER AISC.
3. ALL FABRICATION, ERECTION, IDENTIFICATION, AND PAINTING SHALL CONFORM TO AISC SPECIFICATIONS.
4. ALL STEEL EXPOSED TO WEATHER, SOIL, MOISTURE, OR AS DENOTED ON PLANS SHALL BE HOT DIP GALVANIZED PER ASTM A-123, OR OTHER APPROVED PROTECTIVE COATING.
5. ALL WELDING SHALL CONFORM TO AWS (LATES EDITION) SPECIFICATIONS.
   A. ALL WELDERS TO BE QUALIFIED UNDER AWS SPECIFICATIONS WITHIN THE PAST TWO YEARS FOR THE TYPE OF WELDING PERFORMED.
   B. ALL WELDS SHALL BE PERFORMED USING PRE-QUALIFIED WELDING PROCEDURES.
   C. WELDS FILLER METAL SHALL BE AWS A5.1 OR A5.5 E70XX ELECTRODES OR AWS A5.18 ER70S-X OR A5.2 E7XT-X.
   D. AFTER FABRICATION, BUT BEFORE INSTALLATION, REMOVE RUST, SCALE, GREASE, AND OIL BY WIRE BRUSHING AND CHEMICAL TREATMENT.
   E. WELDING OF REINFORCING STEEL SHALL BE AS SPECIFIED IN THESE STRUCTURAL NOTES UNDER **"CONCRETE REINFORCING STEEL"**.
   F. WELDS TO METAL DECK, METAL STUDS OR OTHER LIGHT GAUGE METALS SHALL CONFORM WITH AWS D1.3.
6. ALL MEMBERS SHALL BE CONNECTED WITH SEMI-FINISHED MACHINE BOLTS, UNLESS NOTED OTHERWISE ON PLANS. MACHINE BOLTS SHALL CONFORM TO ASTM A 307, GRADE A.
7. STRUCTURAL STEEL AND MISCELLANEOUS IRON:
   A. EXPANSION ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTMA 153, A.I.S.I. 304 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP II, TYPE 4, CLASS 1. ACCEPTABLE ANCHORS ARE HILTI "KWIK-BOLT TZ", SIMPSON STRONG BOLT, OR DEWALT POWER STUD+. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
   B. SLEEVE ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, A.I.S.I. 304 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP II, TYPE 3, CLASS 3. AN ACCEPTABLE ANCHOR IS THE HILTI "SLEEVE" ANCHOR, AS MANUFACTURED BY THE HILTI FASTENING SYSTEMS, INC. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
   C. FLUSH SHELL ANCHORS SHALL BE I.C.B.O. APPROVED (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, A.I.S.I. 303 STAINLESS STEEL) AND CONFORM WITH FS FF-S-325, GROUP VIII, TYPE 1. AN ACCEPTABLE ANCHOR IS THE HILTI "HDI" ANCHOR, AS MANUFACTURED BY HILTI FASTENING SYSTEMS, INC. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
   D. ADHESIVE ANCHORS SHALL BE I.C.B.O. APPROVED AND SHALL CONSIST OF ALL-THREAD ANCHOR ROD, NUT, WASHER AND EPOXY INJECTION GEL SYSTEM. ANCHOR RODS SHALL BE MANUFACTURED FROM:
      a. A-36 MATERIAL (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153).
      b. ASTM A 193, GRADE B-7 MATERIAL (ZINC PLATED IN ACCORDANCE WITH ASTM B 633, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153).
      c. A.I.S.I. 304 OR 316 STAINLESS STEEL, IN ACCORDANCE WITH ASTM F 593. ANCHOR RODS SHALL HAVE ROLLED THREADS. NUTS SHALL CONFORM WITH ASTM A 194. ACCEPTABLE ADHESIVE INJECTION GEL SYSTEMS ARE THE HILTI HIT-RE 500 V3, SIMPSON SET XP OR DEWALT 1000+. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
8. ANCHOR BOLT SHALL CONFORM WITH ASTM A 307, GRADE A, AND SHALL BE PROVIDED WITH STANDARD WASHERS AND NUTS. GALVANIZE EXTERIOR BOLTS. GALVANIZING SHALL BE IN ACCORDANCE WITH ASTM A 153, CLASS C. NUTS SHALL BE OVER-TAPPED TO CLASS 2A FIT BEFORE GALVANIZING, IN ACCORDANCE WITH ASTM A 563.
9. BOLT HEADS OR NUTS BEARING ON SLOPING FLANGES SHALL BE EQUIPPED WITH BEVELED WASHERS.
10. ERECTION AIDS (SUCH AS BOLTS, CLIPS, SHIMS, SEATS OR ANY OTHERS REQUIRED TO FACILITATE CONSTRUCTION) ARE THE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN AND PROVIDE.
11. ALL BRACING SHALL HAVE TWO (2) BOLT CONNECTIONS, UNLESS NOTED OTHERWISE. ALL CROSS BRACING SHALL BE BOLTED AT INTERSECTIONS WITH TWO (2) BOLT MINIMUM FOR ST AND ONE (1) BOLT FOR ANGLES. PROVIDE FILLER PLATE BETWEEN CROSS BRACES, AS REQUIRED.
12. ALL FIELD WELDS TO GALVANIZED STEEL AND AREAS DAMAGED BY WELDING, FLAME CUTTING OR HANDLING, SHALL BE REPAIRED WITH AN ORGANIC COLD GALVANIZING COMPOUND HAVING A MINIMUM OF NINETY-FOUR PERCENT (94%) ZINC DUST IN THE DRY FILM. APPLY IN MULTIPLE COATS, UNTIL AN 8 MIL THICKNESS HAS BEEN ACHIEVED. SURFACES TO RECEIVE ZINC-RICH PAINT SHALL BE CLEAN, DRY AND FREE OF OIL, GREASE, SALT AND CORROSION PRODUCTS.
13. ALL HAND RAILS SHALL BE 1 1/2" DIAMETER STEEL PIPE, STANDARD WEIGHT, HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 123.
14. STEEL LADDERS AND STAIRS SHALL BE CONSTRUCTED OF MEMBERS OF THE SIZES SHOWN. LADDERS AND STAIRS SHALL BE ALL-WELDED CONSTRUCTION, FINISHED SMOOTH AND NEAT. PROVIDE ANCHOR CLIPS AND ACCESSORIES, AS REQUIRED FOR COMPLETE INSTALLATION.
15. ALL EMBEDDED STEEL SHALL BE FABRICATED FROM MATERIAL CONFORMING WITH THE REQUIREMENTS OF ASTM A 36. HOT-DIP GALVANIZE IN ACCORDANCE WITH ASTM A 123, UNLESS NOTED OTHERWISE.
16. ALL DECK PLATE SHALL BE 1/4" DIAMOND OR CHECKERED PLATE, OR APPROVED, UNLESS NOTED OTHERWISE.
17. ALL FLOOR PLATING SHALL BE HOT-DIPPED GALVANIZED, IN ACCORDANCE WITH ASTM A 123.
18. STEEL FLOOR GRATING SHALL BE 1-1/4"x3/16" 19W4, UNLESS NOTED OTHERWISE. MATERIAL, FABRICATION, QUALITY ASSURANCE AND INSTALLATION SHALL COMPLY THE APPLICABLE PROVISIONS AND RECOMMENDATIONS OF THE N.A.A.M.M. METAL BAR GRATING MANUALS (N.N.S.I./N.A.A.M.M. MBG531 AND MBG532).
   A. STAIR TREADS SHALL BE 1-1/4"x3/16" 19W4 WITH CHECKERED PLATE NOSING.
   B. ALL EDGES SHALL BE BANDED. FOR EXTERIOR APPLICATIONS, SERRATED GRATING AND TREAD SHALL BE USED.
   C. ALL FLOOR GRATING SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM 123.
   D. FLOOR GRATING SHALL BE FASTENED TO FLOOR STEEL USING GRATING MANUFACTURER'S STAINLESS STEEL HOLDOWN CLIPS, IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
   E. ALL OPENINGS IN GRATING AT LEG SUPPORTS SHALL BE 1" LARGER THAN THE BASE PLATE DIMENSIONS, UNLESS NOTED OTHERWISE.
   F. PROVIDE PIPING OPENINGS IN GRATING AS REQUIRED. ALL OPENINGS THROUGH GRATING SHALL BE BANDED.

LIGHT GAUGE STEEL FRAMING:

1. STEEL IS TO BE:
   A. ASTM A 446 GRADE D (FY = 50 KSI) FOR 12, 14 AND 16 GAUGE.
   B. ASTM A 446 GRADE A (FY = 33 KSI) FOR 18 GAUGE AND LIGHTER.
   C. ASTM 653 SQ., GRADE A (FY = 33 KSI) FOR 18 GAUGE OR 20 GAUGE FOR SHEAR WALLS.
2. ALL FABRICATION, ERECTION AND IDENTIFICATION OF LIGHT GAUGE STEEL FRAMING SHALL CONFORM WITH OSSC SECTION 2210 AND AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
3. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS.
4. FASTENING OF COMPONENTS SHALL BE WITH WELDING.
   A. WELDS SHALL BE OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION.
   B. ALL WELDING SHALL CONFORM WITH A.W.S. SPECIFICATIONS. TOUCH UP WELDS WITH ZINCRICH PAINT.
   C. WELDERS SHALL BE A.W.S. CERTIFIED FOR LIGHT GAUGE METAL WELDING.
   D. WELDING ELECTRODES SHALL BE E6OXX.
5. STEEL STUDS OR JOISTS SHALL BE "S" STUDS OR "J" JOISTS WITH STIFFENED LIPS, UNLESS NOTED OTHERWISE, AS MANUFACTURED BY KNORR STEEL FRAMING SYSTEMS, MEMBERS OF THE METAL STUD MANUFACTURER'S ASSOCIATION, OR APPROVED. SIZE AND GAUGE ARE AS NOTED ON DRAWINGS.
6. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT NECESSARILY LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION.
7. END BLOCKING SHALL BE PROVIDED WHERE JOIST ENDS ARE NOT OTHERWISE RESTRAINED FROM ROTATION.
8. JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS, UNLESS NOTED OTHERWISE.
9. SPLICES IN AXIALLY LOADED STUDS SHALL NOT BE PERMITTED.
10. EACH FLANGE OF STUDS SHALL BE SECURELY ATTACHED TO FLANGES OF BOTH UPPER AND LOWER RUNNERS.
11. WHEN METAL STUDS ARE USED IN BEARING WALL CONSTRUCTION, STUDS MUST FIT TIGHTLY INTO THE TOP AND BOTTOM TRACKS. END GAPS WILL NOT BE ALLOWED.

DIVISION 06 - WOOD, PLASTICS AND COMPOSITES

FRAMING LUMBER:

1. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH AND SHALL BE GRADED UNDER THE MOST RECENTLY ADOPTED RULES OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB).
2. ALL BEAMS AND JOISTS SHALL BE NO. 2 MINIMUM, UNLESS INDICATED OTHERWISE ON THE PLANS.
3. ALL STUDS AND BLOCKING SHALL BE NO. 2.
4. ALL LUMBER IN CONTACT WITH CONCRETE OR EXPOSED SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 AND SHALL BEAR THE AWPA QUALITY MARK.
5. DOUBLE ALL JOISTS UNDER WALL PARTITIONS, AND PROVIDE BLOCKING BETWEEN JOISTS WHERE BEARING WALLS ARE PERPENDICULAR TO JOISTS.
6. ALL GLULAM BEAMS TO BE 24F-V4 TYPICAL. 24F-V8 FOR CANTILEVERED OR CONTINUOUS SPAN.
7. ALL LVL LUMBER TO BE MICROLAM LVL OR APPROVED EQUAL.

PLYWOOD SHEATHING:

1. ALL PLYWOOD SHALL BE C-D GRADE WITH EXTERIOR GLUE MANUFACTURED IN ACCORDANCE WITH THE UNITED STATES PRODUCT STANDARDS PS 1-83/ANSI A199.1 "FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD" AND SHALL CONFORM TO OSSC SECTION 2303 AND SHALL BEAR THE APA TRADEMARK OF THE APA.
2. PLYWOOD SHALL BE LAID WITH END JOINTS STAGGERED.
3. BLOCK ALL SHEAR WALL SHEATHING WITH 2x BLOCKING AT ALL EDGES..
4. EXTERIOR WALLS TO BE 7/16" EXPOSURE I, C-D PLY. OR OSB SHEATHING U.N.O. SEE PLANS FOR SHEAR WALL TYPE AND CORRESPONDENCE SHEAR WALL SCHEDULE FOR REQUIREMENTS.
5. OSB MAY BE SUBSTITUTED FOR PLYWOOD WITH SAME SPAN RATING.

NAILING AND FASTENERS:

1. NAILING INDICATED ON PLANS AND DETAILS ARE "COMMON" NAILS. MINIMUM FRAMING NAILING SHALL CONFORM TO OSSC TABLE 2304.10.1. SEE DETAILS FOR ADDITIONAL TYPICAL NAILING REQUIREMENTS. SUBSTITUTION OF NAILS OTHER THAN "COMMON" IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
2. POWER DRIVEN NAILS OTHER THAN "COMMON" NAILS MAY BE USED IF DATA IS SUBMITTED AND APPROVED PRIOR TO USE.
3. APPLY 1/4 DIAMETER CONTINUOUS BEAD OF GLUE TO TOPS OF WOOD FRAMED FLOOR JOISTS, BLOCKING, AND PLATES IMMEDIATELY PRIOR TO PLACEMENT OF FLOOR SHEATHING.
4. ALL BOLTED CONNECTIONS SHALL BE MADE WITH MACHINE BOLTS (M.B.) CONFORMING TO ASTM A307. ALL BOLTS AND LAGS SHALL BE INSTALLED WITH STANDARD WASHERS, UNLESS NOTED.
5. JOIST HANGERS, HOLDOWNS AND OTHER FRAMING ACCESSORIES ARE REFERRED TO ON PLANS BY PARTICULAR TYPE AS MANUFACTURED BY SIMPSON COMPANY, SAN LEANDRO, CA. ALL HARDWARE IS TO BE FASTENED PER MANUFACTURER'S SPECIFICATIONS, U.N.O.
6. ALL PLATES AND LEDGERS SHALL BE ANCHORED WITH A MINIMUM OF THREE FASTENERS PER PIECE.
7. EPOXY ANCHOR BOLTS AND ADHESIVE INDICATED ON DRAWINGS MAY BE SUBSTITUTED UPON CONTRACTORS REQUEST WITH E.O.R. APPROVED EQUAL. DEPTH OF EMBEDMENT SHALL BE AS PER MANUFACTURER SPECIFICATIONS, UNLESS NOTED OTHERWISE. INSTALL ALL EPOXY FASTENERS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS.
8. SILL AT WALLS SHALL BE BOLTED TO CONCRETE WITH 5/8" DIAMETER x 10" LONG ANCHOR BOLTS AT 4'-0" o.c. MAXIMUM AND WITHIN 1'-0" OF SILL PLATE ENDS, CORNERS OR SPLICES, UNLESS DETAILED OTHERWISE. STEEL PLATE WASHERS SHALL BE INSTALLED AT ALL ANCHOR BOLTS IN SHEAR WALLS, WITH MINIMUM DIMENSIONS OF 3" x 3" x 1/4" THICK, AND SHALL BE INSTALLED WITHIN 1/2" OF SHEATHING FACE. SIMPSON BPS BEARING PLATE OR APPROVED EQUAL MAY BE USED.

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ALSEA SCHOOL DISTRICT
301 S. 3RD ST.
ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY- NOT FOR CONSTRUCTION
EXPIRES: 06-30-26

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

STRUCTURAL GENERAL NOTES

S0.10

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

GENERAL - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FABRICATORS	1705.11 1704.2.5				SPECIAL INSPECTION IS REQUIRED FOR STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES FABRICATED ON THE PREMISES OF A FABRICATOR'S SHOP. SPECIAL INSPECTIONS SHALL BE PERFORMED DURING FABRICATION. PERFORMING SPECIAL INSPECTIONS IS NOT REQUIRED, WHERE FABRICATOR HAS BEEN APPROVED AS AN APPROVED FABRICATOR, PER SECTION 1704.2.5.1.
DEFERRED SUBMITTALS				X	SPECIAL INSPECTION REQUIREMENTS FOR DEFERRED SUBMITTAL ITEMS, INCLUDING REQUIREMENTS FOR DESIGNATED SEISMIC SYSTEMS IN ACCORDANCE WITH OSSC SECTION 1705.13.4 IF APPLICABLE, TO BE SPECIFIED BY THE SYSTEM ENGINEER AND INCLUDED WITH DEFERRED SUBMITAL DOCUMENTS.
SUBMITTALS TO THE BUILDING OFFICIAL	1704.5			X	CERTIFICATES OF COMPLIANCE, REPORTS OF PRE-CONSTRUCTION TESTS, OR REPORTS OF MATERIAL PROPERTIES SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.
FIBER-REINFORCED COMPOSITE SYSTEMS	1705.1.1	AC178		X	MATERIALS AND INSTALLATION SPECIAL INSPECTIONS PER ICC REPORT
POST INSTALLED MECHANICAL ANCHORS AND ADHESIVE ANCHORS IN HARDENED CONCRETE AND COMPLETED MASONRY				X	

SOILS/GEOTECHNICAL - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
SOILS					
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	1705.6	GEOTECHNICAL REPORT		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				X	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS				X	
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.			X		
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				X	

SOILS/GEOTECHNICAL - TESTING

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FILL IN-PLACE DENSITY OR PREPARED SUBGRADE DENSITY	1705.6	VARIES; GEOTECHNICAL REPORT OR MINIMUM PER OSSC 1803.5.5, 1803.5.8, WHICHEVER IS GREATER		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
MATERIAL VERIFICATION		VARIES; CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
TEST ELEMENTS	1705.6 1705.7		REFERENCE SPECIFICATIONS FOR PERFORMANCE VERIFICATION AND PROOF LOAD TESTING REQUIREMENTS		BY THE GEOTECHNICAL ENGINEER

CONCRETE - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
GENERAL	1705.3 1901.6	ACI 318: 26.13			SPECIAL INSPECTIONS OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1705.3 OF THE IBC AND SECTION 26.13 OF ACI 318.
REINFORCING STEEL AND PLACEMENT	1901.5	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3		X	REINFORCING TO COMPLY WITH ALL CODE PROTECTION, SPACING AND TOLERANCE LIMITS.
INSPECT ANCHORS/BOLTS CAST IN CONCRETE	1705.3	ACI 318: 17.2.5 26.13		X	ALL CAST-IN-PLACE ANCHORS/BOLTS SHALL BE VISUALLY INSPECTED. REFERENCE STEEL INSPECTIONS FOR ADDITIONAL INSTALLATION, MATERIAL AND WELDING INSPECTIONS OF STEEL ITEMS EMBEDDED IN CONCRETE (HEADED STUDS, DBA's, ETC.)
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1904.1 1904.2 1908	ACI 318: CH. 19, 26.4.3, 26.4.4		X	
CONCRETE SPECIMENS FOR TESTING		ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	X		PRIOR TO CONCRETE PLACEMENT, FABRICATE CONCRETE SPECIMENS FOR TESTING. SEE THE CONCRETE TESTING TABLE FOR ADDITIONAL INFORMATION.
CONCRETE/SHOTCRETE PLACEMENT, NON-SHRINK GROUT	1908	ACI 318: 26.5, 26.13.3.2(a)	X		
CONCRETE/SHOTCRETE CURING	1908.1	ACI 318: 26.5.3 - 26.5.5		X	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURES AND TECHNIQUES
VERIFICATION OF IN-SITU CONCRETE PRIOR TO REMOVAL OF FORMS AND SHORES FROM ELEVATED BEAMS AND SLABS		ACI 318: 26.11.2		X	
VERIFICATION OF FORMWORK		ACI 318: 26.11.1.2(b), 26.13.3.3		X	SPECIAL INSPECTIONS APPLY TO SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED
EMBEDDED ITEMS IN CONCRETE				X	ALL NON-STRUCTURAL EMBEDDED ITEMS, SUCH AS CONDUITS, PIPES AND SLEEVES, SHALL BE REVIEWED FOR CONFORMANCE WITH STRUCTURAL DOCUMENTS FOR SIZE, SPACING, LOCATION, EDGE DISTANCE AND TRIM REINFORCING.
REINFORCING STEEL MECHANICAL COUPLERS, TERMINATORS AND FORMSAVERS		ICC EVALUATION REPORTS		X	

CONCRETE - TESTING

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)	REMARKS
CONCRETE STRENGTH	1705.3 ASTM C172 ASTM C 31 ACI 318 26.12, ACI 318 26.5	ASTM C39	EACH 150 CY NOR LESS THAN EACH 5000 SF OF SLAB OR WALL PLACED EACH SHIFT	FABRICATE SPECIMENS AT TIME FRESH CONCRETE IS PLACED
CONCRETE SLUMP		ASTM C143		
CONCRETE AIR CONTENT		ASTM C231		
CONCRETE TEMPERATURE		ASTM C1064		
SHOTCRETE STRENGTH	1705.3 1908.1	ASTM C42 ASTM C1140	EACH 50 CY NOR LESS THAN EACH 5000 SF OF WALL PLACED EACH SHIFT	UNREINFORCED SPECIMEN TAKEN FROM THE IN-PLACE OR FROM TEST PANELS
SHOTCRETE TEST PANEL	1705.3 1908.1	ACI 506.2 ASTM C 1140	PANELS SHALL BE PROVIDED FOR EACH NOZZLEMAN, MIX DESIGN AND SHOT ANGLE USED ON THE PROJECT	PANEL SIZE AND QUANTITY SHALL BE AS REQUIRED TO OBTAIN REQUIRED CONFORMING TEST CYLINDERS AND MINIMUM SIZES PER PROJECT AND CODE REQUIREMENTS

STEEL - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	INSPECTION (NOTES 5 AND 6)		REMARKS
			CONTINUOUS/ PERFORM	PERIODIC/ OBSERVE	
CONTRACTOR QUALITY CONTROL REQUIREMENTS		AISC 360 CHAPTER N	X	X	CONTRACTOR TO PROVIDE QUALITY CONTROL FOR ALL ITEMS INDICATED TO BE OBSERVED AND/OR PERFORMED IN TABLE BELOW
STEEL FABRICATION					
FABRICATION OF STRUCTURAL ELEMENTS	1704.2.5	AISC 360		X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL COMPONENTS	1705.2.1 TABLE 1705.2-3	ASTM A6 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS AISC 360 A3.1 AISC 360 N2.1		X	CERTIFIED MILL TEST REPORTS
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS		AISC 360 A3.4 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1705.2.1.1 TABLE 1705.2-5	AISC 360 A3.5 AISC 360 N3.2 APPLICABLE AWS A5 DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
STRUCTURAL STEEL WELDING					
VERIFYING USE OF PROPER WPS'S	1705.2.1 AWS D1.1	AISC 360 N3.2			RETAIN A RECORD OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS	1705.2	AWS D1.1		X	RETAIN A RECORD OF QUALIFICATION CARDS
COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS	TABLE 1705.2-6a	AWS D1.1 CLAUSE 6	X		
MULTIPASS FILLET WELDS			X		
SINGLE PASS FILLET WELDS GREATER THAN 5/16"			X		ALL WELDS VISUALLY INSPECTED PER AWS D1.16.9
PLUG AND SLOT WELDS			X		
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"				X	

LIGHT GAUGE AND OTHER STEEL - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
GENERAL					
IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	1705.2.2 1705.2.3 1705.2.4 TABLE 1705.2-4	APPLICABLE ASTM STANDARDS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION: REQUIRED SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE					
SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, BRACES, DIAPHRAGMS, COLLECTORS (DRAG STRUTS) AND HOLDOWNS	1705.13.3	AWS D1.3		X	EXCEPTION: NOT REQUIRED FOR SHEAR WALLS, BRACES, DIAPHRAGMS, DRAG STRUT AND HOLDOWNS, IF SHEATHING IS GYP OR FIBERBOARD OR IF SHEATHING IS WOOD STRUCTURAL PANEL/STEEL SHEET ONE SIDE ONLY AND FASTENER SPACING IS GREATER THAN 4" ON CENTER

WOOD - SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
WOOD - REQUIRED SEISMIC RESISTANCE INSPECTIONS					
CONNECTIONS FOR DIAPHRAGM CHORDS, COLLECTORS, BRACING, AND SHEAR WALL ANCHORAGE AND HOLDOWNS	1705.13.2.2			X	ALL FASTENERS/CONNECTIONS VISUALLY INSPECTED
FASTENING OF DIAPHRAGM AND SHEAR WALL SHEATHING WITH EDGE NAILING ≤ 4"	1705.13.2			X	FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS. THIS INCLUDES NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO OTHER COMPONENTS IN THE SEISMIC FORCE RESISTING SYSTEM

STATEMENT OF SPECIAL INSPECTION NOTES:

1. INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2022 OSSC. CONTRACT DOCUMENTS AND APPROVED SUBMITTALS. REFER TO SPECIAL INSPECTION AND TESTING TABLES FOR PROJECT REQUIREMENTS.
2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER ARCHITECT A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1 (1) OF AWS D1.1.
3. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.
4. THE SPECIAL INSPECTOR AND GEOTECHNICAL ENGINEER SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, ARCHITECT, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.
5. QUALITY ASSURANCE (QA) IS REQUIRED FOR STRUCTURAL STEEL ITEMS PER AISC 360 AND 341 UNLESS SPECIFICALLY NOTED OTHERWISE. QUALITY CONTROL (QC) TO BE PROVIDED BY THE FABRICATOR, ERECTOR OR OTHER RESPONSIBLE CONTRACTOR AS APPLICABLE. CONTRACTOR AND SPECIAL INSPECTOR TO DOCUMENT QUALITY CONTROL AS REQUIRED IN AISC 360 SECTION N3 AND AISC 341 SECTION J2.
6. **INSPECTION TYPES:**
   - CONTINUOUS : THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
   - PERIODIC : THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
   - OBSERVE : OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.
   - PERFORM : INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.
7. PERFORM INSPECTION PRIOR TO FINAL ACCEPTANCE OF THE ITEM FOR TEN WELDS TO BE MADE BY A GIVEN WELDER. WITH THE WELDER DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEMS, THE PERFORM DESIGNATION OF THIS TASK SHALL BE REDUCED TO OBSERVE, AND THE WELDER SHALL PERFORM THIS TASK. SHOULD THE INSPECTOR DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK, THE TASK SHALL BE RETURNED TO PERFORM UNTIL SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE INSPECTION TASKS LISTED.
8. SPECIAL INSPECTION OF MECHANICAL POST INSTALLED ANCHORS SHALL BE IN STRICT CONFORMANCE WITH THE ICC REPORT AND MANUFACTURER'S INSTALLATION REQUIREMENTS. ANCHOR INSTALLERS SHALL BE QUALIFIED AS REQUIRED BY JURISDICTION REQUIREMENTS.
   - INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.
   - SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE ANCHORS WERE INSPECTED PER APPROVED ANCHOR EVALUATION REPORT.
9. TESTING ABBREVIATIONS:
   A. NDT - NON-DESTRUCTIVE TESTING
   B. C.J.P. - COMPLETE JOINT PENETRATION
   C. MT - MAGNETIC PARTICLE TESTING
   D. RBS - REDUCED BEAM SECTION
10. DOCUMENT (D): INDICATES CONTRACTOR AND SPECIAL INSPECTOR TO PROVIDE DOCUMENTATION IN ACCORDANCE WITH AISC 341.

CONTRACTOR RESPONSIBILITY:

EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF THE MAIN WIND-OR SEISMIC-FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND-OR SEISMIC-RESISTING COMPONENT LISTED THE TABLES SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.

1. ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
2. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND DISTRIBUTION OF THE REPORTS.
3. IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

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ALSEA SCHOOL DISTRICT
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ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

PRELIMINARY; NOT FOR CONSTRUCTION
EXPIRES: 06-30-25

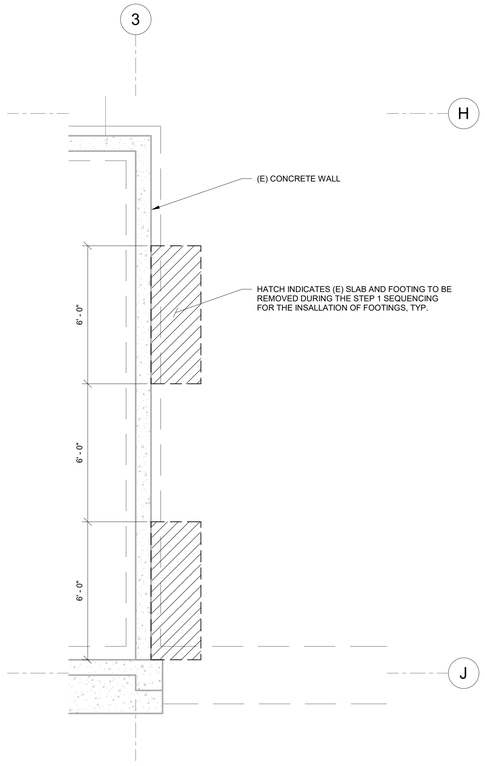
△	DESCRIPTION	DATE:

PROJECT NO. P-2935-24
DRAWN: DKS / JDH
CHECKED: KDM
DATE: 12-03-24

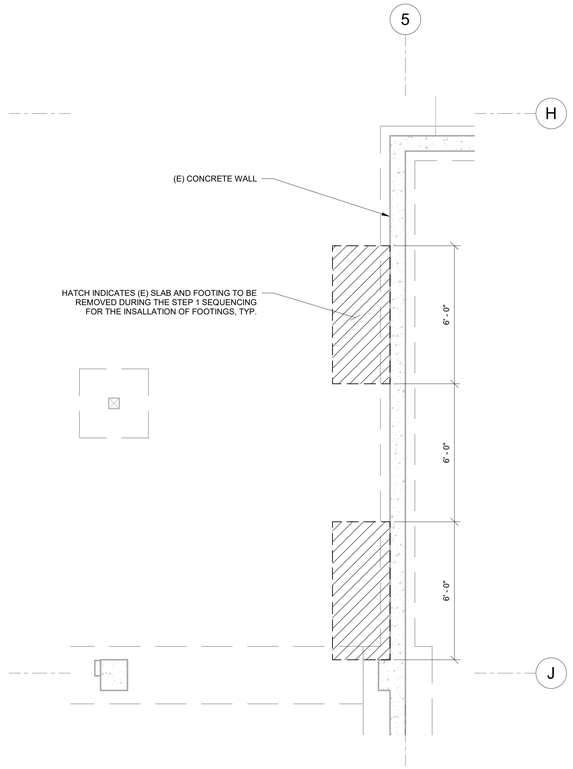
SPECIAL INSPECTIONS AND TESTING

S0.20

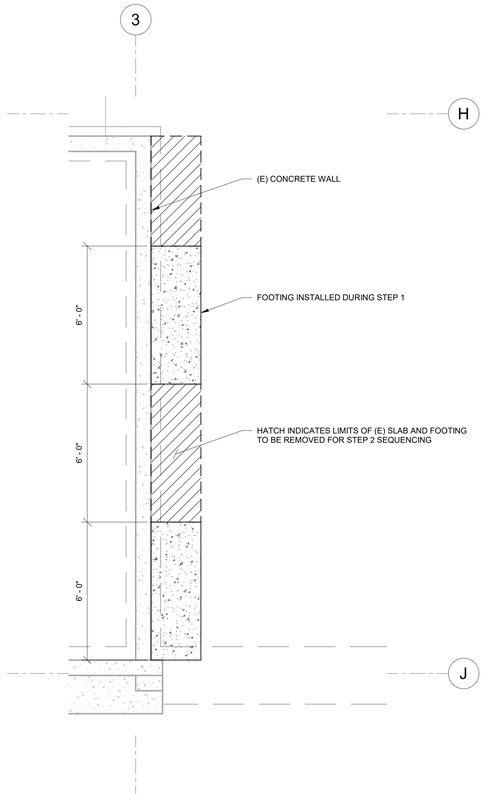
100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



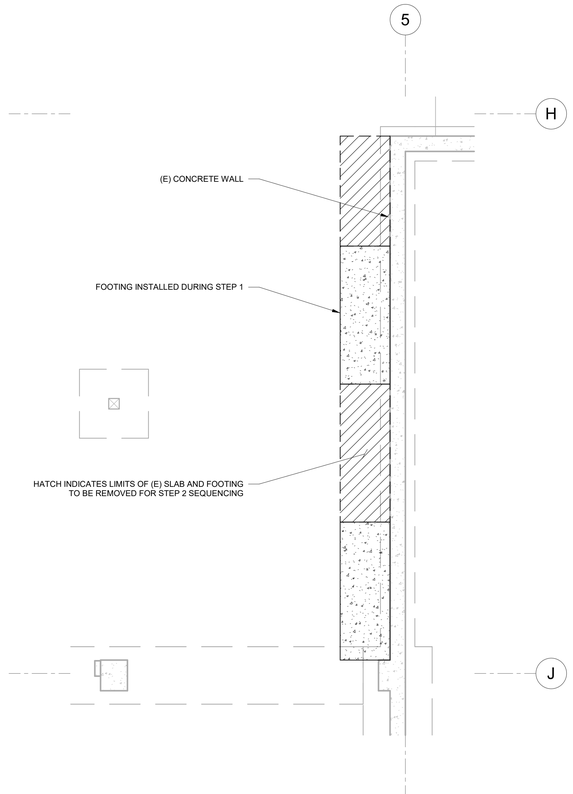
1 S0.30 BOYS LOCKER ROOM
STEP 1 SLAB REMOVAL & SOIL EXCAVATION SEQUENCING PLAN
3/8" = 1'-0"



2 S0.30 GIRLS LOCKER ROOM
STEP 1 SLAB REMOVAL & SOIL EXCAVATION SEQUENCING PLAN
3/8" = 1'-0"



3 S0.30 BOYS LOCKER ROOM
STEP 2 SLAB REMOVAL & SOIL EXCAVATION SEQUENCING PLAN
3/8" = 1'-0"



4 S0.30 GIRLS LOCKER ROOM
STEP 2 SLAB REMOVAL & SOIL EXCAVATION SEQUENCING PLAN
3/8" = 1'-0"

STEP 1 SEQUENCING FOR SLAB REMOVAL & SOIL EXCAVATION

1. PROVIDE ALL NECESSARY TEMPORARY SHORING AS REQUIRED TO SUPPORT THE STRUCTURE ABOVE.
2. REMOVE THE 6' PERIMETER SLAB SEGMENTS AS SHOWN ON STEP 1 PLAN WHILE LEAVING THE ADJACENT SLAB SECTIONS ON BOTH SIDES AND THE INTERIOR SLABS AS SHOWN.
3. INSTALL CONCRETE FOOTINGS AT THE 6' SEGMENTS PER STEP 1 BEFORE STEP 2.
4. PROVIDE COUPLERS AT DISCONTINUOUS REBAR IN FOOTINGS POURED IN STEP 1.
5. CONTINUE TO STEP 2 SEQUENCING FOR THE REMAINDER OF THE SLAB REMOVAL AND SOIL EXCAVATION.

STEP 2 SEQUENCING FOR SLAB REMOVAL & SOIL EXCAVATION

1. PROVIDE ALL NECESSARY TEMPORARY SHORING AS NEEDED FOR THE BASEMENT SLAB REMOVAL.
2. ONCE THE FOOTINGS FROM STEP 1 HAVE BEEN INSTALLED AND HAVE REACHED THEIR DESIGN STRENGTH, SHORING OF (E) CONCRETE WALL MAY BE REMOVED.
3. REMOVE THE REMAINING PERIMETER CONCRETE SLAB AS SHOWN ON STEP 2 PLAN.
4. INSTALL THE REMAINING FOOTINGS AND ASSOCIATED COMPONENTS.

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SLAB REMOVAL & SOIL EXCAVATION SEQUENCING

S0.30

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

ONE INCH EQUALS FULL SCALE

FOUNDATION SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCEMENT	NOTES
F-1	2' - 0"	CONT.	1' - 6"	(3) #5 LONGITUDINAL BARS	
F-2	1' - 4"	CONT.	1' - 6"	(2) #5 LONGITUDINAL BARS	
F-3	3' - 0"	CONT.	1' - 6"	(4) #5 BARS TOP & BOTTOM w/ #4 TRANSVERSE BARS @ 18" o.c.	SEE S0.30 FOR SEQUENCING REQUIREMENTS

1. UNLESS NOTED OTHERWISE, REINFORCEMENT TO BE SPACED EVENLY IN FOOTING WITH CORRECT SIDE AND BOTTOM CLEARANCES.

HOLDOWN SCHEDULE

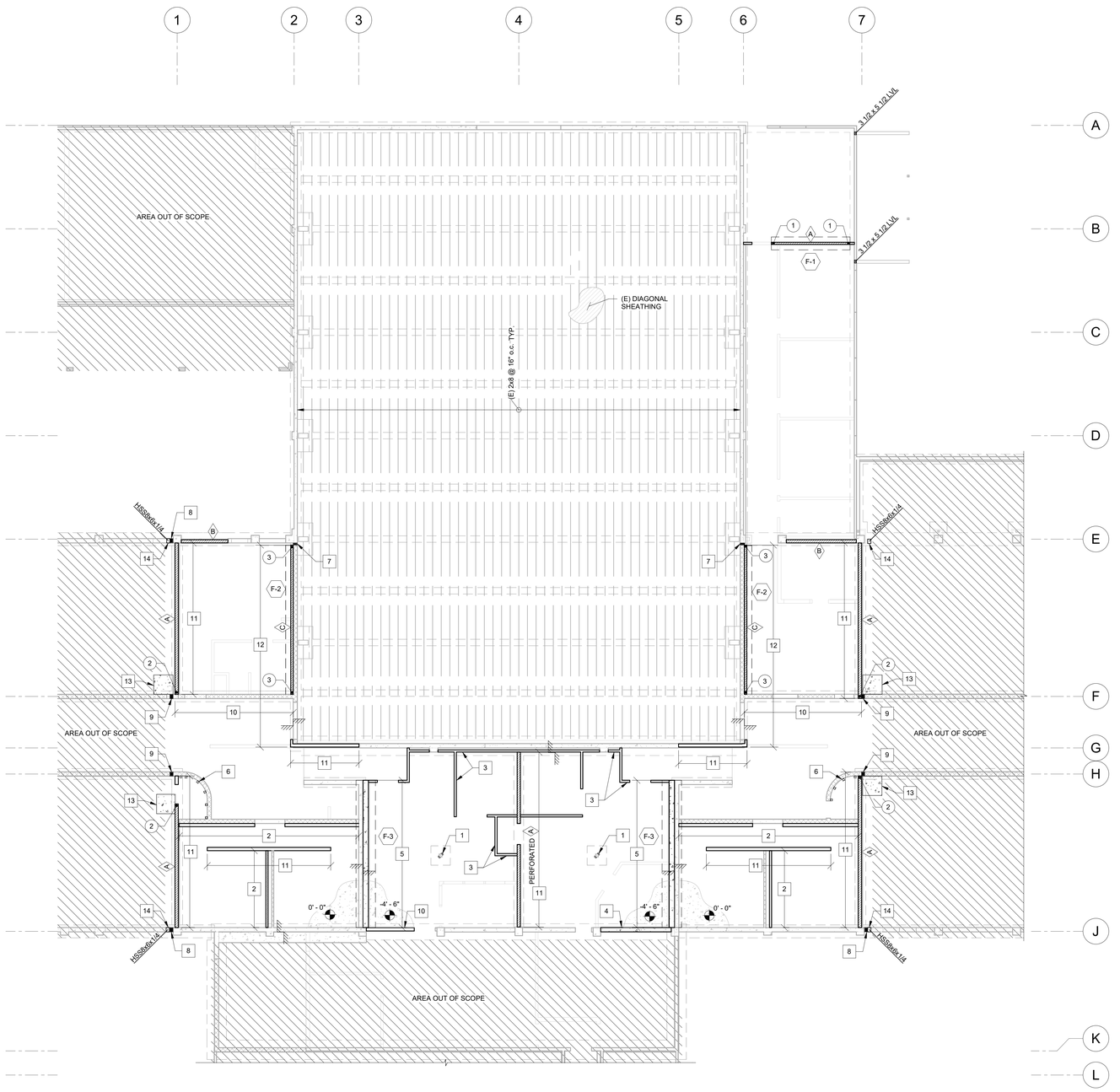
(#)	SIMPSON MODEL	POST THICKNESS	FASTENER	ANCHOR	REMARKS
1	HTT4	2x	(18) 0.162" DIA. x 1 1/2"	5/8" DIA.	PL1/4x3x0'-3" w/ DBL NUT AT BOT. OF THREADED ROD
2	HTT4	4x	(18) SD #10 x 1 1/2	5/8" DIA.	THRU BOLT w/ GALVANIZED PL6x6x1/4 WASHER
3	HTT4	DOUBLE 400S200-54	(18) #10 SCREWS	5/8" DIA.	PL 1/4x3x0'-3" w/ DBL NUT AT BOT. OF THREADED ROD

1. NAILS ARE TO BE COMMON WIRE NAILS, U.N.O.
2. HARDWARE IS TO BE SIMPSON, U.N.O.
3. HOLDOWN HARDWARE CAN BE EXTENDED WITH A307 THREADED ROD AND COUPLER.
4. ALIGN ALL HOLDOWNS FOR THE FULL HEIGHT OF STRUCTURE.
5. ALL HARDWARE TO BE INSTALLED PER MANUFACTURE'S SPECIFICATIONS.
6. HOLDOWN ANCHOR BOLTS ARE IN ADDITION TO TYPICAL SILL PLATE ANCHOR BOLTS.
7. EXTEND THREADED ROD TO WITHIN 3" CLEAR OF BOTTOM OF FOOTING.

SHEAR WALL SCHEDULE

(X)	SHEATHING	NAIL SIZE (LENGTH, SHANK DIA., HEAD DIA.)	EDGE NAILING (o.c.)	FIELD NAILING (o.c.)	PANEL EDGE STUDS	TOP PLATE A35 (o.c.)	BOTTOM PLATE DIMENSION	SILL PLATE ANCHORAGE (o.c.)	REMARKS
A	7/16" APA RATED	2 1/2"x0.131"x0.281"	6"	12"	2x	2'-0"	2x	THD62600 @ 4' - 0" o.c.	
B	7/16" APA RATED	2 1/2"x0.131"x0.281"	4"	12"	2x	1'-6"	2x	THD62600 @ 3' - 0" o.c.	
C	7/16" APA RATED OSB	#8 SCREWS	4"	12"	DOUBLE 400S200-54	2'-0"	400T200-54	THD62600 @ 3' - 0" o.c.	

1. ALL PLYWOOD TO BE APA RATED STRUCTURAL 1 EXTERIOR SHEATHING
2. ALL NAILS TO BE COMMON OR GALVANIZED BOX TYPE.
3. FLOOR AND ROOF DIAPHRAGMS TO BE NAILED WITH 3"x0.148"ø NAILS @ 6" o.c. EDGE NAILING AND 12" ON CENTER FIELD NAILING U.N.O. USE PLYWOOD THICKNESS AS INDICATED ON PLAN.
4. ATTACH RIM JOIST AND / OR BLOCKING TO SHEAR WALL AS INDICATED IN TABLE ABOVE.
5. ALL WALL SHEATHING TO EXTEND FULL HEIGHT OF WALL, TOP PLATE TO BOTTOM PLATE.
6. ALL SHEAR WALLS AND HOLDOWNS MUST HAVE CONTINUOUS LOAD PATH TO FOUNDATION.
7. USE WASHER PL1/4x3x0'-3" TYPICAL AT ALL ANCHOR BOLTS.
8. WHERE TOP PLATE FASTENING IS LESS THAN 12" o.c., USE MINIMUM BLOCKING OF 2 1/2" MANUFACTURED LUMBER (MICROLLAM LVL, OR PARALLAM PSL).
9. ALL SHEAR WALLS TO BE FULLY BLOCKED U.N.O. BLOCKING TO MATCH REQUIREMENTS FOR PANEL EDGE STUDS.
10. FOR SHEAR WALLS w/ STUDS SPACED @ 24" o.c. MAX. INSTALL SHEATHING WITH LONG DIMENSION ACROSS STUDS.



1 S1.10 FOUNDATION PLAN
1/8" = 1'-0"

FOUNDATION AND FRAMING PLAN NOTES

A. DIMENSIONS SHOWN ARE FOR REFERENCE ONLY, CONFIRM w/ ARCHITECTURAL PLAN & DETAILS.

B. BOTTOM OF FOOTINGS TO BE PLACED BELOW FROST DEPTH OR AS NOTED IN THE GEOTECHNICAL REPORT, WHICHEVER IS GREATER.

C. COORDINATE PENETRATIONS OF SITE UTILITIES, MECHANICAL DUCTS, PIPING, AND ELECTRICAL CONDUIT/PANELS TO MINIMIZE IMPACT TO STRUCTURAL FRAMING. PLUMBING FIXTURES SHOWN ON FLOOR FOR REFERENCE AND POSSIBLE FRAMING CONFLICTS ONLY.

D. ALL FOOTINGS ARE TO BE CENTERED UNDER COLUMNS U.N.O.

E. ALL FOOTINGS TO BEAR OVER GRADE OVER FIRM, UNDISTURBED, NON-ORGANIC, NON-EXPANSIVE NATIVE MATERIAL, OR STRUCTURAL FILL AS REQUIRED PER GEOTECHNICAL REPORT.

F. SEE SHEET S0.10 FOR ALL NOTES.

G. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.

H. INDICATES FLOOR STEP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.

I. S S INDICATES STEP IN FOOTING. SEE DETAIL X/SX.X FOR ADDITIONAL INFORMATION.

J. INDICATES (E) CMU WALL.

K. INDICATES (E) CAST IN PLACE CONCRETE WALL.

L. INDICATES CONCRETE WALL INFILL.

M. (X) INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

N. INDICATES SHEAR WALL LOCATION ABOVE FOUNDATION. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

O. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.

P. (#) INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.

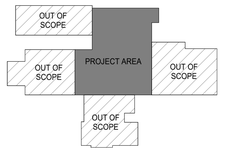
Q. X'-X" INDICATES TOP OF SLAB ELEVATION.

R. (F-XX) INDICATES FOOTING TYPE PER FOUNDATION SCHEDULE.

(#) FOUNDATION PLAN KEYNOTES

1. PROVIDE (2) SIMPSON RPBZ POST BASE w/ 3/8"ø TITEN HD ANCHORS w/ 3" MIN. EMBED.
2. DOUBLE 350S200-54 CFS STUD @ 16" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES AT 16" HORIZONTAL AND 24" o.c. VERTICAL GRID. ATTACH TO (E) SLAB w/ SIMPSON THDB25234H @ 3'-0" o.c.
3. DEMO (E) URM PARTITION WALL & REPLACE WITH 2x4 @ 16" o.c. STUD WALL w/ SINGLE P.T. BOTTOM PLATE AND DOUBLE TOP PLATE.
4. 8" CONCRETE INFILL w/ #5 BARS @ 16" o.c. VERTICAL AND #5 BARS @ 12" o.c. HORIZONTAL.
5. NEW 8" THICK REINFORCED SHOTCRETE WALL w/ #5 REBAR @ 12" o.c. E.W., ANCHOR REINFORCEMENT TO (E) CMU WALL w/ PYTHON ANCHORS @ 24" o.c. VERTICAL & HORIZONTAL GRID.
6. DOUBLE 350S200-54 CFS STUD @ 24" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES @ 16" o.c. HORIZONTAL AND 24" o.c. VERTICAL GRID. TRIM STUD FLANGES & EXTEND WEB w/ 90° BEND, ATTACH EA. STUD TO (E) SLAB w/ SIMPSON THDB25234H.
7. SAWCUT 3" OF FULL HEIGHT (E) CMU ADJACENT TO (E) CONCRETE WALL. TERMINATE NEW CFS STUD WALL 3" AWAY FROM ADJACENT CONCRETE WALL.
8. DEMO 8" OF (E) CONCRETE WALL TO PROVIDE VERTICAL SEISMIC ISOLATION.
9. DEMO 6" OF (E) CMU WALL TO PROVIDE VERTICAL SEISMIC ISOLATION.
10. FIBER-REINFORCED POLYMER (FRP) STRENGHTENING OF (E) CMU WALL BOTH SIDES OF THE WALL w/ VERTICAL STRIPS @ 4'-0" o.c.
11. DEMO (E) CMU WALL & REPLACE w/ 2x6 STUD WALL @ 16" o.c. w/ DOUBLE TOP PLATE & SINGLE P.T. BOTTOM PLATE.
12. DOUBLE 400S200-54 CFS STUD @ 16" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES AT 16" o.c. HORIZONTAL AND 24" o.c. VERTICAL GRID. ATTACH TO (E) SLAB w/ SIMPSON THDB25234H @ 3'-0" o.c..
13. HATCH INDICATES EXTENTS OF (E) SLAB DEMO FOR INSTALLATION OF HOLDOWN. PATCH BACK SLAB TO MATCH (E) THICKNESS.
14. PROVIDE 1/2" THICK A36 BASEPLATE w/ (2) SIMPSON THDB62600H ANCHORS. ATTACH COLUMN TO WALL w/ (3) EQUALLY SPACED SIMPSON THD50400H ANCHORS.

KEY PLAN



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ALSEA GYM SEISMIC RETROFIT

PRELIMINARY - NOT FOR CONSTRUCTION

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

FOUNDATION PLAN

S1.10

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

SHEAR WALL SCHEDULE

X	SHEATHING	NAIL SIZE (LENGTH, SHANK DIA., HEAD DIA.)	EDGE NAILING (o.c.)	FIELD NAILING (o.c.)	PANEL EDGE STUDS	TOP PLATE A35 (o.c.)	BOTTOM PLATE DIMENSION	SILL PLATE ANCHORAGE (o.c.)	REMARKS
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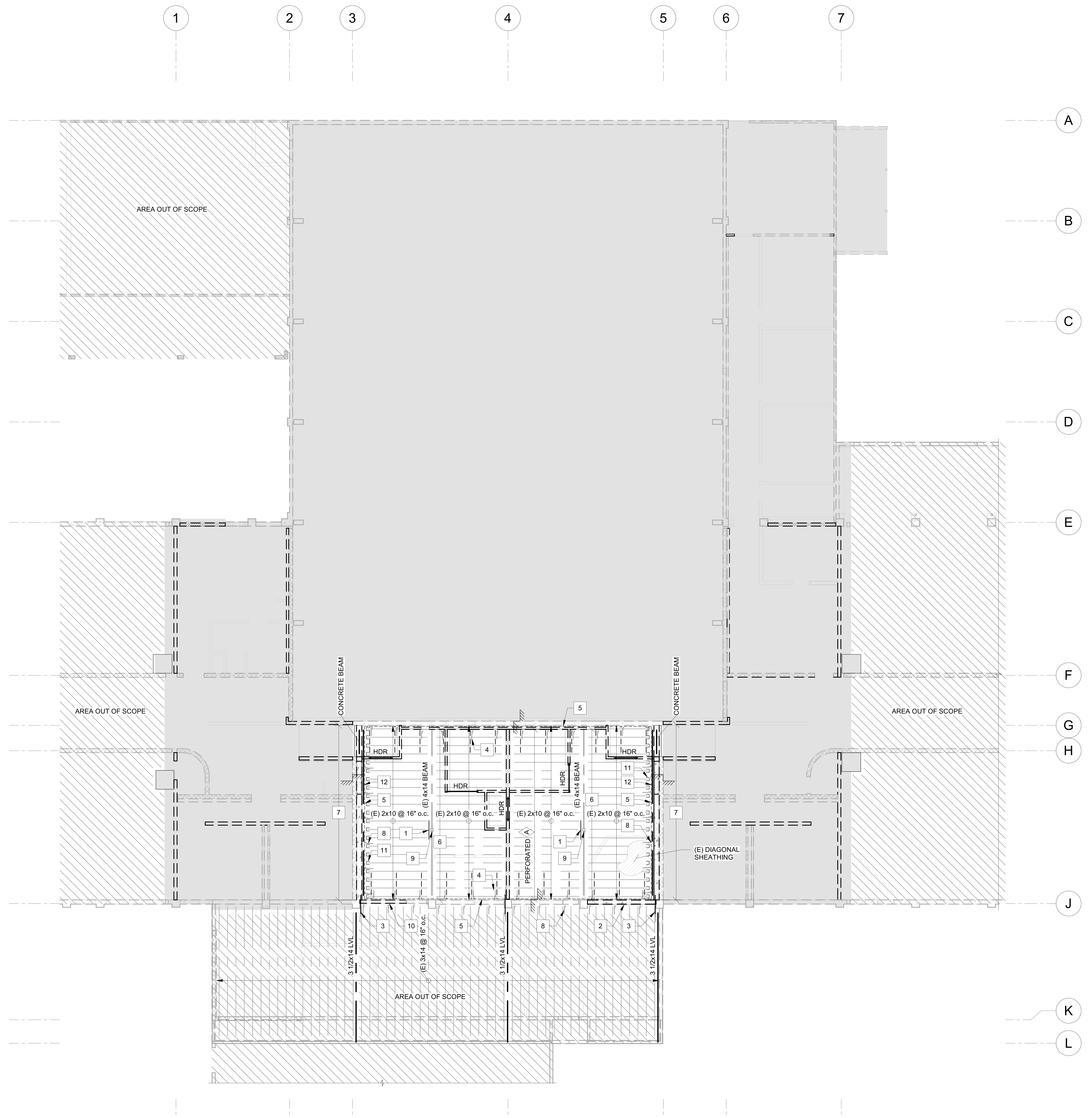
B	7/16" APA RATED	2 1/2"x0.131"x0.281"	4"	12"	2x	1'-6"	2x	THD62600 @ 3' - 0" o.c.	
C	7/16" APA RATED OSB	#8 SCREWS	4"	12"	DOUBLE 400S200-54	2'-0"	400T200-54	THD62600 @ 3' - 0" o.c.	

1. ALL PLYWOOD TO BE APA RATED STRUCTURAL 1 EXTERIOR SHEATHING
2. ALL NAILS TO BE COMMON OR GALVANIZED BOX TYPE.
3. FLOOR AND ROOF DIAPHRAGMS TO BE NAILED WITH 3"x0.148"Ø NAILS @ 6" o.c. EDGE NAILING AND 12" ON CENTER FIELD NAILING U.N.O. USE PLYWOOD THICKNESS AS INDICATED ON PLAN.
4. ATTACH RIM JOIST AND / OR BLOCKING TO SHEAR WALL AS INDICATED IN TABLE ABOVE.
5. ALL WALL SHEATHING TO EXTEND FULL HEIGHT OF WALL, TOP PLATE TO BOTTOM PLATE.
6. ALL SHEAR WALLS AND HOLDOWNS MUST HAVE CONTINUOUS LOAD PATH TO FOUNDATION.
7. USE WASHER PL1/4x3x0'-3" TYPICAL AT ALL ANCHOR BOLTS.
8. WHERE TOP PLATE FASTENING IS LESS THAN 12" o.c., USE MINIMUM BLOCKING OF 2 1/2" MANUFACTURED LUMBER (MICROLLAM LVL, OR PARALLAM PSL).
9. ALL SHEAR WALLS TO BE FULLY BLOCKED U.N.O. BLOCKING TO MATCH REQUIREMENTS FOR PANEL EDGE STUDS.
10. FOR SHEAR WALLS w/ STUDS SPACED @ 24" o.c. MAX. INSTALL SHEATHING WITH LONG DIMENSION ACROSS STUDS.



1 S1.20 STAGE & LIBRARY FLOOR FRAMING
1/8" = 1'-0"

FLOOR FRAMING PLAN NOTES

A. COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.

B. SEE SHEET S0.10 FOR ALL NOTES.

C. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.

D. INDICATES ROOF STEP, TYP. SEE ARCHITECTURAL PLANS FOR ADD'L. INFORMATION.

E. BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.

F. BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.

G. INDICATES (E) CMU WALL BELOW FRAMING.

H. INDICATES (E) CAST IN PLACE CONCRETE WALL.

I. INDICATES CONCRETE INFILL WALL.

J. X INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

K. INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

L. ALL SHEAR WALLS INDICATED AS "PERFORATED" THE CONTRACTOR SHALL PROVIDE NAILING PATTERN AROUND ALL WALL PENETRATIONS AS CALLED OUT ON FRAMING PLANS IN CORRESPONDENCE WITH THE SHEAR WALL SCHEDULE.

M. # INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.

N. HDR TYPICAL EXTERIOR HEADER IS 4x8 D.F. NO.2 USE 1 KING STUD AND 1 TRIMMER UNLESS NOTED OTHERWISE. TYPICAL INTERIOR HEADER TO BE (2) - 2x6 D.F. NO.2 WITH SINGLE TRIMMER AND SINGLE KING STUD (U.N.O.).

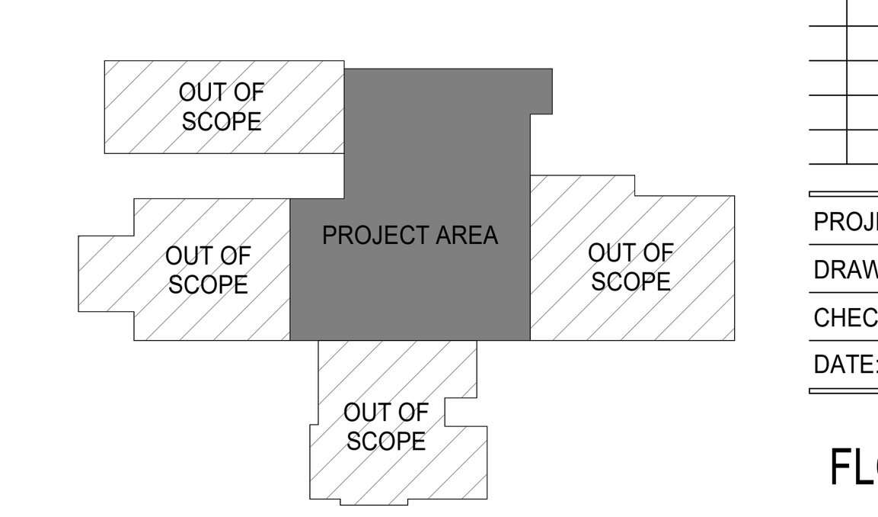
O. INDICATES NEW 2xX @ 16" o.c. PARTITION WALL BELOW FRAMING.

P. INDICATES SEISMIC ISOLATION JOINT. SEE DEMO PLANS FOR EXTENTS OF DEMOLITION.

FLOOR FRAMING PLAN KEYNOTES

1. SIMPSON CS14 DRAG STRAP w/ (26) 0.148"Ø x 2 1/2" NAILS AND MIN. 15" END LENGTH.
2. 8" CONCRETE INFILL w/ #5 BARS @ 16" o.c. VERTICAL AND #5 BARS @ 12" o.c. HORIZONTAL.
3. SIMPSON HTT4 DRAG CONNECTION w/ (18) SD#10x1 1/2" FASTENERS AND 5/8" Ø A307 THREADED ROD.
4. 2x FULL DEPTH BLOCKING w/ SIMPSON HTT4 TENSION TIE w/ (18) 0.148" Ø x 1 1/2" NAILS AND SIMPSON CS14 STRAP w/ 0.148" Ø x 2 1/2" NAILS @ 4'-0" o.c. DRILL & BOND 5/8" Ø A307 THREADED ROD w/ SIMPSON SET-3G EPOXY, MIN. 4" EMBEDMENT.
5. SIMPSON THDB62600H @ 4'-0" o.c.
6. PROVIDE FULL DEPTH BLOCKING EACH BAY AT (E) BEAM w/ SIMPSON A35 CLIPS @ 32" o.c.
7. NEW 8" THICK REINFORCED SHOTCRETE WALL w/ #5 REBAR @ 12" o.c. E.W.. ANCHOR REINFORCEMENT TO (E) CMU WALL w/ PYTHON ANCHORS @ 24" o.c. VERTICAL & HORIZONTAL GRID.
8. SIMPSON HTT4 @ 4'-0" o.c.. DRILL & BOND 5/8"Ø A307 THREADED ROD w/ SIMPSON SET-3G EPOXY, MIN. 6" EMBEDMENT.
9. PROVIDE SIMPSON A34 EACH SIDE FROM (E) BEAM TO COLUMN.
10. DEMOLISH (E) CMU WALL. PROVIDE NEW WOOD STUD WALL w/ 2x6 STUDS @ 16" o.c. w/ DOUBLE TOP PLATE AND SINGLE BOTTOM PLATE.
11. SIMPSON JB210A HANGER AT EA. (E) JOIST.
12. 3x10 P.T. LEDGER. ATTACH TO CONCRETE WALL w/ SIMPSON THDT75600H @ 6" o.c.

KEY PLAN



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ALSEA SCHOOL DISTRICT
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ALSEA GYM SEISMIC RETROFIT

-PRELIMINARY- NOT FOR CONSTRUCTION

DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

FLOOR FRAMING PLAN

S1.20

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION

SHEAR WALL SCHEDULE								
⟨X⟩	SHEATHING	NAIL SIZE (LENGTH, SHANK DIA., HEAD DIA.)	EDGE NAILING (o.c.)	FIELD NAILING (o.c.)	PANEL EDGE STUDS	TOP PLATE A35 (o.c.)	BOTTOM PLATE DIMENSION	SILL PLATE ANCHORAGE (o.c.)	REMARKS
A	7/16" APA RATED	2 1/2"x0.131"x0.281"	6"	12"	2x	2'-0"	2x	THD62600 @ 4' - 0" o.c.	
B	7/16" APA RATED	2 1/2"x0.131"x0.281"	4"	12"	2x	1'-6"	2x	THD62600 @ 3' - 0" o.c.	
C	7/16" APA RATED OSB	#8 SCREWS	4"	12"	DOUBLE 400S200-54	2'-0"	400T200-54	THD62600 @ 3' - 0" o.c.	

1. ALL PLYWOOD TO BE APA RATED STRUCTURAL 1 EXTERIOR SHEATHING
2. ALL NAILS TO BE COMMON OR GALVANIZED BOX TYPE.
3. FLOOR AND ROOF DIAPHRAGMS TO BE NAILED WITH 3"x0.148"ø NAILS @ 6" o.c. EDGE NAILING AND 12" ON CENTER FIELD NAILING U.N.O. USE PLYWOOD THICKNESS AS INDICATED ON PLAN.
4. ATTACH RIM JOIST AND / OR BLOCKING TO SHEAR WALL AS INDICATED IN TABLE ABOVE
5. ALL WALL SHEATHING TO EXTEND FULL HEIGHT OF WALL, TOP PLATE TO BOTTOM PLATE.
6. ALL SHEAR WALLS AND HOLDOWNS MUST HAVE CONTINUOUS LOAD PATH TO FOUNDATION.
7. USE WASHER PL1/4x3x0"-2" TYPICAL AT ALL ANCHOR BOLTS.
8. WHERE TOP PLATE FASTENING IS LESS THAN 12" o.c., USE MINIMUM BLOCKING OF 2 1/2" MANUFACTURED LUMBER (MICROLLAM LVL, OR PARALLAM PSL).
9. ALL SHEAR WALLS TO BE FULLY BLOCKED U.N.O. BLOCKING TO MATCH REQUIREMENTS FOR PANEL EDGE STUDS.
10. FOR SHEAR WALLS w/ STUDS SPACED @ 24" o.c. MAX. INSTALL SHEATHING WITH LONG DIMENSION ACROSS STUDS.

ROOF / CEILING FRAMING PLAN NOTES

A. COORDINATE ALL DIMENSIONS & FEATURES NOT SHOWN WITH ARCHITECT.

B. SEE SHEET S0.10 FOR ALL NOTES.

C. ALL KEYNOTES INDICATE NEW ITEMS TYPICALLY UNLESS NOTED OTHERWISE.

D. INDICATES ROOF STEP, TYP. SEE ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.

E. BEAMS ARE EQUALLY SPACED IN BAYS, U.N.O.

F. BEAMS ARE CENTERED ON COLUMNS, WALLS, AND/OR GRID LINES, U.N.O.

G. INDICATES (E) CMU WALL BELOW FRAMING.

H. INDICATES (E) CAST IN PLACE CONCRETE WALL.

I. INDICATES CONCRETE INFILL WALL.

J. ⟨X⟩ INDICATES SHEAR WALL TYPE. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

K. INDICATES SHEAR WALL LOCATION BELOW FRAMING. SEE SHEAR WALL SCHEDULE FOR ADDITIONAL INFORMATION.

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M. (#) INDICATES HOLDOWN TYPE AND LOCATION. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.

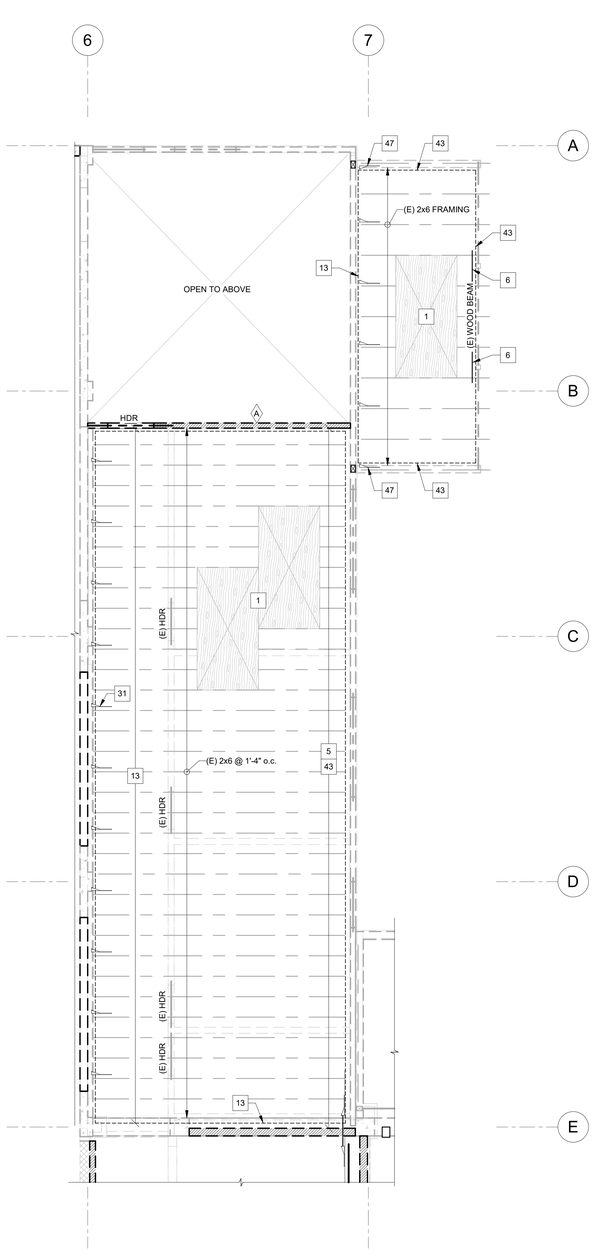
N. HDR TYPICAL EXTERIOR HEADER IS 4x8 D.F. NO.2 USE 1 KING STUD AND 1 TRIMMER UNLESS NOTED OTHERWISE. TYPICAL INTERIOR HEADER TO BE (2) - 2x6 D.F. NO.2 WITH SINGLE TRIMMER AND SINGLE KING STUD (U.N.O.).

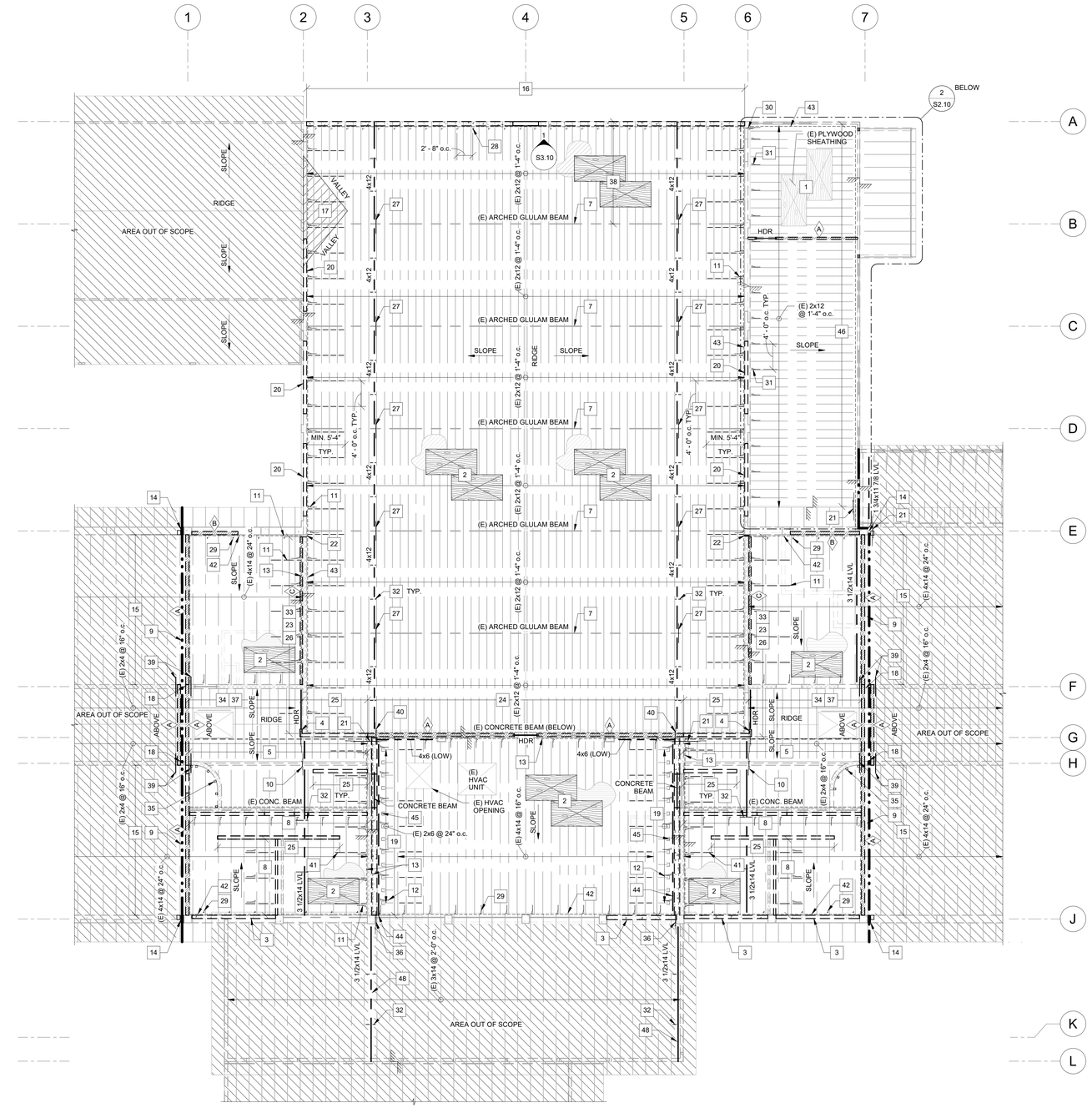
O. INDICATES SEISMIC ISOLATION JOINT. SEE DEMO PLANS FOR EXTENTS OF DEMOLITION.

ROOF / CEILING FRAMING PLAN KEYNOTES

[#]

1. RENAIL (E) PLYWOOD SHEATHING w/ 3"x0.148"ø NAILS @ 6" o.c. PANEL EDGES & 12" o.c. FIELD NAILING.
2. 15/32" STRUCTURAL 1 PLYWOOD SHEATHING OVER (E) 1x DECKING w/ 3"x0.148"ø NAILS @ 6" o.c. PANEL EDGES & 12" o.c. FIELD NAILING.
3. 8" CONCRETE INFILL w/ #5 BARS @ 16" o.c. VERTICAL AND #5 BARS @ 12" o.c. HORIZONTAL. DRILL & BOND REINFORCEMENT w/ SIMPSON SET-3G EPOXY w/ MIN. 6" EMBEDMENT, U.N.O.
4. SIMPSON HDU8-SDS2.5 TENSION TIE TO WOOD & SIMPSON S/HD8S TO CFS WALL w/ 7/8"ø A307 THREADED ROD.
5. 4x FULL DEPTH BLOCKING w/ SIMPSON CMST14 DRAG STRAP w/ 0.162" ø x 2 1/2" NAILS.
6. SIMPSON CS20 DRAG STRAP w/ 0.148" ø x 2 1/2" NAILS.
7. STRENGTHEN (E) GLU-LAM TRUSS. DEMO (E) DIAGONAL SHEATHING & PROVIDE 2x BLOCKING OVER FULL LENGTH OF (E) GLU-LAM BEAM.
8. DOUBLE 350S200-54 CFS STUD @ 16" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES AT 16" HORIZONTAL & 24" o.c. VERTICAL GRID.
9. LINE INDICATES 6" SEISMIC ISOLATION GAP WITH L8x8x3/8" CONTINUOUS STEEL ANGLE.
10. PAIR OF SIMPSON HDU8-SDS2.5 TENSION TIES w/ 7/8"ø A307 THREADED ROD.
11. SIMPSON HGA10KT CLIPS w/ SIMPSON CS20 DRAG STRAPS.
12. SISTER RIPPED 2x14 BLOCK ONTO (E) 2x6. ATTACH TO WALL w/ SIMPSON HTT4 w/ 5/8"ø A307 THREADED ROD. PROVIDE SIMPSON CS20 STRAP w/ 0.148" ø x 2 1/2" NAILS AT FULL DEPTH BLOCKING.
13. IN-PLANE ATTACHMENTS.
14. DEMO 2" OF (E) CONCRETE BEAM TO PROVIDE VERTICAL SEIMSIC ISOLATION.
15. DEMO (E) CMU WALL & REPLACE w/ 2x6 STUD WALL @ 16" o.c. w/ DOULBE TOP PLATE & SINGLE P.T. BOTTOM PLATE.
16. SHORE (E) ROOF FRAMING & DEMOLISH (E) URM ABOVE CONCRETE. PROVIDE NEW 8" THICK REINFORCED CONCRETE WALL w/ #4 BARS @ 16" o.c. VERTICAL & 12" o.c. HORIZONTAL.
17. HATCH INDICATES AREA TO BE OVERFRAMED. PROVIDE 2x8 RIDGE BOARD w/ 2x6 RAFTERS @ 24" o.c. ALIGN WITH (E) RAFTERS. SUPPORT WITH 2x4 POSTS @ 16" o.c. DOWN TO EXSITING RAFTERS. ROOF SHEATHING TO BE 5/8" PLYWOOD SHEATHING w/ 3"x0.148" ø NAILS @ 6" o.c. PANEL EDGES & 12" o.c. FIELD NAILING.
18. DEMO 6" OF (E) CMU WALL TO PROVIDE VERTICAL SEISMIC ISOLATION.
19. NEW 8" THICK REINFORCED SHOTCRETE WALL w/ #5 REBAR @ 12" o.c. E.W.. ANCHOR REINFORCEMENT TO (E) CMU WALL w/ PYTHON ANCHORS @ 24" o.c. VERTICAL & HORIZONTAL GRID.
20. DEMO (E) CMU INFILL AND INFILL OPENING w/ P.T. 2x6 @ 16" o.c. STUD WALL w/ SINGLE BOTTOM PLATE AND DOUBLE TOP PLATE.
21. PAIR OF SIMPSON HTT4 TENSION TIE w/ (18) 0.162"ø x2 1/2" NAILS.
22. SAWCUT 3" OF FULL HIEGHT (E) CMU ADJACENT TO (E) CONCRETE WALL. TERMINATE NEW CFS STUD WALL 3" AWAY FROM ADJACENT CONCRETE WALL.
23. DEMO (E) SILL PLATE 1 1/2" EACH SIDE OF (E) ANCHOR BOLTS.
24. SHORE (E) ROOF FRAMING & DEMOLISH (E) CMU ABOVE CONCRETE. PROVIDE NEW TYPE A WOOD SHEAR WALL SCHEDULE.
25. DEMOLISH (E) CMU WALL. PROVIDE NEW WOOD STUD WALL w/ 2x6 STUDS @ 16" o.c. w/ DOUBLE TOP PLATE & SINGLE P.T. BOTTOM PLATE. PROVIDE 2x FULL DEPTH BLOCKING EA. BAY w/ SIMPSON A35 CLIP @ 16" o.c.
26. DOUBLE 400S200-54 CFS STUD @ 16" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES AT 16" o.c. HORIZONTAL & 24" o.c. VERTICAL GRID.
27. (2) PAIRS OF SIMPSON HDU8-SDS2.5 TENSION TIES w/ 7/8" ø A307 THREADED ROD (4 TOTAL).
28. (2) SIMPSON A35 CLIPS @ 2'-8" o.c. AND 3/4"ø x 12" LONG J-BOLTS @ 18" o.c.. PROVIDE IN-PLANE A35 CLIPS @ 16" o.c.
29. SIMPSON HGA10KT CLIP @ 4'-0" o.c.
30. SIMPSON LTTP2 TENSION TIE w/ 5/8"ø A307 THREADED ROD. DRILL AND BOND THREADED ROD 12" INTO CONCRETE WALL w/ SIMPSON SET-3G EPOXY.
31. SIMPSON LTTP2 TENSION TIE w/ 1/2"ø A307 THREADED ROD. DRILL AND BOND THREADED ROD 4" INTO CONCRETE BEAM w/ SIMPSON SET-3G EPOXY.
32. PROVIDE FULL DEPTH 2x BLOCKING EACH SIDE OF DRAG BEAM AT 1/3 POINTS.
33. 2x FULL DEPTH BLOCKING @ 4'-0" o.c. w/ SIMPSON CS14 DRAG STRAP w/ (26) 0.148"ø x 2 1/2" NAILS AND MIN. 15" END LENGTH.
34. FIBER-REINFORCED POLYMER (FRP) STRENGTHENING OF (E) CMU WALL BOTH SIDES OF THE WALL w/ VERTICAL STRIPS @ 4'-0" o.c.
35. DOUBLE 350S200-54 CFS STUD @ 24" o.c. ATTACH BUILT-UP STUD TO (E) CMU WALL w/ 8mm PROSOCO STITCH TIES @ 16" o.c. HORIZONTAL AND 24" VERTICAL GRID.
36. SIMPSON HDU5-SDS2.5 DRAG CONNECTION w/ 5/8" ø A307 THREADED ROD.
37. 5/8" THICK BENT CONTINUOUS PLATE TOP OF WALL CONNECTION w/ 1/2"ø THROUGH BOLTS @ 24" o.c.
38. PROVIDE 15/32" STRUCTURAL 1 SHEATHING w/ FLAT 2x4 BLOCKING TO EXTENTS SHOWN. FASTEN w/ 3"x0.148"ø NAILS @ 6" o.c. BOUNDARY EDGES & ALL SUPPORTED PANEL EDGES AND 12" o.c. FIELD NAILING.
39. SIMPSON CMSTC16 DRAG STRAP w/ (50) 0.148"ø x 3 1/4" NAILS, MIN. 20" END LENGTH.
40. (2) SIMPSON HDU8-SDS2.5 DRAG CONNECTORS. EXTEND 7/8"ø THREADED ROD INTO SHOTCRETE WALL. TERMINATE w/ 1/2" A36 PLATE.
41. FULL DEPTH 2x BLOCKING w/ SIMPSON HTT4 ANCHOR @ 4'-0" o.c. w/ (18) 0.148" ø x 1 1/2" NAILS AND CS14 STRAP w/ 0.148" ø x 2 1/2" NAILS.
42. SIMPSON A35 CLIP @ 2'-0" o.c. AND SIMPSON THDB62600H @ 4'-0" o.c.
43. SIMPSON A35 CLIP @ 4'-0" o.c.
44. P.T. 2x8 LEDGER w/ SIMPSON THDB2600 @ 18" o.c.
45. SIMPSON LUS26 HANGER.
46. SIMPSON A35 CLIP @ 4'-0" o.c. AT BREAKS IN (E) DBL TOP PLATE PROVIDE SIMPSON CMSTC16 STRAP w/ 0.148"ø x 3 1/4" NAILS & MIN. 20" END LENGTH.
47. SIMPSON DTT1Z w/ (8) 0.148"ø x 1 1/2" NAILS @ 4'-0" o.c.
48. ATTACH DRAG BEAM TO (E) ROOF SHEATHING w/ SIMPSON A35 w/ (12) PH312I FASTENERS @ 24" o.c.



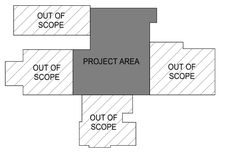


2 S2.10 ENLARGED CEILING FRAMING PLAN
1/4" = 1'-0"

1 S2.10 ROOF FRAMING PLAN
1/8" = 1'-0"

N

KEY PLAN



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ALSEA, OR 97324

ALSEA GYM SEISMIC RETROFIT

PRELIMINARY - NOT FOR CONSTRUCTION

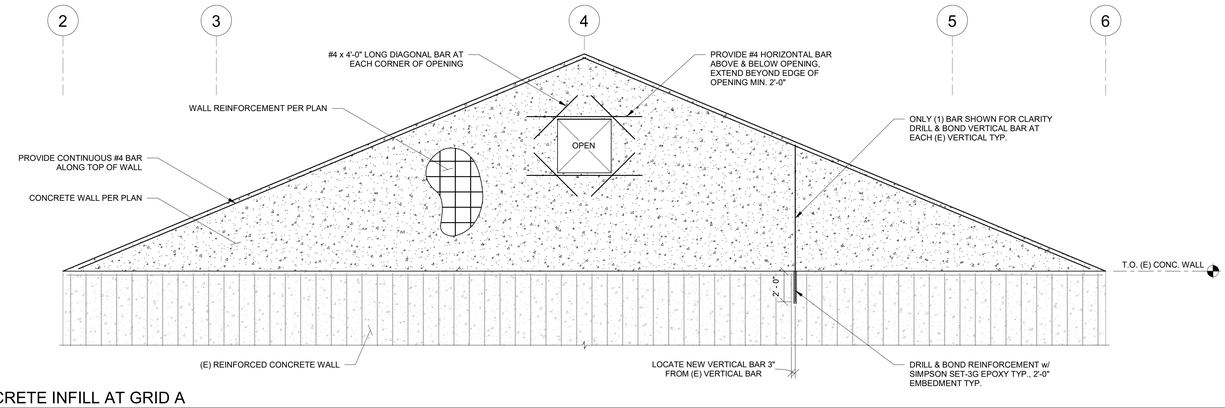
△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

ROOF FRAMING PLAN

S2.10

100% DESIGN DEVELOPMENT | NOT FOR CONSTRUCTION



1 / S3.10 CONCRETE INFILL AT GRID A

1/4" = 1'-0"

ONE INCH EQUALS FULL SCALE

12/3/2024 3:41:35 PM Autodesk Docs://P2935 Alsea Gym & Office Addition/P2935_STRUCT_R23.rvt

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ALSEA GYM
SEISMIC RETROFIT

PRELIMINARY: NOT FOR CONSTRUCTION

EXPIRES: 06-30-25

△	DESCRIPTION	DATE:

PROJECT NO.	P-2935-24
DRAWN:	DKS / JDH
CHECKED:	KDM
DATE:	12-03-24

CONCRETE WALL ELEVATIONS

S3.10

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